Bridge Modeling

Mark Jensen, M.S. Alex Sanchez, Ph.D, Cameron Ackerman, P.E., D.WRE

USACE, Institute for Water Resources, Hydrologic Engineering Center

1



US Army Corps of Engineers ®





Introduction

- Two Approaches for Bridge Modeling
 - Simplified 1D/2D Bridge Modeling
 - Enforces precomputed 1D bridge curves from a nested 1D bridge model
 - Can handle any flow regime
 - Cannot simulate detailed flow
 - Terrain Modifications not necessary
 - Full 2D Modeling
 - Detailed mesh and terrain modifications used to simulate detailed flow through bridge
 - Currently, cannot handle pressured flow or overtopping



Simplified 1D/2D Bridge Hydraulics



- Utilizing existing HEC-RAS 1D Bridge Hydraulics methods inside of a 2D Flow Area
- Model complete range of Bridge Hydraulic flow regimes
 - Low flow
 - Pressure flow
 - Pressure flow and weir flow (road over topping)
 - Low flow and weir flow





Approach

- Use HEC-RAS 1D Bridge Geometry and Cross Section Layout (automated layout) inside of 2D Flow Area
- Develop a family of Headwater/Tailwater Flow curves from RAS 1D bridge hydraulic calculations during geometry pre-processing
- Uses a special momentum equation that gets applied only at the 2D bridge centerline faces:
 - Friction loss, pressure differential, and convective acceleration forces are equated from the water surface difference in the bridge curves.
 - The forces are distributed across faces though the bridge.
 - Local acceleration is calculated on the fly, as it is not a force in curves
- Bridge faces are then solved in 2D just like all other faces
- Extremely efficient almost no increase in computations





- 1D Bridge loses computed between "XS" downstream of deck through the bridge to "XS" upstream of bridge deck.
- Enforced as Breakline
- Set cells sizes (or cell US/DS length) so that the cell centers match the domain covered by 1D Bridge loses
- Model Bridge opening (not overbanks)







- Cell Size Considerations
 - Bridge Deck width = 40 ft and US Distance = 5 feet
 - Half deck width = 40/2 = 20 ft
 - Distance from centerline to first cell center = 20 + 5 ft
 - Row of cells needs to be 50 ft







Steps for Adding a 2D Bridge

- 1) Draw a centerline for the bridge opening/embankment using the SA/2D Area Connection tool
 - Bridge centerline is drawn left to right looking downstream.
- 2) Develop an appropriate mesh (cell size and orientation) for the bridge
 - Use the structure mesh controls (cell size and enforcement). Some hand editing may be required depending on the bridge and what else is near the bridge (i.e. levee, another bridge, railroad tracks, road, etc...)
- 3) Enter the bridge data
 - Deck/roadway; distance from upstream bridge deck to outside cross section piers; abutments; bridge modeling approach; Manning's n values for the 1D bridge cross sections; and hydraulic tables controls (HTAB) into the SA/2D Area Connection editor.
- 4) Pre-process the geometry in order to create the bridge curves.
 - Review the bridge family of rating curves for hydraulic accuracy.
- 5) Run the model and review the results.
 - Make any necessary changes to the data in order to improve the results.



Bridge Centerline and Mesh Adjustments

- Select the SA/2D Area Connection layer tool and Draw the centerline of the bridge from left to right looking downstream
- Choose Bridge Type





Bridge Centerline and Mesh Adjustments

- Edit 2D Connection Breakline Properties
 - Cell Spacing
 - Near Repeats
- Cell Spacing =deck width + u/s distance
- Enforce 2D Connection as Breakline





🜃 Bridge Data



- Open in Geometric Data Editor
- Structure Type: Bridge





Ĭ

Deck and Roadway

Deck/ Roadway
\sim

- Distance (this is the distance from the upstream side of the bridge deck to the cross section upstream outside of the bridge, as wells as downstream outside)
- Width of the bridge deck in the direction of flow
- Weir Coefficient for flow going over the roadway
- Station (distance from left to right along the bridge deck/roadway), High Chord, and Low Chord elevations for the upstream and downstream side of the bridge deck

	Distanc	e	Wid	lth	W	Weir Coef										
		4	0.		2.6	2.6										
Cle	ear D	el Row	ns Row		Сор	y US to DS										
	L.	lpstream			Downstrea	am										
	Station	high chord	low chord	Station	high chord	low chord										
1	40	950	930	40	950	930										
2	47	950	939	47	950	939										
3	50	950	941.175	50	950	941.175										
4	53	950	942.45	53	950	942.45										
5	56	950	943.2	56	950	943.2 943.8										
6	59	950	943.8	59	950											
7	62	950	944.25	62	950	944.25										
8	65	950	944.55	65	950	944.55										
S I Ve Iax (•	Embankmer ir Data C Submerge <u>ir Crest Sha</u> Broad Cre	nt SS 0 ence: 0.1 ape sted	95	D.S Er Min W	nbankment S eir Flow El:	s o										
					ок	Cancel										



Automated Cross Sections

 HEC-RAS will automatically create the four needed cross sections for preprocessing the bridge hydraulics into a family of curves.

ĬŀŤĬĬ

- 1. Upstream just outside the bridge deck, normally at the toe of the upstream embankment. This cross section is automatically generated upstream of the bridge deck based on the user entered Distance field.
- 2. Upstream just inside the bridge deck/roadway.
- 3. Downstream just inside the bridge deck/roadway.
- 4. Downstream outside of the bridge deck/roadway, normally at the toe of the downstream embankment. This cross section is automatically generated downstream of the bridge deck a distance equal to what the user entered for the upstream **Distance** field.





Piers and Abutments

Hreff

- A Centerline Station for both the u/s and d/s for bridge pier.
- The pier is formed by entering pairs of elevations vs widths, starting below the ground and going up past the low chord of the bridge deck.
- This must be done for both the u/s and d/s side of bridge
- Copy Up to Down, if appropriate

A	dd Copy	Delete	Pier #	<u> </u>
De	Row Ce	nter <mark>line S</mark> tatio	n Upstream	110
In	s Row Ce	nterline Static	n Downstream	n 110
			THE REPORT OF TH	
	Uncha	Deb	ris Height:	
	Upstre	Deb	ris Height: Dow	vnstream
	Upstre Pier Width	am Elevation	ris Height: Dow Pier Width	Instream Elevation
1	Upstre Pier Width 6	Deb am Elevation 925	ris Height: Dow Pier Width 6	vinstream Elevation 925
1	Upstre Pier Width 6	Deb am Elevation 925 945	ris Height: Dow Pier Width 6 6	vnstream Elevation 925 945
1 2 3	Upstre Pier Width 6 6	Elevation 925 945	ris Height: Dow Pier Width 6 6	Elevation 925 945
1 2 3 4	Upstre Pier Width 6	Elevation 925 945	ris Height: Dow Pier Width 6 6	Elevation 925 945
1 2 3 4 5	Upstre Pier Width 6	Elevation 925 945	ris Height: Dow Pier Width 6 6	Elevation 925 945
1 2 3 4 5	Upstre Pier Width 6 6	Deb Deb am 925 945	Pier Width 6 6	vinstream Elevation 925 945





- For the **two cross sections inside the bridge**, they follow the edges of the deck/roadway.
- The outside cross section locations are automated by simply creating cross sections that are parallel to the inside cross sections, and the distance upstream and downstream from the bridge deck is based on what the user entered for the Distance field in the Deck/Roadway editor.
- Note: If the user changes the Deck/roadway data (Bridge width or distance field), then the location of the 1D cross sections will change.
- The user can view each of the four cross sections, as well as the centerline from the **plot by selecting the** location to view.
- If the bridge data is changes, the user will see that the terrain under the current 1D cross section line is different than what they currently have entered. The user can recut any of the 1D cross sections by simply pressing the **Cut profile from terrain button** while viewing a specific cross section.







Manning's n Data for the XS's

• Options | External and Internal Bridge Cross Sections...

	Up	stream Outs	ide		U	pstream Insid	le			Do	wnstream Ins	ide		Downstream Outside											
	Main Ch	hannel Bank S	Stations		Main Ch	annel Bank S	tations			Main Ch	nannel Bank S	Stations			Main C	hannel Bank	Stations								
	Left Bank	Sta Righ	nt Bank Sta		Left Bank	Sta Right	t Bank Sta			Left Bank	Sta Righ	it Bank Sta			Left Bank	Sta Rig	ht Bank Sta								
þ		348.9	96	0		348.96	5		0		348.9	6		0	0	348.9	96								
	Cross Se	ction X-Y Co	ordinates		Cross Se	ection X-Y Co	ordinates			Cross Se	ction X-Y Co	ordinates		1	Cross Se	ction X-Y Coo	ordinates								
	Station	Elevation	Mann n		Station	Elevation	Mann n	-		Station	Elevation	Mann n	-		Station	Elevation	Mann n								
1	0	948.51	0.035	1	0	948.61	0.035		1	0	947.96	0.035		1	0	947.78	0.035								
2	7.3	949.17		2	3.37	948.91			2	0.63	948			2	3.85	948									
3	8.8	949.27		3	10.53	949.31			3	6.58	948.66			3	9.8	948.66									
4	12.42	949.33		4	11.34	949.33			4	12.52	948.97			4	15.75	948.97									
5	16.03	949.55		5	12.59	949.42			5	18.47	949.31			5	21.7	949.31									
6	19.65	949.64		6	16.48	949.62			6	24.42	949.62			6	27.65	949.62									
7	23.27	949.91		7	18.57	949.69			7	30.37	949.97			7	33.6	949.97									
8	25.15	949.97		8	22.19	949.96			8	31.64	950			8	39.55	950.28									
9	29.57	950.2		9	25.81	950.04			9	33.61	950.14			9	45.5	950.62									
10	30.5	950.27		10	29.42	950.25			10	36.32	950.28			10	50.75	935.55									
11	34.12	950.37		11	33.04	950.32			11	38.88	950.35			11	56.7	934.53									
12	37.05	950.56		12	36.66	950.56			12	42.49	950.62			12	59.64	934.17									
13	38.15	947.49		13	40.28	950.62			13	48.22	950.62			13	62.65	933.86									
14	42.3	935.27		14	45.53	934.82			14	50.63	943.49			14	68.6	933.29									
15	44.91	934.73		1 15	49.48	934.14		-	15	53.47	934.89		-	15	72.32	932.95									
10	45 04	024.24		1 1 10	F4 47	000 05			40	FF 34	024 5		- I	1. 10	75 04	000 61									



1D Bridge Modeling Approach

Bridge Modeling Approach

- Low flow methods:
 - Energy
 - Momentum
 - Yarnell
- High flow methods:
 - Energy
 - Pressure/Weir flow



HEC



Hydraulic Table Parameters

- Number of points on free flow curve (100 max)
- Number of submerged curves (60 max)
- Number of points on each submerged curve (60 max)
- Headwater maximum elevation
- Tailwater maximum elevation is optional, as is the Maximum Flow.

HTab Param.



Hydraulic Table Parameters



- Number of points on free flow curve (maximum is 100)
- Number of submerged curves (maximum is 60)
- Number of points on each submerged curve (maximum is 60)
- Head water maximum elevation.
- Tail water maximum **elevation** is optional, as is the Maximum Flow.

Connection Hydraulic	Property Table Para	meters
Number of points on free	flow curve:	100
Number of submerged cu	rves:	60
Number of points on each	n submerged curves:	60
Apply number of	points to all Connect	ions
Head water maximum ele	vation:	595
Tail water <mark>m</mark> aximum eleva	ation (Optional):	
Maximum Flow (Recomme	ended):	1
	ок 1	Cancel

18









1D vs 2D Model Comparison





ID and 2D Comparison Animation











Computation Procedure

- Current (last time step or last iteration) DS XS water surface and flow are used to compute the US XS water surface
- Matrix Coefficients for US XS row are changed to impose this computed US water surface





Family of Rating Curves - 2D Flow Areas

- Curves are used differently from 1D solution
- Compute US XS and DS XS are used to compute expected 1D Flow
- Friction lose terms are adjusted so that compute 2D flows match expected 1D Flow (one adjustment factor for all the bridge faces)





• Stage and Flow Plot





Bridge Output









- Spacing/cell sizes through a bridge modeled as 1D are generally larger than users would like.
- Necessary to use these cell sizes so that we do not duplicate contraction and expansion losses (however small they may be) in 2D domain and in 1D curves.



Detailed Bridge Modeling

- Simulating all energy loses with a high-resolution 2D model
- Requirements high-resolution terrain and mesh
- Pros
 - Physics-based
 - High-resolution results
- Cons
 - Depth-averaged assumption
 - No pressurized flow or overtopping in Versions 6.3 and earlier
 - Computationally Expensive









Terrain for Detailed Bridge Modeling

- Requirements
 - High-resolution
 - High-quality
- Terrain Modifications
 - Piers
 - Embankments
 - Bathymetry Modifications







Diffusion Wave Equation

- For preliminary runs and/or initialization
- Cannot simulate contraction and expansions
- Cannot simulate mixed flow

Shallow Water Equations

- Production runs
- Necessary to include Turbulence
- SWE-EM has better momentum conservation

HEC-RAS Unsteady Computation Options and Tolerances General 2D Flow Options 1D/2D Options Advanced Time Step Control 1D Mixed Flow Options Use Coriolis Effects (not used with Diffusion Wave equation) (Default) Parameter Perimeter 1 1 Theta (0.5-1.0) 1 2 Theta Warmup (0.5-1.0) 1 3 Water Surface Tolerance [max=0.2](ft) 0.01 0.01 4 Volume Tolerance (ft) 0.01 0.01 5 Maximum Iterations 20 20 6 Equation Set Diffusion Wave SWE-ELM (original/faster) N 7 Initial Conditions Time (hrs) Diffusion Wave 8 Initial Conditions Ramp Up Fraction (0-1) 0.1 VE-FIM SWE-EM (stricter momentum) 9 Number of Time Slices (Integer Value) 1 10 Turbulence Model None None 11 Longitudinal Mixing Coefficient 0.3 0 ... 12 Transverse Mixing Coefficient 0.1 0.1 13 Smagorinsky Coefficient 0.05 14 Boundary Condition Volume Check 15 Latitude for Coriolis (-90 to 90) 16 Solver Cores All Available 4 Cores 17 Matrix Solver PARDISO (Direct) PARDISO (Direct) 18 Convergence Tolerance 19 Minimum Iterations 20 Maximum Iterations 21 Restart Iteration 10 22 Relaxation Factor 1.3 1.3 23 SOR Preconditioner Iterations 10

Cancel De

Defaults .

OK



Computational Mesh

Ĭ

- Preliminary runs with simple coarse mesh
- Breaklines and refinement regions for high-resolution meshes
 - Minimum cell size will drive time step
- Perform grid convergence





Turbulence Modeling

- Turbulence Model
 - Use the Conservative Formulation!
- Eddy Viscosity Model
 - Use Parabolic-Smagorisnky
 - Calibrate if possible
 - In lieu of calibration, perform sensitivity analysis







- Use spatially variable Manning's n
- Different flows may require different bottom roughness





Boundary Conditions

- Tempting to place them very close to the bridge but don't
- Perform sensitivity on boundary placement and boundary values (such as friction slope)
- Place boundaries in areas with 1D flow (i.e. no recirculation, or sharp contracts and expansions)





Advanced Computational Parameters and Options

- Time Step
 - Variable time step algorithm
 - Hand calculations of Courant
 - Model stability
- Implicit Weighting Factor
 - 1.0 for preliminary runs
 - 0.5-0.6 for production unsteady flow runs
 - Not important for steady flow



HE



Limitations

- Flow must be below bridge deck
 - No pressured flow
 - No flow overtopping
- 2D Flow Assumption
 - Breaks down when cell resolution is much finer than depth
- Hydrostatic Pressure Assumption
- Cannot simulate
 - Bridge openings such as culverts
 - Debris (can be done in 1D)
 - Ice (can be done in 1D)
 - Waves





Detailed Bridge Modeling





Detailed Bridge Modeling

		- 64	•						5			10					1.0		s in	1					es i	i e	1.0	1000	8	1		100		-		άř.					31		i io			0.1					-		
		•				•					- 13		٠	•						•		•	•	•													۲	0.0	1	0	٠		i.			5		6	50				
						•					, Ê			•																																							
		5						100					•								•		•						•	•																			5				
										. 18																					. 2																						
																																															-						
										8																		100																	1000				6				
						Pro I								븕			4												ewe:	246	5.466	-		Percent state	MIL		36				5.47				10	1-49	Set 1				-	<u>844 B</u>	
-																																			-	-															-		
													-	-							-																																
							P10 107		anne G											H.																																	
																														-				-										-						-			-
		•	•	•	•		• •				• •					•															2			•	•															•		•	•
	•		•	• •	•									•		•	4		•							46		•		•			•		•	181	•	9		9		• •		•		9				•	•		•
	•	•	•	• •	•		• •			•	•					•			•							26				•		30			•					•		• •		•		•				•	•	•	•
•	•	•	•	• •	•		• •		•	•	• •			9	٠	•	30		•					•	•	36		•		•	•		•	•	•		•	•		•	2	• •		•		2	•	•	2	•	•	•	•
٠	•	2	•	• •	•		• •			•	• •		•		•	• •	3		•			•	2	•		21				•		•			•		•	•		•			•	٠				•		•		•	•
	•	•	•	• •	•		• •			•	•					•			•													20			•							• •	•	•		٠			30	•	•		•
٠		٠	•	•		•	• •	•	٠	•	• •				٠	•	2					•	•	•	•	32		•	٠	٠			٠	•	•	100	٠					• •		٠			•	•			٠	•	•
٠	•	•	•	•		•	• •			•	• •		•	•	•			1	•						•			٠		•	•	•		•	•		•	•		•	•	• •		•									•
٠			•	•	٠		•		۲	•			•	9		•			•		•		•	•		20				•					•					•		• •		٠			•	•	32	•		•	•
٠		•	•	• •	•	•	• •			•	• •				•	•		•	•					•	•	2		•		•	•		•		•		•	•				• •					•	•	38	•	•	•	•
٠	•	۲	•	•		•	• •			•	• •		•	Q	•	•		1						•		35				•					•							• •	1				•	•	30				•
۲	Ø		•	• •		•	• •										20								30	35								•	•					D		•				01		•	30	٠	٠	•	
٠			•	• •	•	•	• •	•		•	• •	•	٠		•	•			•			•		•	•	3		٠	٠	•				•	•		•					• •		•	•		•	•	1	•	٠	•	•
٠		•	•	• •		•	• •	•		•	• •		•		•	•						•	•	•	•			•		•	• •	•	•	•	•		•	•	• •	•		• •	•	•	•	•		•			•		
	•	•	•	•			• •		٠					Ð	•	•			•			•	•	•		3 C			۲		•		•	•	•	• •	•			D	٠	• •				0		•		•	٠	•	
٠	•		•	• •						•			•		٠	•	20	•		•		•			•	X					•		•	•			•			•	•		•								•	•	
		•	•	• •		•				•	• •			0	•	•								•		38		•		•	•		•		•			•		•		• •		•				•			•		•
۲	0	•	•	• •	•	•	• •	•	•	•			٠	•	•	•	5			•	۲		•		•	1		•	۰	•	•		•	•	•	• •	٠	•		•	•	• •						•		•	•	•	
	•	•	•	• •		•							•			•					•							٠	•	•	•		٠	•				•								•					•		
		•	•			•		•		• •			•	•		•						•			• •	2 2				•	• •		•	•	•		•	• •							•	•		•			•		
•	•	•	•			-		•					•	•									•							•				•		100	•	•	1	•		• •				•			5			•	
		•																							-					•		1	•	•			•			•								•			•	•	
																	۰.													-	Ξ.													•						•	•		
•																				-	•	•				37																			•								
						1										•						100																							1999g								
																										밀물																			1000					1.		-	
																			-			•																															-
				28 DM 28 DM				1990 1990	NAME OF						23		명양					199			550 bi Filip											10 2.4 12 970		10 di 2							100			130 Å	영 (이				

38











Detailed Bridge Animation









Questions?



US Army Corps of Engineers ®

42