



**US Army Corps
of Engineers**

Hydrologic Engineering Center

Application of the Finite Element Method to Vertically Stratified Hydrodynamic Flow and Water Quality

May 1980

REPORT DOCUMENTATION PAGE

Form Approved OMB No. 0704-0188

The public reporting burden for this collection of information is estimated to average 1 hour per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. Send comments regarding this burden estimate or any other aspect of this collection of information, including suggestions for reducing this burden, to the Department of Defense, Executive Services and Communications Directorate (0704-0188). Respondents should be aware that notwithstanding any other provision of law, no person shall be subject to any penalty for failing to comply with a collection of information if it does not display a currently valid OMB control number.

PLEASE DO NOT RETURN YOUR FORM TO THE ABOVE ORGANIZATION.

1. REPORT DATE (DD-MM-YYYY) May 1980		2. REPORT TYPE Technical Paper		3. DATES COVERED (From - To)	
4. TITLE AND SUBTITLE Application of the Finite Element Method to Vertically Stratified Hydrodynamic Flow and Water Quality			5a. CONTRACT NUMBER		
			5b. GRANT NUMBER		
			5c. PROGRAM ELEMENT NUMBER		
			5d. PROJECT NUMBER		
6. AUTHOR(S) Robert C. MacArthur, William R. Norton			5e. TASK NUMBER		
			5f. WORK UNIT NUMBER		
			8. PERFORMING ORGANIZATION REPORT NUMBER TP-72		
7. PERFORMING ORGANIZATION NAME(S) AND ADDRESS(ES) US Army Corps of Engineers Institute for Water Resources Hydrologic Engineering Center (HEC) 609 Second Street Davis, CA 95616-4687					
9. SPONSORING/MONITORING AGENCY NAME(S) AND ADDRESS(ES)			10. SPONSOR/ MONITOR'S ACRONYM(S)		
			11. SPONSOR/ MONITOR'S REPORT NUMBER(S)		
12. DISTRIBUTION / AVAILABILITY STATEMENT Approved for public release; distribution is unlimited.					
13. SUPPLEMENTARY NOTES Presented at the third International Conference on Finite Elements in Water Resources, at the University of Mississippi, Oxford, MS, 19-23 May 1980					
14. ABSTRACT Computer program RMA-7 (King, et al, 1973) has been expanded to be able to simulate density induced flows and water quality conditions typically found in deep reservoirs. The mathematical basis for the program and methods of implementing the code for various kinds of flow are discussed. Results from two different applications are presented. The first example simulates flow conditions that were measured in a physical model of a deep reservoir. Results are also included for simulations of the circulation, temperature and dissolved oxygen concentrations for Lake Taneycomo, Missouri. Comparison of measured and computed results from both applications shows that the RMA-7 model provides reasonably accurate results for both flow circulation and water quality.					
15. SUBJECT TERMS finite element methods, stratified flow, two-dimensional flow, reservoir modeling, water quality, computer modeling					
16. SECURITY CLASSIFICATION OF:			17. LIMITATION OF ABSTRACT UU	18. NUMBER OF PAGES 20	19a. NAME OF RESPONSIBLE PERSON
a. REPORT U	b. ABSTRACT U	c. THIS PAGE U			19b. TELEPHONE NUMBER

Application of the Finite Element Method to Vertically Stratified Hydrodynamic Flow and Water Quality

May 1980

US Army Corps of Engineers
Institute for Water Resources
Hydrologic Engineering Center
609 Second Street
Davis, CA 95616

(530) 756-1104
(530) 756-8250 FAX
www.hec.usace.army.mil

TP-72

Papers in this series have resulted from technical activities of the Hydrologic Engineering Center. Versions of some of these have been published in technical journals or in conference proceedings. The purpose of this series is to make the information available for use in the Center's training program and for distribution with the Corps of Engineers.

The findings in this report are not to be construed as an official Department of the Army position unless so designated by other authorized documents.

The contents of this report are not to be used for advertising, publication, or promotional purposes. Citation of trade names does not constitute an official endorsement or approval of the use of such commercial products.

APPLICATION OF THE FINITE ELEMENT METHOD TO VERTICALLY
STRATIFIED HYDRODYNAMIC FLOW AND WATER QUALITY¹

R. C. MacArthur² and W. R. Norton³

INTRODUCTION

Continuing interest in the internal processes of reservoirs, lakes and estuaries has intensified the development of mathematical models for simulation of vertically stratified flow. Motivated primarily by the long term desire to predict the water quality response to system modifications, current modeling efforts have focused on the need to describe flow coupled with temperature and/or salinity fields in order to forecast the influence of density induced flows. Computational algorithms have shown sufficient promise that efforts are under way to collect prototype data which can be used for calibration and verification of both flow and water quality models.

Among the several models which have been formulated to simulate density induced flows and water quality is one called RMA-7. This model, which was originally developed for the Office of Water Resources Research, (King, 1973) has existed for several years but has received relatively little use in prototype applications. It is the intent of the work reported herein to demonstrate the operation of the current version of RMA-7 and to offer appropriate information and comments on the use and implementation of the model.

Specifically, the paragraphs which follow contain a brief description of the mathematical basis of RMA-7 as well as an example of its implementation on a prototype system which approximates the physical dimensions of the so-called GRH flume at the Corps' Waterways Experiment Station in Vicksburg. Also included are some results obtained by the model from the simulation of temperature and dissolved oxygen for Lake Taneycomo in Missouri. Several statistical comparisons are included between simulated and measured values for Lake Taneycomo which are designed to quantify, to some extent, the accuracy of the model.

¹Presented at the 3rd International Conference on Finite Elements in Water Resources, May 19-23, 1980, the University of Mississippi, Oxford, MS.

²Hydraulic Engineer, the Hydrologic Engineering Center, Davis, California.

³Principal, Resource Management Associates, Lafayette, California.

GOVERNING EQUATIONS - DIFFERENTIAL FORMS

RMA-7 is a two dimensional mathematical model which describes the behavior of velocity, pressure, temperature and dissolved oxygen in the vertical plane with homogeneity assumed in the third (lateral) direction; the model will accommodate width gradients in both the X and Y directions. This model is formulated on the classical concepts of conservation of mass, momentum, and energy although it is somewhat unusual as it retains the complete vertical momentum equation and does not make the assumption that pressure must be hydrostatic.

Hydrodynamic Model

The equations used in RMA-7 have the following differential forms:

Velocity equations.

$$\rho \left(\frac{\partial u}{\partial t} + u \frac{\partial u}{\partial x} + v \frac{\partial u}{\partial y} \right) + \frac{\partial p}{\partial x} - \epsilon_{xx} \frac{\partial^2 u}{\partial x^2} - \epsilon_{xy} \frac{\partial^2 u}{\partial y^2} = 0 \quad (1)$$

$$\rho \left(\frac{\partial v}{\partial t} + u \frac{\partial v}{\partial x} + v \frac{\partial v}{\partial y} \right) + \frac{\partial p}{\partial y} + \rho g - \epsilon_{yx} \frac{\partial^2 v}{\partial x^2} - \epsilon_{yy} \frac{\partial^2 v}{\partial y^2} = 0 \quad (2)$$

Continuity equation.

$$\frac{\partial}{\partial x} (wu) + \frac{\partial}{\partial y} (wv) = 0 \quad (3)$$

Temperature equation.

$$c\rho \frac{\partial T}{\partial t} + C\rho u \frac{\partial T}{\partial x} + C\rho v \frac{\partial T}{\partial y} - D_x \frac{\partial^2 T}{\partial x^2} - D_y \frac{\partial^2 T}{\partial y^2} - \phi_2 = 0 \quad (4)$$

where

- u = X direction velocity
- v = Y direction velocity
- p = pressure
- T = temperature, degrees C
- t = time
- ρ = fluid density
- $f(T)$
- g = gravitational acceleration
- C = specific heat
- ϕ_2 = thermal heat source or sink
- w = width
- $\epsilon_{xx}, \epsilon_{xy}$ = eddy viscosity coefficients
- $\epsilon_{yx}, \epsilon_{yy}$
- D_x, D_y = eddy dispersion coefficients

Water Quality Model

The equations used in RMA-7 to describe the behavior of dissolved oxygen have the differential forms:

Dissolved oxygen equation.

$$\frac{\partial c_1}{\partial t} + u \frac{\partial c_1}{\partial x} + v \frac{\partial c_1}{\partial y} - D_x \frac{\partial^2 c_1}{\partial x^2} - D_y \frac{\partial^2 c_1}{\partial y^2} + K_2 c_2 - \alpha \mu c_3 = 0 \quad (5)$$

Carbonaceous biochemical oxygen demand (BOD) equation.

$$\frac{\partial c_2}{\partial t} + u \frac{\partial c_2}{\partial x} + v \frac{\partial c_2}{\partial y} - D_x \frac{\partial^2 c_2}{\partial x^2} - D_y \frac{\partial^2 c_2}{\partial y^2} + K_2 c_2 = 0 \quad (6)$$

Phytoplankton equation.

$$\frac{\partial c_3}{\partial t} + u \frac{\partial c_3}{\partial x} + v \frac{\partial c_3}{\partial y} - D_x \frac{\partial^2 c_3}{\partial x^2} - D_y \frac{\partial^2 c_3}{\partial y^2} - \mu c_3 = 0 \quad (7)$$

- where c_1 = dissolved oxygen concentration
 c_2 = carbonaceous BOD concentration
 c_3 = phytoplankton concentration (dry weight)
 K_2 = BOD decay rate
 α = the mass of oxygen produced per unit mass of phytoplankton growth
 μ = the local rate of phytoplankton growth
= $\mu_{\max} \left(\frac{I}{I + K_I} \right) - r$

- where μ_{\max} = the maximum specific phytoplankton growth rate at the local temperature
 I = the intensity of light at the local depth
 K_I = the light half saturation constant
 r = the phytoplankton respiration rate at the local temperature

and all other terms as previously defined.

Equations 1 through 7 represent the mathematical basis for the model RMA-7. It is recognized that these equations provide an approximate representation of the governing processes in that certain second order terms with respect to width have been dropped and that the viscosity/dispersion terms are largely empirical. Notwithstanding these shortcomings, however, the above relationships have shown promise in simulation of observed phenomena, and provide a general framework for testing and improving the procedures necessary for simulation of vertically stratified flow.

As can be seen, the first four equations and the second three equations each form a closed set which must be solved simultaneously. The coupling between the two sets is manifest in the convective velocities u and v , and in the water temperature, T . Fortunately, the coupling between the

two sets is of the "feed forward" type in that equations 1 through 4 may be solved independently of equations 5, 6 and 7, with the results of the hydrodynamic solution acting like coefficients in the solution of the water quality model.

In addition to the volumetric terms shown above, the model contains a number of additional features to account for surface effects and source/sink terms. Most important of these are: 1) bottom friction at the soil-water interface calculated as a function of bottom velocity and a Chezy coefficient; 2) surface heat exchange as a function of the water temperature, the equilibrium temperature and an exchange coefficient; 3) internal absorption of short wave solar radiation as a function of depth; and 4) surface oxygen exchange as a function of the local oxygen deficit and an exchange coefficient which is a function of wind speed.

SOLUTION TECHNIQUE

The governing equations are solved by the finite element method using Galerkin's criteria for the method of weighted residuals. The formulation employs a mixed set of basis functions, with quadratic functions used for all state variables except pressure where a linear function is used. The linear pressure function implies a constant element density, which is calculated as a function of average nodal temperatures. Green's Theorem is used to lower the order of all second derivatives in the viscosity/dispersion terms, resulting in surface integrals which must be evaluated (either implicitly or explicitly) along system extremities. Green's Theorem is also used on the pressure terms of equations 1 and 2 permitting a surface integral to be used as a discharge boundary condition rather than a specified pressure value; this procedure permits retention of all nodal continuity equations and substantially improves the model's performance.

RMA-7 uses an implicit, Newton-Raphson computation scheme to achieve a solution to the set of nonlinear equations which define the model. The computer program accommodates either triangular or quadrilateral isoparametric elements with numerical integration used to evaluate all surface and area integrals. The use of the isoparametric formulation with interelement geometric slope continuity allows flow to move parallel to boundaries at all points while at the same time providing a means for reasonable representation of real physical systems.

EXAMPLE PROBLEMS

The model described above has been applied to several physical situations, two of which are summarized below. The first example shows results calculated from tests conducted with the geometry of the GRH flume at WES. The second shows results calculated from conditions found in Lake Taneycomo in Missouri. The first example was run using only the flow and temperature portions of the model, while the second includes flow, temperature and dissolved oxygen. The networks used for each of these example problems are reproduced in figure 1.

Example Problem 1 - GRH Flume

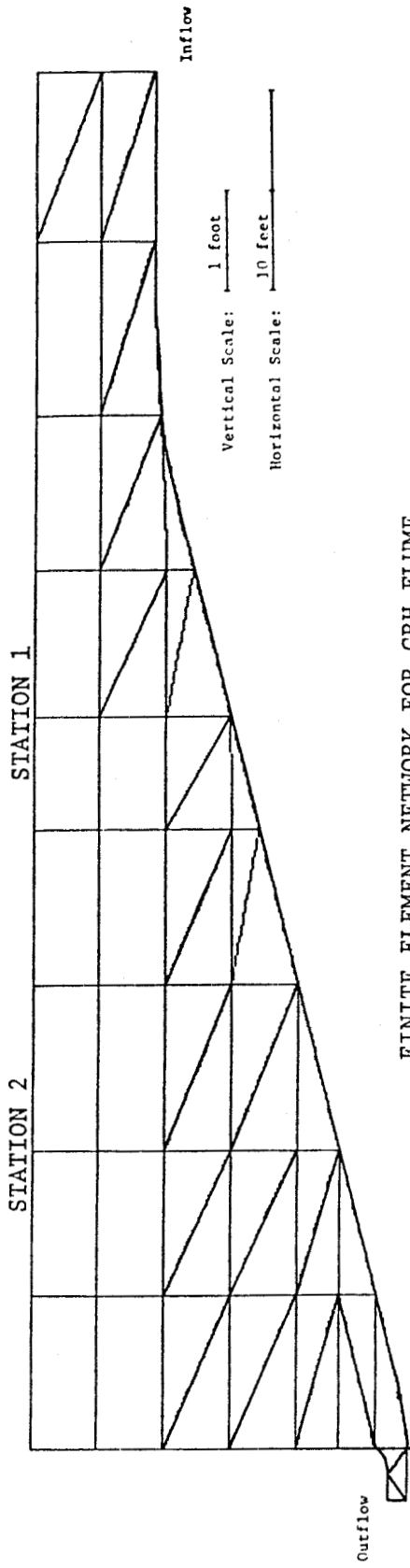
The GRH flume is a hydrodynamic test facility which is 80 feet long and varies in depth from approximately 1 foot at its upper (inflow) end to 3 feet at its lower (outflow) end, with two different cross sections along its length. The inflow section, which is 20 feet long, has a horizontal bottom and varies linearly in width from 1.0 foot to 2.85 feet. The lower section has a constant width of 2.85 feet, but a bottom which drops 2 feet over its 60 foot length.

RMA-7 was applied to the GRH flume geometry by construction of a finite element network containing 57 elements and 158 node points as shown in figure 1. In constructing this network it was felt desirable to allow flow to move parallel to the flume bottom at all locations. For this reason continuous curves were passed through the breakpoint on the flume's bottom 20 feet from the upstream end and at the transition to the outlet. This type of construction permits continuous velocities to exist along the flume bottom and completely eliminates artificial stagnation points. The small discharge nozzle at the outlet has been included for each boundary condition specification and does not exist on the physical flume.

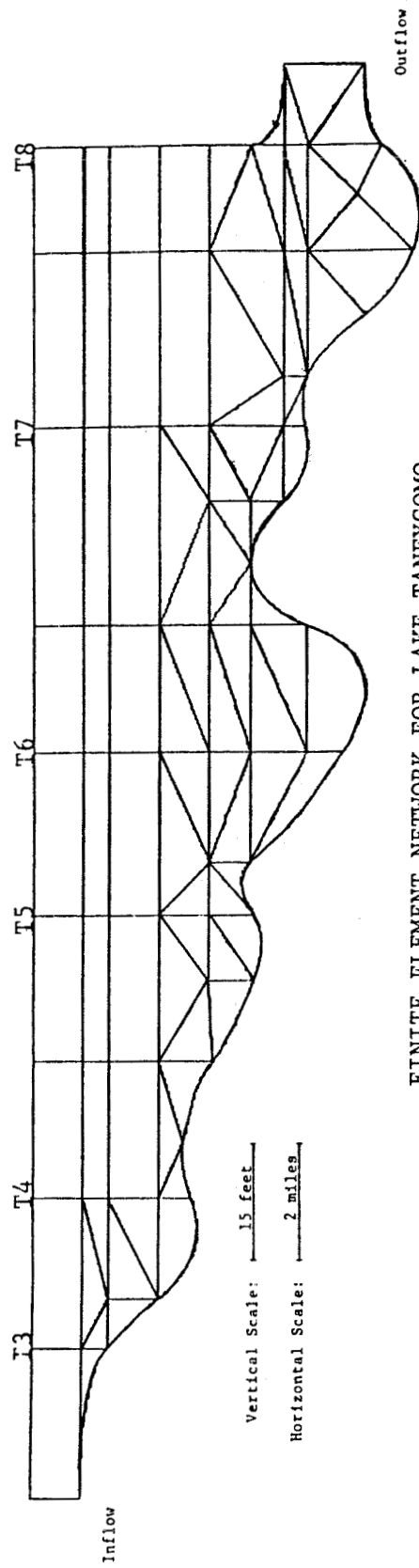
To run RMA-7 it is necessary to specify values for eddy viscosity and eddy diffusion coefficients. At present, this is done by experience and numerical testing of problems which have known or assumed velocity and temperature distributions. In the GRH flume case, a series of numerical tests were conducted on a steady state problem to determine a set of satisfactory coefficients, and the network's sensitivity to the various coefficients. The values determined for the GRH flume had the relative values of $\epsilon_{xx} = 0.05$, $\epsilon_{xy} = 0.0005$, $\epsilon_{yx} = 0.01$, $\epsilon_{yy} = 0.1$, $D_x = 0.1$, and $D_y = 0.0005$, after accounting for element distortion and size.

Results from two examples are shown, one for a homogeneous flow and one for a nonhomogeneous flow. In each case a flow of 10 gpm was introduced into a still flume with a linear velocity distribution in the lower element at the inflow end. The homogeneous case was run isothermally at a temperature of 10.3°C, while the nonhomogeneous case was started with an initial temperature of 10.3°C and an inflow of 5°C.

Velocity distributions produced by each condition are graphically compared at three times in figure 2. The effects of the density stratification are quite evident between the two cases with the colder, more dense water underflowing the lighter and warmer water near the surface. The results shown are representative of ongoing work with the GRH flume, although no measured data is currently available for model/prototype comparisons under the conditions simulated. The general shape of velocity distributions, and the arrival time of the temperature front (15-18 min) are in general agreement with measured data, however, and suggest the model will perform well when suitable data become available.



FINITE ELEMENT NETWORK FOR GRH FLUME
EXAMPLE PROBLEM NUMBER 1



FINITE ELEMENT NETWORK FOR LAKE TANEYCOMO
EXAMPLE PROBLEM NUMBER 2

Figure 1.--Example Problem Networks

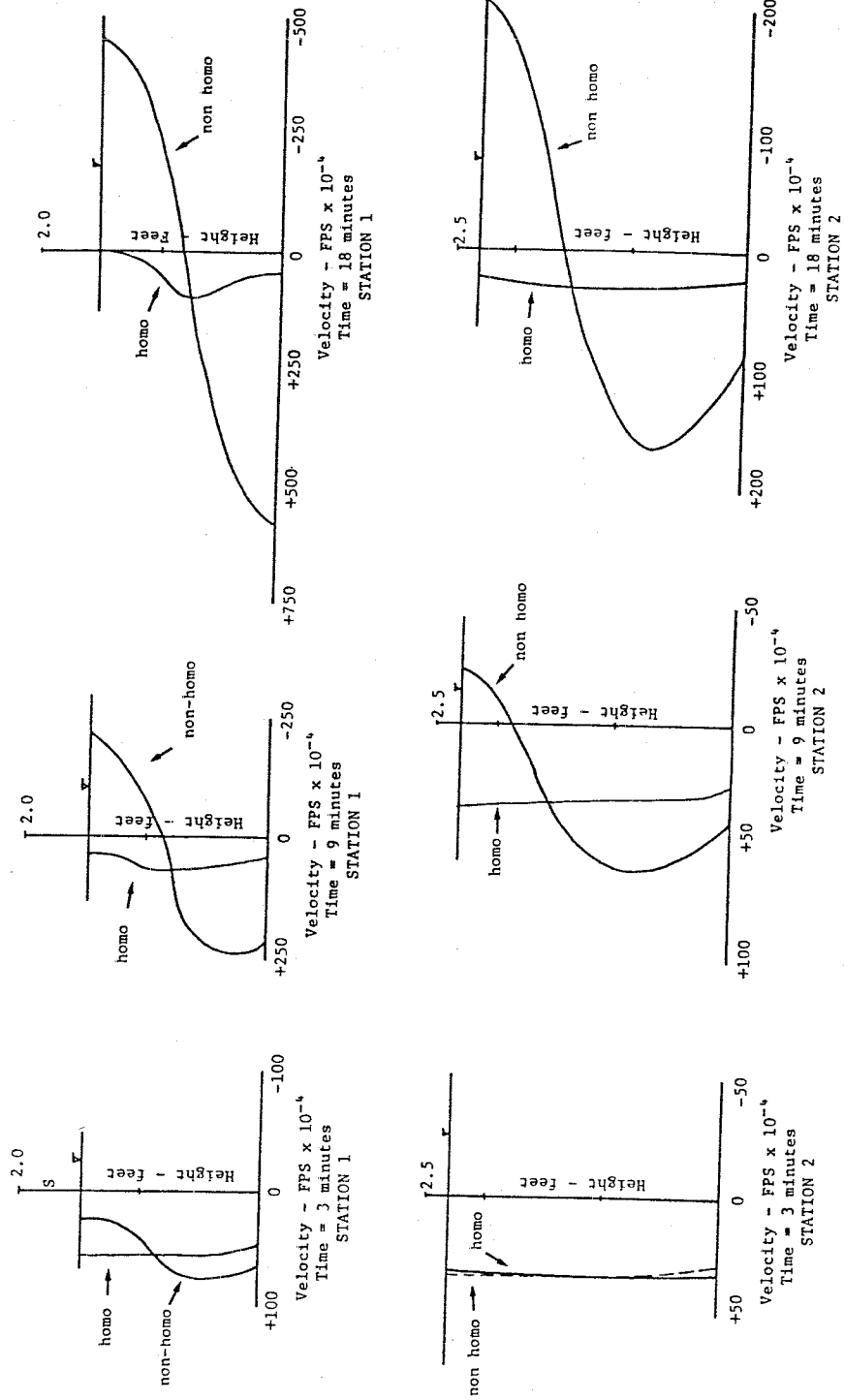


Figure 2.--Comparison of Homogeneous and Non Homogeneous Simulated Velocity Distributions for the GRH Flume Example Problem

Example Problem 2 - Lake Taneycomo

The second example problem presents results obtained from the simulation of Lake Taneycomo in southern Missouri. Lake Taneycomo is a 26-mile-long reservoir bounded by Table Rock Dam upstream and Bull Shoals Reservoir downstream, and is used for power production and recreation among other things. As low dissolved oxygen has been observed in the lake, RMA-7 was applied with the goal of evaluating the impact of changes in reservoir operation on the ambient levels of dissolved oxygen.

To conduct the required simulations a network of 92 elements and 246 node points was constructed as shown in figure 1; lateral width varied from about 200 to 1000 feet with depth at a typical cross section. Diffusion coefficients and eddy viscosity coefficients were chosen to be consistent with those used in the GRH flume when scaled for element distortion.

Detailed water temperature and dissolved oxygen measurements were available for three separate week long periods in the fall of 1977. RMA-7 was calibrated against two of these periods and verified against the third. Typical measured and simulated vertical profiles for temperature and dissolved oxygen are presented for stations T6 and T8 in figure 3 for the earliest calibration period. It should be noted that the model had simulated over five days of operation by the time shown in these figures, and that the inflow hydrograph varied from 0 to 7000 cfs in a four to six hour period on a regular basis.

In order to bring a degree of quantification to the accuracy of the model, a linear least squares regression of simulated to observed temperature and dissolved oxygen has been made as shown in figure 4, with the statistics for each fit given in table 1.

As can be seen, these statistics indicate a fairly good fit of both temperature and dissolved oxygen, and seem reasonable in light of the uncertainty in both the model's upstream inputs (BOD, dissolved oxygen, meteorological data, etc.) and the usual measurement errors.

Table 1.--Regression Statistics

Regression Statistic	STATION T6		STATION T8	
	Temperature	Oxygen	Temperature	Oxygen
intercept	4.77	3.27	0.29	3.30
slope	0.58	0.45	0.96	0.39
s.d. error	0.65	0.49	0.43	0.30
correlation	0.44	0.30	0.89	0.81

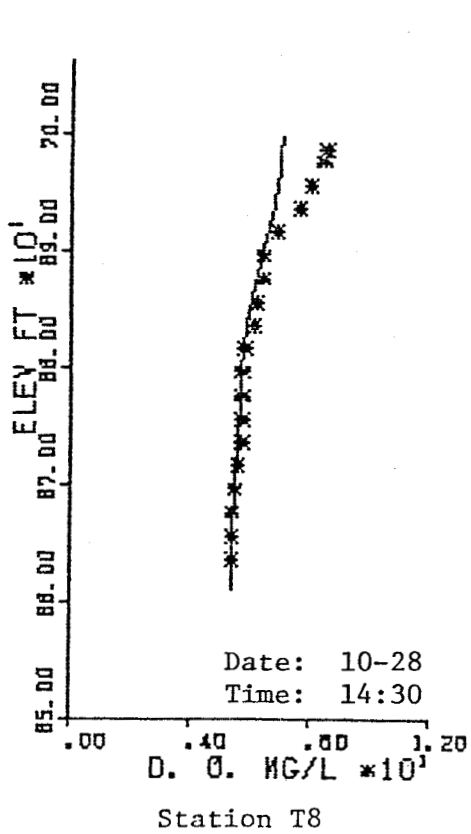
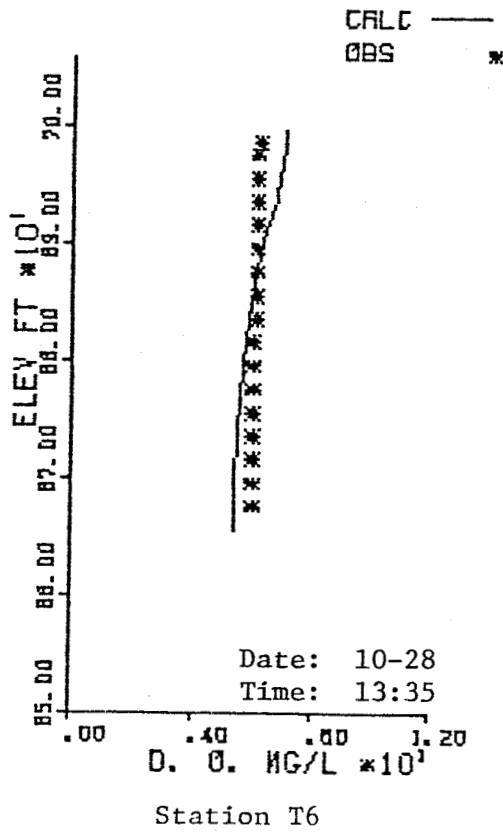
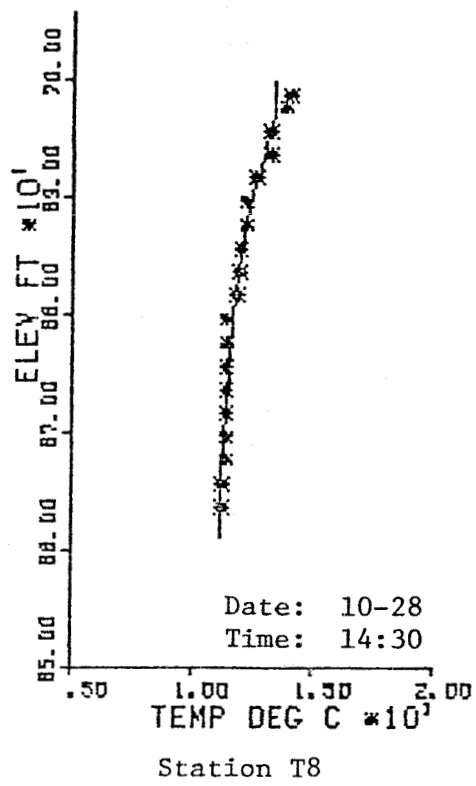
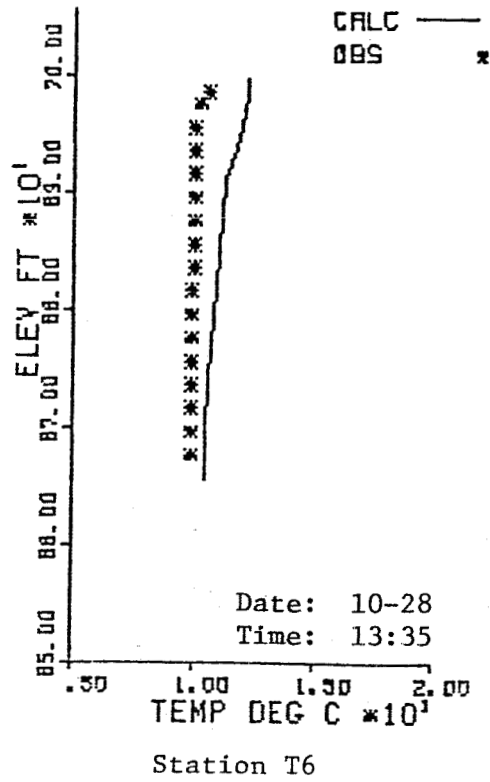
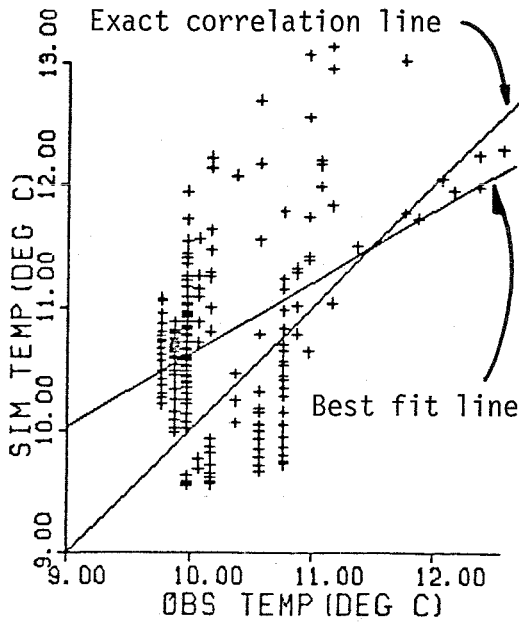
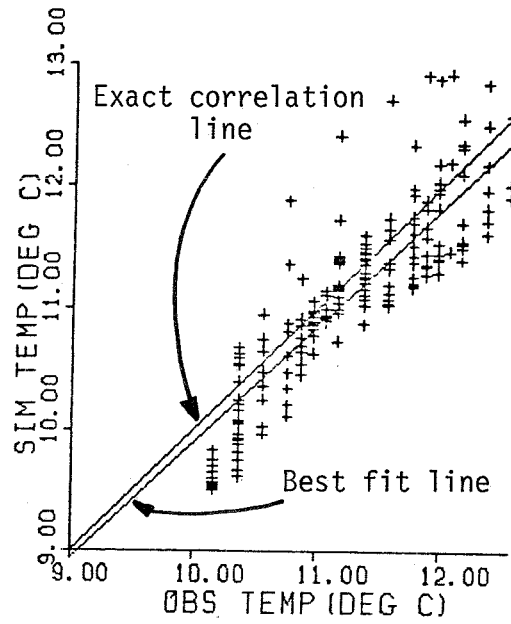


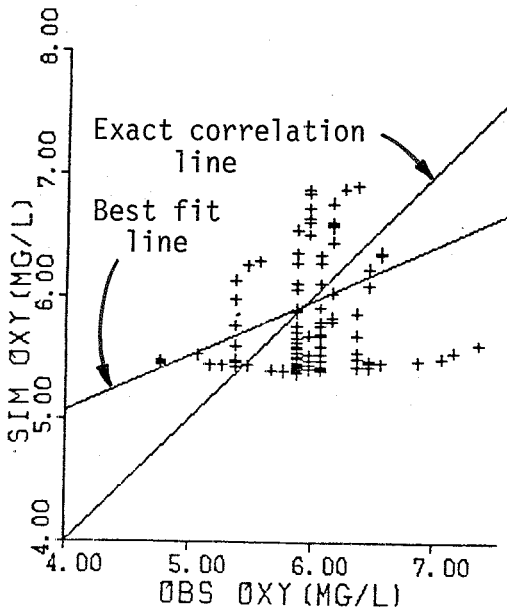
Figure 3.--Observed and Simulated Water Quality for Lake Taneycomo



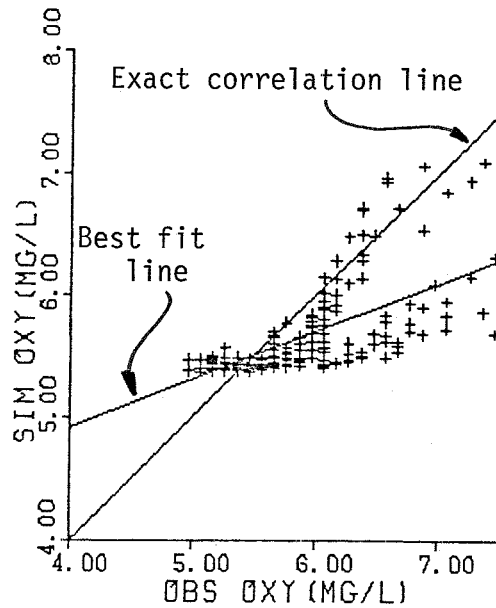
Station T6



Station T8



Station T6



Station T8

Figure 4.--Statistical Regressions for Lake Taneycomo Temperature and Dissolved Oxygen

SUMMARY AND CONCLUSIONS

The above information summarizes the application of a two dimensional finite element model (RMA-7) to two prototype stratified flow situations. In each application a stratified flow was simulated as a result of density differences arising from temperature gradients. In the second example dissolved oxygen, BOD and phytoplankton were also routed in accordance with the overall flow fields.

Based on the data contained herein it seems reasonable to conclude:

- . the finite element method in general and RMA-7 in particular, is capable of simulating vertically stratified two dimensional flow.
- . the proper definition of eddy viscosity and dispersion coefficients is essential to proper model operation, and that there may be transferability of values from problem to problem if element size and distortion is accounted for.
- . the RMA-7 model appears to give reasonable answers, but it is not currently possible to evaluate its accuracy in relation to velocity due to a lack of prototype data; initial comparisons of temperature and dissolved oxygen data are promising and will improve as calibrations become more rigorous and quantified.

REFERENCE

- King, I. P.; W. R. Norton; G. T. Orlob
1973. A finite element solution for two-dimensional density stratified flow. Prepared for the U. S. Dept. of the Interior, Office of Water Resources Research. Water Resources Engineers.

Technical Paper Series

TP-1	Use of Interrelated Records to Simulate Streamflow	TP-39	A Method for Analyzing Effects of Dam Failures in Design Studies
TP-2	Optimization Techniques for Hydrologic Engineering	TP-40	Storm Drainage and Urban Region Flood Control Planning
TP-3	Methods of Determination of Safe Yield and Compensation Water from Storage Reservoirs	TP-41	HEC-5C, A Simulation Model for System Formulation and Evaluation
TP-4	Functional Evaluation of a Water Resources System	TP-42	Optimal Sizing of Urban Flood Control Systems
TP-5	Streamflow Synthesis for Ungaged Rivers	TP-43	Hydrologic and Economic Simulation of Flood Control Aspects of Water Resources Systems
TP-6	Simulation of Daily Streamflow	TP-44	Sizing Flood Control Reservoir Systems by System Analysis
TP-7	Pilot Study for Storage Requirements for Low Flow Augmentation	TP-45	Techniques for Real-Time Operation of Flood Control Reservoirs in the Merrimack River Basin
TP-8	Worth of Streamflow Data for Project Design - A Pilot Study	TP-46	Spatial Data Analysis of Nonstructural Measures
TP-9	Economic Evaluation of Reservoir System Accomplishments	TP-47	Comprehensive Flood Plain Studies Using Spatial Data Management Techniques
TP-10	Hydrologic Simulation in Water-Yield Analysis	TP-48	Direct Runoff Hydrograph Parameters Versus Urbanization
TP-11	Survey of Programs for Water Surface Profiles	TP-49	Experience of HEC in Disseminating Information on Hydrological Models
TP-12	Hypothetical Flood Computation for a Stream System	TP-50	Effects of Dam Removal: An Approach to Sedimentation
TP-13	Maximum Utilization of Scarce Data in Hydrologic Design	TP-51	Design of Flood Control Improvements by Systems Analysis: A Case Study
TP-14	Techniques for Evaluating Long-Term Reservoir Yields	TP-52	Potential Use of Digital Computer Ground Water Models
TP-15	Hydrostatistics - Principles of Application	TP-53	Development of Generalized Free Surface Flow Models Using Finite Element Techniques
TP-16	A Hydrologic Water Resource System Modeling Techniques	TP-54	Adjustment of Peak Discharge Rates for Urbanization
TP-17	Hydrologic Engineering Techniques for Regional Water Resources Planning	TP-55	The Development and Servicing of Spatial Data Management Techniques in the Corps of Engineers
TP-18	Estimating Monthly Streamflows Within a Region	TP-56	Experiences of the Hydrologic Engineering Center in Maintaining Widely Used Hydrologic and Water Resource Computer Models
TP-19	Suspended Sediment Discharge in Streams	TP-57	Flood Damage Assessments Using Spatial Data Management Techniques
TP-20	Computer Determination of Flow Through Bridges	TP-58	A Model for Evaluating Runoff-Quality in Metropolitan Master Planning
TP-21	An Approach to Reservoir Temperature Analysis	TP-59	Testing of Several Runoff Models on an Urban Watershed
TP-22	A Finite Difference Methods of Analyzing Liquid Flow in Variably Saturated Porous Media	TP-60	Operational Simulation of a Reservoir System with Pumped Storage
TP-23	Uses of Simulation in River Basin Planning	TP-61	Technical Factors in Small Hydropower Planning
TP-24	Hydroelectric Power Analysis in Reservoir Systems	TP-62	Flood Hydrograph and Peak Flow Frequency Analysis
TP-25	Status of Water Resource System Analysis	TP-63	HEC Contribution to Reservoir System Operation
TP-26	System Relationships for Panama Canal Water Supply	TP-64	Determining Peak-Discharge Frequencies in an Urbanizing Watershed: A Case Study
TP-27	System Analysis of the Panama Canal Water Supply	TP-65	Feasibility Analysis in Small Hydropower Planning
TP-28	Digital Simulation of an Existing Water Resources System	TP-66	Reservoir Storage Determination by Computer Simulation of Flood Control and Conservation Systems
TP-29	Computer Application in Continuing Education	TP-67	Hydrologic Land Use Classification Using LANDSAT
TP-30	Drought Severity and Water Supply Dependability	TP-68	Interactive Nonstructural Flood-Control Planning
TP-31	Development of System Operation Rules for an Existing System by Simulation	TP-69	Critical Water Surface by Minimum Specific Energy Using the Parabolic Method
TP-32	Alternative Approaches to Water Resources System Simulation		
TP-33	System Simulation of Integrated Use of Hydroelectric and Thermal Power Generation		
TP-34	Optimizing flood Control Allocation for a Multipurpose Reservoir		
TP-35	Computer Models for Rainfall-Runoff and River Hydraulic Analysis		
TP-36	Evaluation of Drought Effects at Lake Atitlan		
TP-37	Downstream Effects of the Levee Overtopping at Wilkes-Barre, PA, During Tropical Storm Agnes		
TP-38	Water Quality Evaluation of Aquatic Systems		

- TP-70 Corps of Engineers Experience with Automatic Calibration of a Precipitation-Runoff Model
- TP-71 Determination of Land Use from Satellite Imagery for Input to Hydrologic Models
- TP-72 Application of the Finite Element Method to Vertically Stratified Hydrodynamic Flow and Water Quality
- TP-73 Flood Mitigation Planning Using HEC-SAM
- TP-74 Hydrographs by Single Linear Reservoir Model
- TP-75 HEC Activities in Reservoir Analysis
- TP-76 Institutional Support of Water Resource Models
- TP-77 Investigation of Soil Conservation Service Urban Hydrology Techniques
- TP-78 Potential for Increasing the Output of Existing Hydroelectric Plants
- TP-79 Potential Energy and Capacity Gains from Flood Control Storage Reallocation at Existing U.S. Hydropower Reservoirs
- TP-80 Use of Non-Sequential Techniques in the Analysis of Power Potential at Storage Projects
- TP-81 Data Management Systems of Water Resources Planning
- TP-82 The New HEC-1 Flood Hydrograph Package
- TP-83 River and Reservoir Systems Water Quality Modeling Capability
- TP-84 Generalized Real-Time Flood Control System Model
- TP-85 Operation Policy Analysis: Sam Rayburn Reservoir
- TP-86 Training the Practitioner: The Hydrologic Engineering Center Program
- TP-87 Documentation Needs for Water Resources Models
- TP-88 Reservoir System Regulation for Water Quality Control
- TP-89 A Software System to Aid in Making Real-Time Water Control Decisions
- TP-90 Calibration, Verification and Application of a Two-Dimensional Flow Model
- TP-91 HEC Software Development and Support
- TP-92 Hydrologic Engineering Center Planning Models
- TP-93 Flood Routing Through a Flat, Complex Flood Plain Using a One-Dimensional Unsteady Flow Computer Program
- TP-94 Dredged-Material Disposal Management Model
- TP-95 Infiltration and Soil Moisture Redistribution in HEC-1
- TP-96 The Hydrologic Engineering Center Experience in Nonstructural Planning
- TP-97 Prediction of the Effects of a Flood Control Project on a Meandering Stream
- TP-98 Evolution in Computer Programs Causes Evolution in Training Needs: The Hydrologic Engineering Center Experience
- TP-99 Reservoir System Analysis for Water Quality
- TP-100 Probable Maximum Flood Estimation - Eastern United States
- TP-101 Use of Computer Program HEC-5 for Water Supply Analysis
- TP-102 Role of Calibration in the Application of HEC-6
- TP-103 Engineering and Economic Considerations in Formulating
- TP-104 Modeling Water Resources Systems for Water Quality
- TP-105 Use of a Two-Dimensional Flow Model to Quantify Aquatic Habitat
- TP-106 Flood-Runoff Forecasting with HEC-1F
- TP-107 Dredged-Material Disposal System Capacity Expansion
- TP-108 Role of Small Computers in Two-Dimensional Flow Modeling
- TP-109 One-Dimensional Model for Mud Flows
- TP-110 Subdivision Froude Number
- TP-111 HEC-5Q: System Water Quality Modeling
- TP-112 New Developments in HEC Programs for Flood Control
- TP-113 Modeling and Managing Water Resource Systems for Water Quality
- TP-114 Accuracy of Computer Water Surface Profiles - Executive Summary
- TP-115 Application of Spatial-Data Management Techniques in Corps Planning
- TP-116 The HEC's Activities in Watershed Modeling
- TP-117 HEC-1 and HEC-2 Applications on the Microcomputer
- TP-118 Real-Time Snow Simulation Model for the Monongahela River Basin
- TP-119 Multi-Purpose, Multi-Reservoir Simulation on a PC
- TP-120 Technology Transfer of Corps' Hydrologic Models
- TP-121 Development, Calibration and Application of Runoff Forecasting Models for the Allegheny River Basin
- TP-122 The Estimation of Rainfall for Flood Forecasting Using Radar and Rain Gage Data
- TP-123 Developing and Managing a Comprehensive Reservoir Analysis Model
- TP-124 Review of U.S. Army corps of Engineering Involvement With Alluvial Fan Flooding Problems
- TP-125 An Integrated Software Package for Flood Damage Analysis
- TP-126 The Value and Depreciation of Existing Facilities: The Case of Reservoirs
- TP-127 Floodplain-Management Plan Enumeration
- TP-128 Two-Dimensional Floodplain Modeling
- TP-129 Status and New Capabilities of Computer Program HEC-6: "Scour and Deposition in Rivers and Reservoirs"
- TP-130 Estimating Sediment Delivery and Yield on Alluvial Fans
- TP-131 Hydrologic Aspects of Flood Warning - Preparedness Programs
- TP-132 Twenty-five Years of Developing, Distributing, and Supporting Hydrologic Engineering Computer Programs
- TP-133 Predicting Deposition Patterns in Small Basins
- TP-134 Annual Extreme Lake Elevations by Total Probability Theorem
- TP-135 A Muskingum-Cunge Channel Flow Routing Method for Drainage Networks
- TP-136 Prescriptive Reservoir System Analysis Model - Missouri River System Application
- TP-137 A Generalized Simulation Model for Reservoir System Analysis
- TP-138 The HEC NexGen Software Development Project
- TP-139 Issues for Applications Developers
- TP-140 HEC-2 Water Surface Profiles Program
- TP-141 HEC Models for Urban Hydrologic Analysis

- TP-142 Systems Analysis Applications at the Hydrologic Engineering Center
- TP-143 Runoff Prediction Uncertainty for Ungauged Agricultural Watersheds
- TP-144 Review of GIS Applications in Hydrologic Modeling
- TP-145 Application of Rainfall-Runoff Simulation for Flood Forecasting
- TP-146 Application of the HEC Prescriptive Reservoir Model in the Columbia River Systems
- TP-147 HEC River Analysis System (HEC-RAS)
- TP-148 HEC-6: Reservoir Sediment Control Applications
- TP-149 The Hydrologic Modeling System (HEC-HMS): Design and Development Issues
- TP-150 The HEC Hydrologic Modeling System
- TP-151 Bridge Hydraulic Analysis with HEC-RAS
- TP-152 Use of Land Surface Erosion Techniques with Stream Channel Sediment Models
- TP-153 Risk-Based Analysis for Corps Flood Project Studies - A Status Report
- TP-154 Modeling Water-Resource Systems for Water Quality Management
- TP-155 Runoff simulation Using Radar Rainfall Data
- TP-156 Status of HEC Next Generation Software Development
- TP-157 Unsteady Flow Model for Forecasting Missouri and Mississippi Rivers
- TP-158 Corps Water Management System (CWMS)
- TP-159 Some History and Hydrology of the Panama Canal
- TP-160 Application of Risk-Based Analysis to Planning Reservoir and Levee Flood Damage Reduction Systems
- TP-161 Corps Water Management System - Capabilities and Implementation Status

