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Hydrologic Engineering Center

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14. ABSTRACT Procedures of a comprehensive computer program that performed an elaborate hydrologic analysis of a system of reservoirs and assigned economic values to the output were described. The objective of the study was to lay a foundation for obtaining objective functions that expressed the socio-economic effects of a system, to provide intermediate hydrologic and economic data and a method of basin the actual design and operation of a hydrologic system on theoretical optimization techniques. The computer program described performed an elaborate and detailed sequential analysis of almost any configuration of water resource projects. System characteristics and requirements could be specified in any desired degree or detail with minimum effort. Output included detailed data on the month to month operation, as well as hydrologic and economic summaries useful for rapid evaluation of system performance.					
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24 May 1968

ECONOMIC EVALUATION OF RESERVOIR SYSTEM ACCOMPLISHMENTS*

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INTRODUCTION

There has been considerable engineering and scientific literature on simulating water resource systems and mathematically optimizing the system design and operation. While a variety of simulation work has been done in planning and design studies, very little actual design has been based on theoretical optimization techniques. One of the reasons is that satisfactory objective functions that express the economic (or preferably the socio-economic) effects of the system in a single figure have not been developed. Work described herein is intended to lay a foundation for obtaining such a function and to provide intermediate hydrologic and economic data required for detailed study.

The increasing complexity of water resources developments and the importance of accurately evaluating system accomplishments require that a detailed sequential analysis of system operation over a long period of time be made. Differences in seasonal variations of runoff and evaporation, and in requirements for storage levels, power generation, and water supplies make the application of simplified storage-yield procedures

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impractical in the analysis of most reservoir systems, particularly where long-term carry-over is concerned.

The procedures described herein are incorporated in a comprehensive computer program that performs an elaborate hydrologic analysis of a system of reservoirs and assigns economic values to the outputs. The procedures include monthly analysis of system operations for all types of water supply and low-flow regulation, and for power generation. Flood control constraints are adhered to, but detailed analysis of flood regulation is not included.

HYDROLOGIC ANALYSIS

The hydrologic analysis for a water resource system such as that illustrated in Figure 1 requires that all of the inflows, requirements, and operating rules be specified, generally as described in the following paragraphs.

The computer program first establishes "local inflows" for all sub-areas above points of interest and below upstream reservoirs. These are computed as linear functions of specified inflows at relevant locations where such flows are available. In this manner, minimum preparation of data is required, and either highly detailed or very general runoff data can be supplied.

Operation controls include the storage and release capacity of each reservoir, power plant capacities, channel and diversion capacities, and

delineation of reservoir operating levels. The operating levels can be specified differently for each month during the year, and differently in different years, if desired. They consist of the full reservoir level, bottom of the flood control space, the top of the buffer space, top of the minimum pool, and intermediate levels between the top of the buffer space and bottom of the flood control space. These intermediate levels are used to control the priority of release among the various reservoirs in the system.

To the extent possible, services that can be provided by more than one reservoir will be provided by that reservoir having storage in the highest range of these intermediate levels. Water is stored in the flood control space only when there is insufficient outlet capacity or channel capacity downstream to release that water. Whenever the buffer pool is not full, only priority requirements are satisfied from that reservoir. Typical specification of reservoir levels is illustrated in Figure 2, which provides that flood-control space in reservoirs 2, 3 and 5 be emptied first, then active storage in reservoir 1, then reservoir 2, then reservoir 3 and half of reservoir 6 simultaneously, then reservoir 5, and then reservoirs 4 and 6 simultaneously. Lastly, buffer storage in reservoirs 2 and 6 is released, but only for priority purposes. The reservoirs can be out of desired balance, of course, if large demands exist that can be satisfied by only certain reservoirs.

Two sets of flow requirements can be specified at each location, total requirements and priority requirements. Each of these can be either constant throughout the year, or different for each month during the year. It is also possible to change requirements at specified locations from year to year.

Requirements for diversion out of the system can be specified as a uniform rate or as a different rate for each month of the year. These can also be changed from year to year, if desired. Such diversions are automatically given priority over river requirements downstream. Diversions into the system can be specified in the same manner. When these constitute return flows, they can be specified as a ratio of some simultaneous diversion upstream.

The net change in evaporation from the reservoir area between conditions with and without the reservoir is specified as a uniform seasonal pattern for all reservoirs. A coefficient by which this pattern is multiplied is specified for each reservoir. The evaporation is applied by the computer to the average area of the reservoir for each period. Since the average stage of the reservoir for any period is not known until the entire system requirements are searched, accurate computation of evaporation requires two system searches for each period, the first to establish average reservoir stages very closely, and the second to compute evaporation accurately.

Hydroelectric power requirements at each power plant are specified by months as total requirement in thousand kilowatt-hours or as a plant factor. Water required to generate that power and the power generated by the water actually released are computed either from the reservoir head and plant efficiency or from a table of power generation versus head for each reservoir. It is also possible to specify an over-all system requirement that exceeds the specified requirements at the individual projects. The excess system requirement is allocated to the various plants in such a way as to most nearly maintain the desired balance of storage within the system. Power computations for each project require two passes through the system during each period, the first to establish average heads for the period, and the second to compute required power releases accurately. System power computation requires three passes through the system each period, the first to determine the amount of excess requirement, if any, and the projects to which the surplus should be allocated, the second to establish average heads at each plant, and the third to compute required releases accurately.

There is also a provision in the program to provide contingency allowances for the fact that the system cannot be operated perfectly. This is done by multiplying the unregulated tributary inflow to any control point by a contingency factor, and considering that such amount does not contribute to the requirement at that location.

Computation of system operation for each period is accomplished by searching the system from upstream to downstream, determining at each control point the total requirements and the reservoirs from which those requirements will be satisfied. Recognizing all of the various controls at each upstream location and the commitments that were set at upstream locations and which must be adhered to, it is apparent that this computation is extremely elaborate. For a system of any size, it is only practical on the fastest and largest computers. A portion of the output for a small sub-system of Figure 1 is shown on Table I.

DETAILED ECONOMIC ANALYSIS

The detailed economic analysis incorporated in this program consists simply of a bookkeeping routine. The output at each control point, which might consist of a rate of river flow, reservoir storage, power generation, or diversion quantity, is assigned an economic function, which can differ from month to month. The function is expressed as a table of the hydrologic quantity versus the economic value in dollars, which is interpolated linearly by the program.

As in the case of hydrologic input, the economic input requirement is designed for maximum convenience. As many as eight output functions can be used. These might represent such items as power, recreation, fish

requirements, navigation, quality control, and diversion for irrigation, municipal or industrial uses. An indicator is specified for each economic function and each control point, which determines whether or not that function is to be evaluated for that control point. An example of economic functions is illustrated in Figure 3.

In the case of economic values associated with river flows, the difference in economic value of such flows with and without upstream projects is of interest. Consequently, at these locations the hydrologic output on tape will include preproject (unregulated) flows at each location for each month. Both the project and preproject flows at these locations are evaluated economically in order to assess the gain (or loss) due to the projects.

At each location and for each month and economic function, the computer will determine the economic value for each item by interpolation, the corresponding economic value of preproject flows, if appropriate, the maximum economic value attainable with any hydrologic quantity, and the difference between this maximum value and the actual value obtained (remaining benefits). These quantities are summarized for the entire period of the operation study as illustrated in Table II.

The contribution of any particular project to over-all benefits of the system can best be assessed by performing a separate operation study without that project. However, there is often a desire to allocate

benefits in some reasonable manner to the projects which created those benefits. For this reason, there is an allocation routine included, which assigns benefits (difference between project and preproject economic values) computed for flows at each location to upstream reservoirs. These benefits are simply allocated in direct proportion to the change in storage at the various upstream reservoirs. When the change in storage at a particular reservoir is opposite in sign to the net change at all upstream reservoirs, an economic contribution of zero is assigned to that particular reservoir. Benefits allocated on this basis are illustrated on Table II.

SHORTAGE INDEX

The above procedure is based on the assumption that benefits or losses occurring during one period are independent of events in preceding periods. In many cases, this assumption is not valid, and the actual interrelationship of benefits is extremely complicated. The recognition of this situation and of the fact that a planning study does not ordinarily reflect the exact quantities that would occur in actual operation, a shortage index useful in planning studies has been suggested.⁽¹⁾ This index consists of the sum of the squares of annual shortages, each expressed as a ratio to the annual requirement, for a 100-year base period.

⁽¹⁾ L. R. Beard, Estimating Long-Term Storage Requirements and Firm Yield of Rivers, XIII General Assembly of IUGG, Berkeley, 1963.

Such a function, when used as an index of economic value, simply assumes that the economic loss associated with a shortage is a function of the square of the total shortage for a year and that the actual operation will be such as to minimize the effects of that shortage through forecasting and spreading the shortage over several months. Although the actual economic function might be more closely related to some other power of the shortage, a quadratic function can usually represent the actual function very closely, given the proper coefficient. Big advantages of adhering to the quadratic function are that the shortage index can be multiplied by a constant to convert the value of shortages to dollars in one simple operation and that the index can be computed independently of the actual economic values. A typical shortage relationship is shown in figure 4. Losses shown are reasonable for agricultural areas. Those for municipal supplies would be far greater.

If a coefficient is assigned to the shortage index at each location and for each function, the shortage indexes that are printed out at the end of the hydrologic computation can be multiplied by these coefficients to establish easily and fairly accurately the economic loss due to the shortages. If this is done, then the basic detailed economic analysis described in the preceding section could ignore shortages by assigning the same value to an inadequate flow as to an adequate flow, thus excluding any penalty for shortages in that computation. The penalty

for shortages could then be assigned on the basis of the shortage index, which appears to be a more reasonable function for this purpose. Furthermore, if there is negligible economic value of surpluses, then the detailed economic computation can be dispensed with, and the shortage index used to subtract a loss quantity from the total value of the firm yields required.

CONCLUSION

The computer program described performs an elaborate and detailed sequential analysis of almost any configuration of water resource projects. System characteristics and requirements can be specified in any desired degree of detail with minimum effort. Output includes detailed data on the month-to-month operation, as well as hydrologic and economic summaries useful for rapid evaluation of system performance.

The procedures for evaluating the economic accomplishments of a water resource system provide a unique expression of benefits and yet provide a detailed break-down of the benefits attributable to the various purposes and to the various projects and to the locations where they occur. Their computation requires a minimum of preparation and is very inexpensive, once that all of the pertinent project features and economic functions have been determined. Benefits can readily be recomputed for changes in any of the project components or economic functions, and are therefore highly useful for cost allocation purposes and for selection of optimum system configuration.

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TABLE I - EXAMPLE OF HYDROLOGIC PRINT-OUT
(Continued)

3 RESERVOIR C		SERVED BY 3						
		SERVING 3 4						
		LOCAL DIVERSIONS 3						
YR 1935	AVG	JAN	FEB	MAR	APR	MAY	JUN	JUL
LOC FLW	418	935	1020	615	870	475	492	75
UNREG	418	935	1020	615	870	475	492	75
INFLOW	418	935	1020	615	870	475	492	75
REQ DIV	127.9	0.	0.	0.	16.0	149.0	317.0	508.0
DIVERSN	127.9	0.	0.	0.	16.0	149.0	317.0	508.0
SHORTGE	0.	0.	0.	0.	0.	0.	0.	0.
EDP STR		10000	58000	84000	109000	117500	117500	86271
EDP EL		728.00	789.47	809.37	824.80	830.00	830.00	810.88
FVAPO	3551	0	0	0	526	519	761	883
CASE		303	303	303	303	303	303	301
LEVEL		1.00	6.00	6.00	6.00	6.00	6.00	5.31
CSV REI	61	61	61	61	61	61	61	61
RIV FLW	285	935	156	192	425	179	162	61
DES FLW	61	61	61	61	61	61	61	61
SHORTGE	0	0	0	0	0	0	0	0
4 CONFLUENCE		SERVED BY 1 2 3						
		LOCAL DIVERSIONS 3 4						
YR 1935	AVG	JAN	FEB	MAR	APR	MAY	JUN	JUL
LOC FLW	-403	562	-1450	419	-1252	-1235	-482	-386
UNREG	2230	5117	3680	3634	4448	2940	2570	769
INFLOW	2034	5117	921	2560	892	317	886	770
REQ DIV	-32.0	0.	0.	0.	-4.0	-37.2	-79.2	-127.0
DIVERSN	-32.0	0.	0.	0.	-4.0	-37.2	-79.2	-127.0
SHORTGE	0.	0.	0.	0.	0.	0.	0.	0.
RIV FLW	2066	5117	921	2560	896	354	966	897
DES FLW	142	100	100	100	150	200	200	200
SHORTGE	0	0	0	0	0	0	0	0
MIN FLW	100	100	100	100	100	100	100	100
SHORTGE	0	0	0	0	0	0	0	0

TABLE II - EXAMPLE OF BENEFITS PRINT-OUT

AVERAGE ANNUAL BENEFITS IN THOUSAND DOLLARS

NET BENEFITS GAINED (UNALLOCATED)

STA	SUM	FUNCTION					
		1	2	3	4	5	6
1	301	0	0	136	164	0	0
2	159	0	0	171	-12	0	0
3	381	0	-15	52	0	344	0
4	68	68	0	0	0	0	0
SM	910	68	-15	360	152	344	0

NET BENEFITS GAINED (ALLOCATED)

STA	SUM	FUNCTION					
		1	2	3	4	5	6
1	354	52	1	136	164	0	0
2	237	76	2	171	-12	0	0
3	369	-15	-12	52	0	344	0
4	-51	-46	-5	0	0	0	0
SM	910	68	-15	360	152	344	0

GROSS BENEFITS

STA	SUM	FUNCTION					
		1	2	3	4	5	6
1	301	0	0	136	164	0	0
2	159	0	0	171	-12	0	0
3	535	0	138	52	0	344	0
4	779	299	480	0	0	0	0
SM	1773	299	618	360	152	344	0

POTENTIAL GROSS BENEFITS

STA	SUM	FUNCTION					
		1	2	3	4	5	6
1	538	0	0	250	288	0	0
2	1502	0	0	350	1152	0	0
3	769	0	240	150	0	379	0
4	1080	600	480	0	0	0	0
SM	3889	600	720	750	1440	379	0

REMAING BENEFITS

STA	SUM	FUNCTION					
		1	2	3	4	5	6
1	237	0	0	114	124	0	0
2	1343	0	0	179	1164	0	0
3	234	0	102	98	0	35	0
4	301	301	0	0	0	0	0
SM	2116	301	102	390	1288	35	0

LEGEND:

-] Control point with reservoir
- Power plant
- Control point with diversion

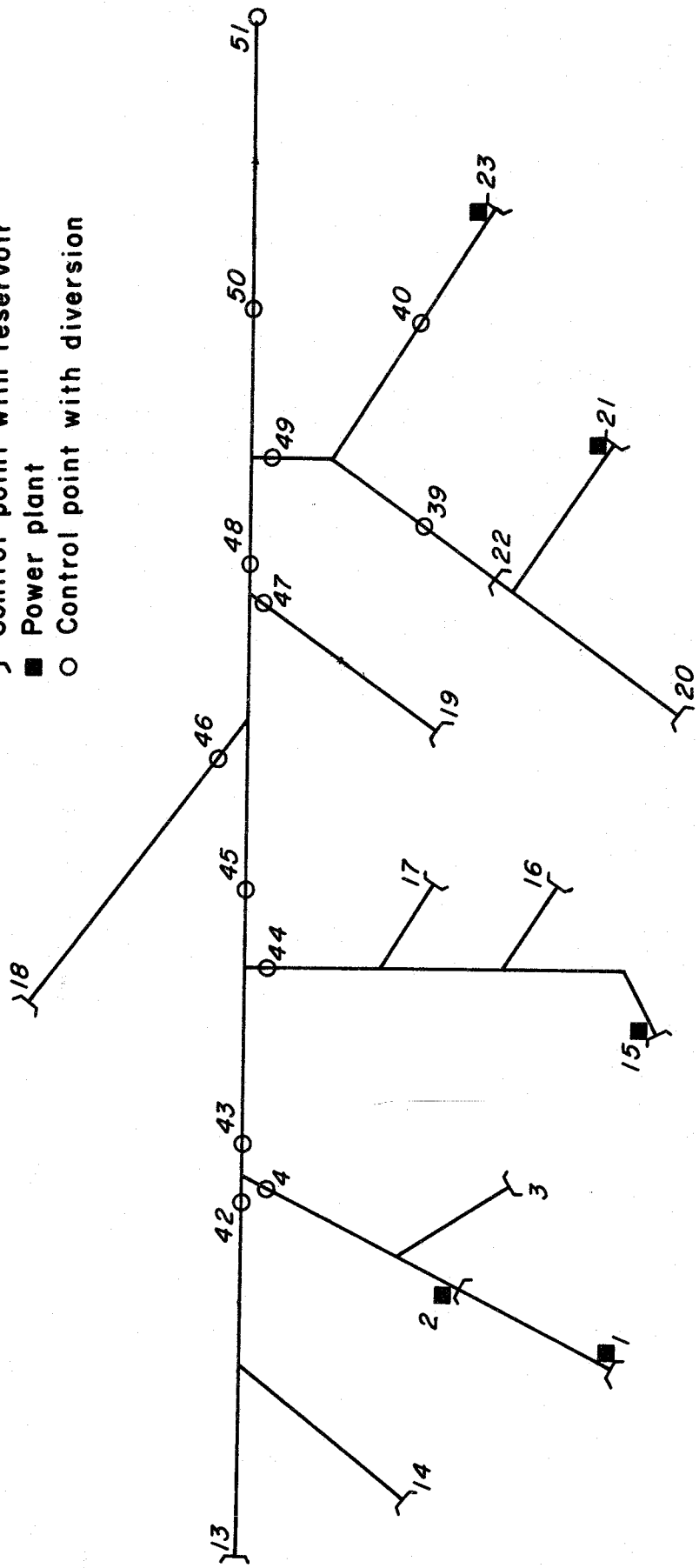


Figure 1
WATER RESOURCES SYSTEM
SCHEMATIC DIAGRAM

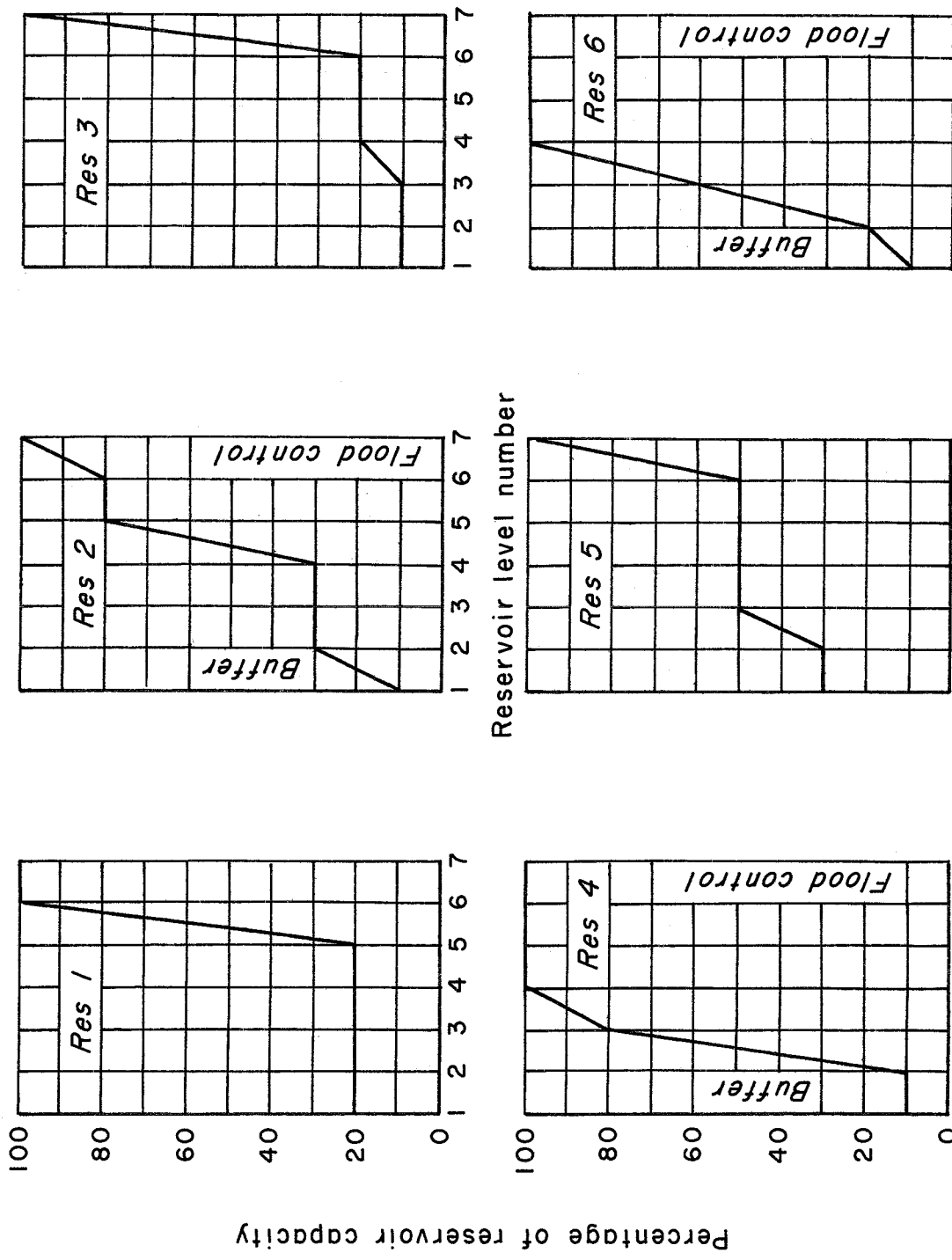


Figure 2
ILLUSTRATIVE PRIORITY OF RESERVOIR USE

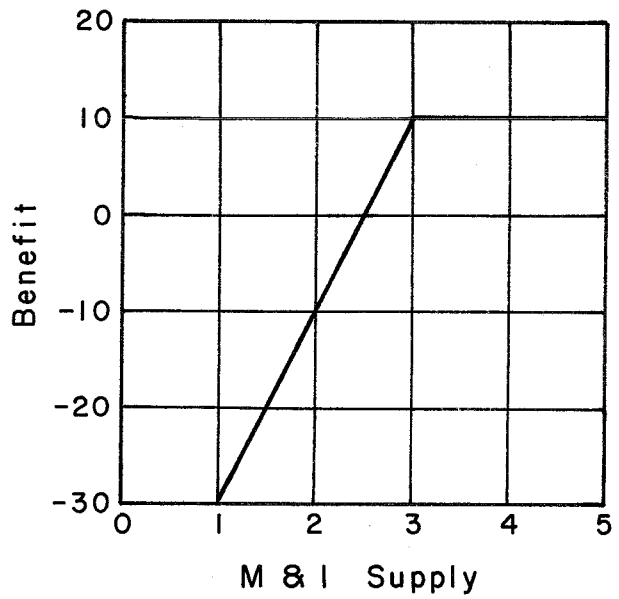
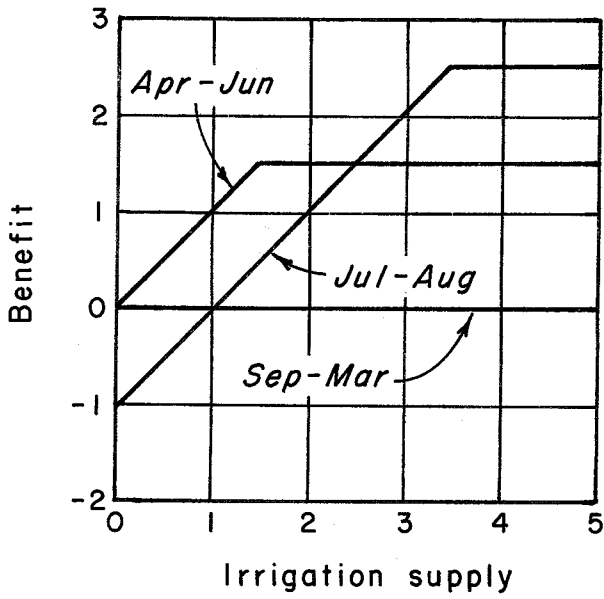
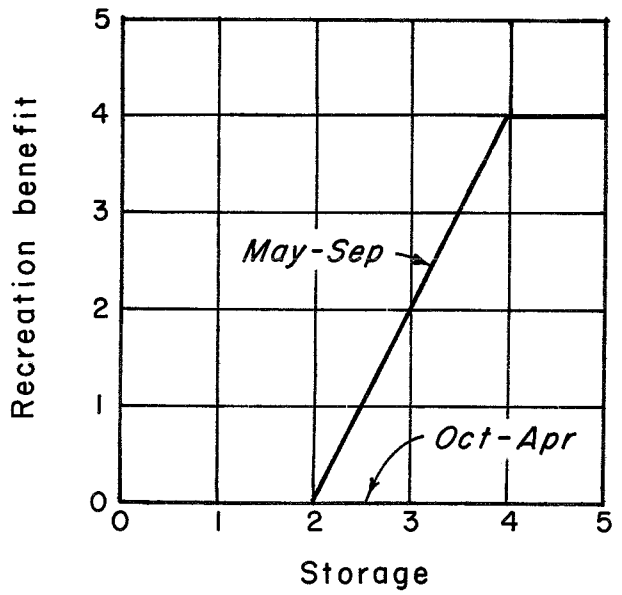
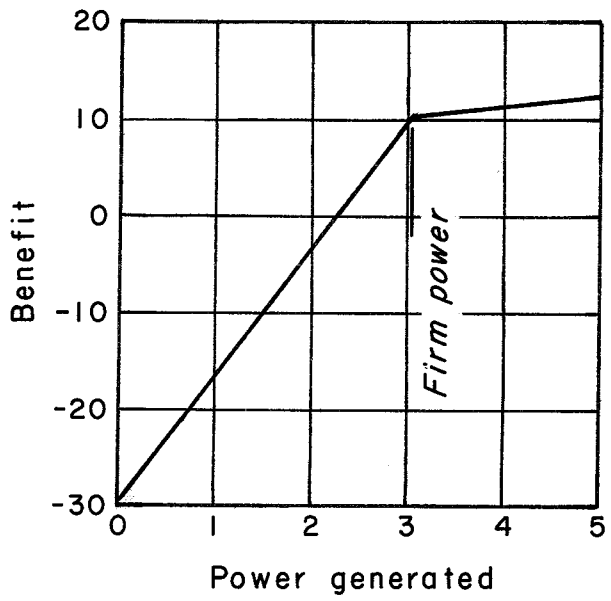


Figure 3

TYPICAL SIMPLIFIED BENEFIT FUNCTIONS

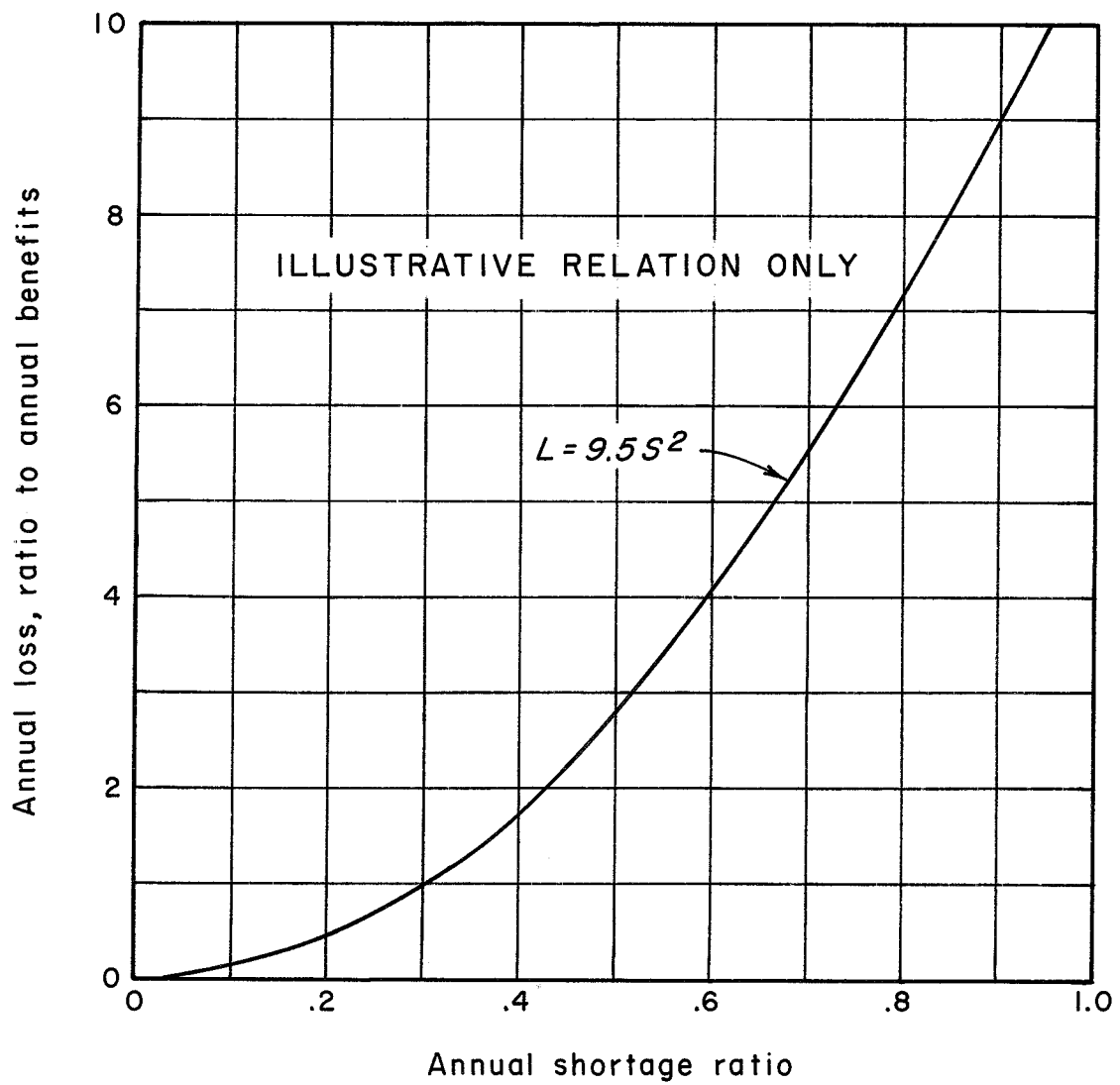


Figure 4

ILLUSTRATIVE SHORTAGE LOSS RELATION

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