

LEGO Dam Breach 2 - A Lego House and Bikers Flooded - Full Video

Camera 1 ↗



<https://youtu.be/E64q4SAiHjg?t=28>



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LEGO Dam Breach #183 - Construction Crew Surprise
11K views • 2 months ago

LEGO Dam Breach #182 - Attack of the C-3POs
37K views • 3 months ago

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52K views • 5 months ago

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LEGO Dam Breach #166 - Sand Castle Collapse
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11K views • 6 months ago



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6:09

LEGO DAM BREACH AND SAND CASTLE - TOTAL...

6.8M views • 5 months ago



6:09

LEGO DAM BREACH - HARD FLOOD!

6.3M views • 2 years ago



4:26

LEGO DAM BREACH - NEW SAND CITY COLLAPSE

6M views • 3 months ago



7:12

New Biggest Rebellion Of LEGO Minifigures Against...

5M views • 1 year ago



4:27

LEGO CITY FIRE RESCUE. LEGO DAM BREACH FILM

3.9M views • 1 year ago



3:35

CAN 30 LEGO MINIFIGURES STOP A LEGO DAM BREAC...

3.8M views • 1 year ago



5:26

Epic Attempt Of The Lego Minifigures To Stop The Leg...

3.4M views • 1 year ago



5:39

LEGO Dam Breach: LEGO City Explore New Big Sand Castle!

2.6M views • 2 years ago



3:15

Lego Adventurers - Destroyed Pyramids. Dam Breach Film

2.4M views • 1 year ago



6:31

LEGO Dam Breach - LEGO Minifigures and Sand Castle...

2.3M views • 1 year ago



10:12

LEGO DAM BREACH - LEGO CITY POLICE TRUCK - TWO...

2.3M views • 1 year ago



5:31

LEGO BEACH - DAM BREACH

2.2M views • 2 years ago



5:44

Rebellion Of LEGO Minifigures Against LEGO...

1.9M views • 2 years ago



5:06

LEGO DAM BREACH - MINE FLOODING AND COLLAPSE

1.8M views • 4 months ago



4:26

LEGO DAM BURST, FLOODING SECRET...

1.8M views • 2 years ago



5:32

LEGO DAM BREACH - LEGO CITY FIRE RESCUE

1.8M views • 9 months ago



4:39

LEGO SAND WALL BREAK

1.7M views • 2 years ago



5:42

Lego Dam Breach Airport

1.7M views • 1 year ago



4:18

LEGO DAM BREACH - DESTRUCTION OF THE SAN...

1.3M views • 3 months ago



3:45

Dam Breach - LEGO Construction Site

1.2M views • 2 years ago



10:04

LEGO DAM BREACH - BRIDGE COLLAPSE

1.2M views • 1 year ago



4:16

LEGO Dam Breach - Dinosaur Attack LEGO Minifigures on...

1M views • 1 year ago



4:38

LEGO Dam Breach SAND CASTLE

1M views • 2 years ago



4:12

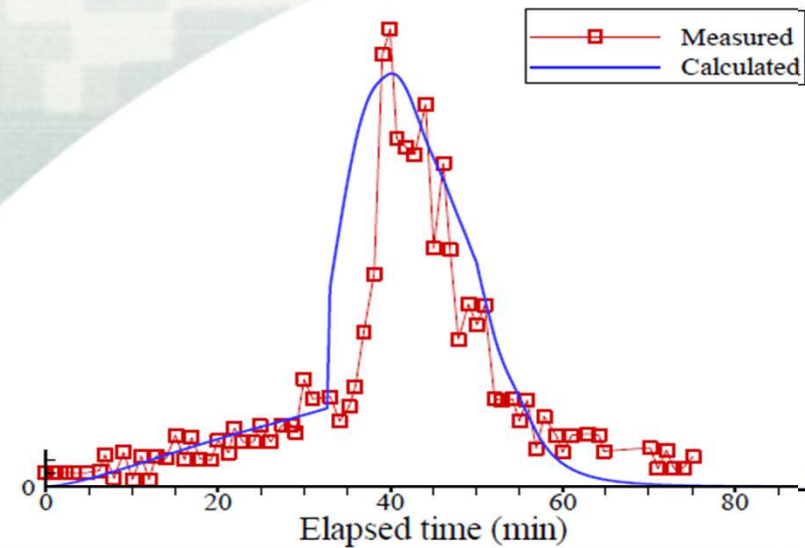
LEGO DAM BREACH - CITY COLLAPSE

942K views • 4 months ago

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Estimating Dam Breach Parameters

Stanford Gibson, PhD
Sediment Specialist
Hydrologic Engineering Center



NRCS Embankment Failure Research (SIMBA/WinDAMB model)



Temple, Darrel M., and Hanson, Gregory J., "Earth Dam Overtopping and Breach Outflow," Presented at the ASCE World Water & Environmental Resources Congress 2005, EWRI, Anchorage, AK, 15-19 May 2005.



Hadlock Pond Dam New York

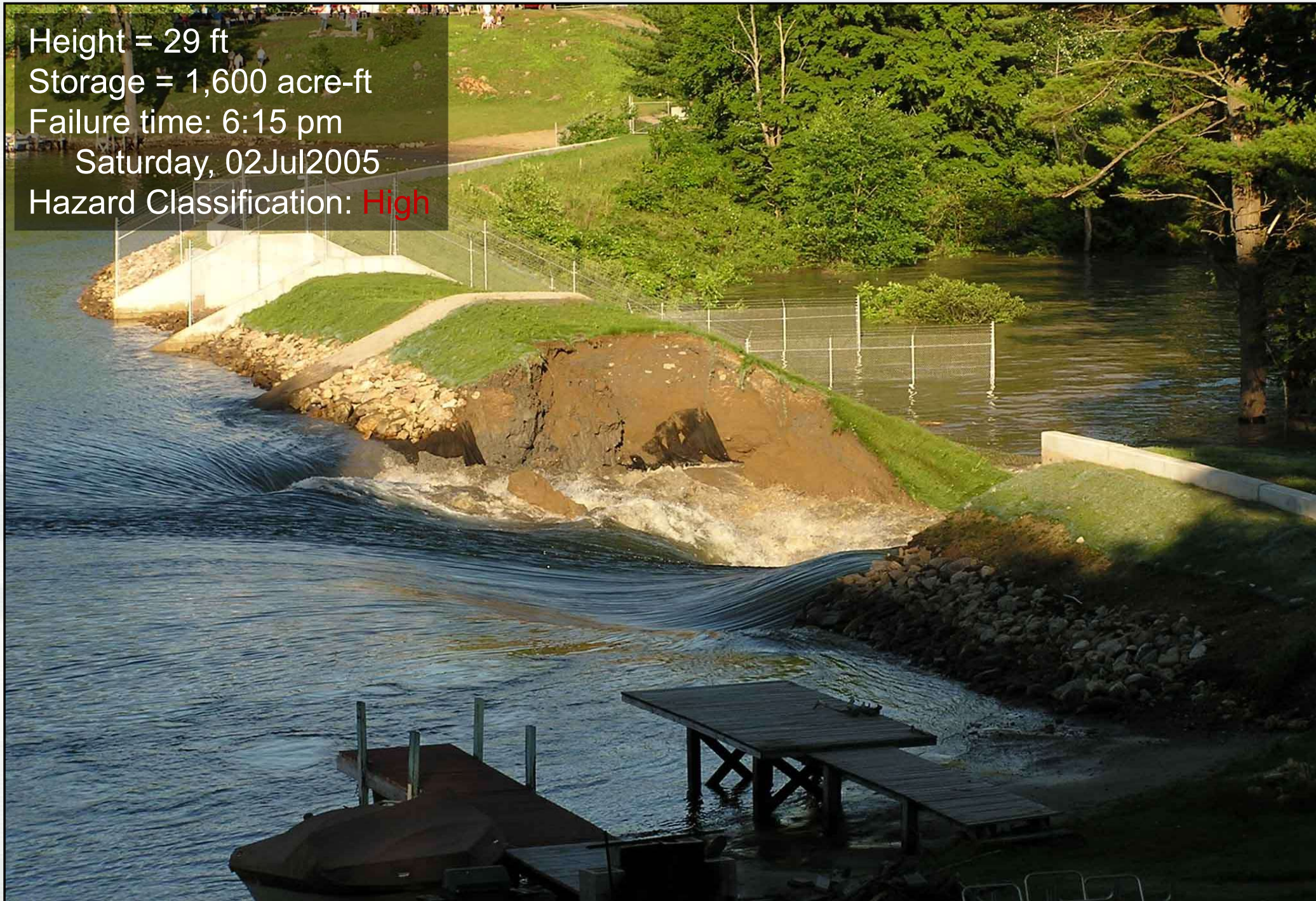
Height = 29 ft

Storage = 1,600 acre-ft

Failure time: 6:15 pm

Saturday, 02Jul2005

Hazard Classification: **High**



Taum Sauk Upper Dam, Missouri

Height = 94 ft
Storage = 1,600 acre-ft
Failure time: 5:20 am
Wednesday, 14Dec2005
Hazard Classification: **High**

Dunlap Dam Failure

Guadalupe Blanco River Authority May 15, 2019 1



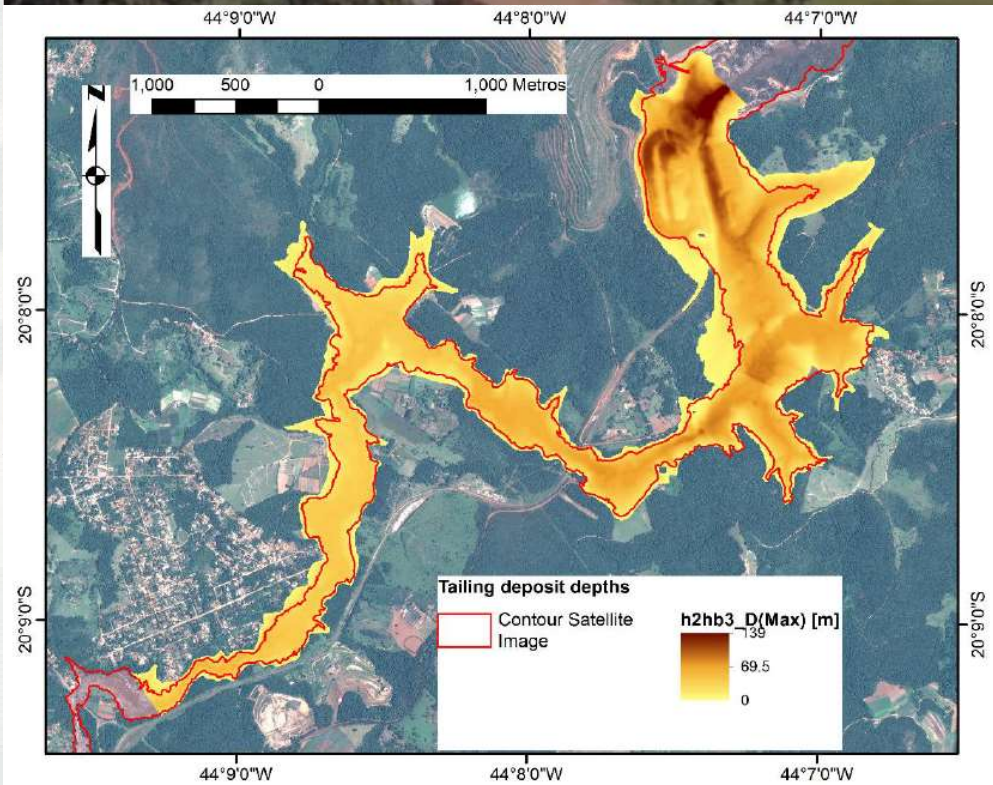
http://herald-zeitung.com/video_e9e0f25c-7756-11e9-b1cc-3f3ca4a52872.html



Brazil Mine Tailings Failure

Modeling by Prof Leonardo Moura

University of Brasilia



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Michigan dam failure caught on video

https://youtu.be/Hc3u_CHVHJ8

2,913,549 views • May 20, 2020

 10K  1.2K  SHARE  SAVE

Three Approaches

1. User Entered Data
-Parameter Estimation

2. Simplified Physical

3. DL Breach

User Entered Data
Simplified Physical
Physical Breaching (DLBreach)



Three Approaches

1. User Entered Data

-Parameter Estimation

User Entered Data



2. Simplified Physical

Simplified Physical



3. DL Breach

Physical Breaching (DLBreach)



User Entered Data

Simplified Physical

Physical Breaching (DLBreach)



HEC-RAS Breach Data

Dam (Inline Structure) Breach Data

Inline Structure: Bald Eagle Cr. Lock Haven 81454

Breach This Structure

Breach Method: User Entered Data

Center Station: 5250

Final Bottom Width: 446

Final Bottom Elevation: 585

Left Side Slope: 0.9

Right Side Slope: 0.9

Breach Weir Coef: 2.6

Breach Formation Time (hrs): 3.2

Failure Mode: Piping

Piping Coefficient: 0.5

Initial Piping Elev: 620

Trigger Failure at: WS Elev

Starting WS: 676.8

Breach Plot | Breach Progression | Simplified Physical | Physical Breaching (DLBreach) | Parameter Calculator | Breach Repair (optional)

User Entered Data

Simplified Physical
Physical Breaching (DLBreach)

Legend

- Ground
- Bank Sta
- Final Breach

Elevation (ft)

Station (ft)

OK Cancel

Which of these parameters are hard?

HEC-RAS Breach Data Input

- **Location:** Centerline of breach
- **Type:** Overtopping or Piping
- **Size:** Bottom elevation, **width** and side slopes
- **Time:** to maximum size and progression type
- **Triggering situation:**
 - ✓ Pool elevation
 - ✓ Pool elevation + Duration
 - ✓ Clock time



Before we get into the tough parameters...
Let's take on some of these others.

Dam (Inline Structure) Breach Data

Inline Structure **Bald Eagle Cr. Lock Haven**

Breach This Structure

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User Entered Data

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But First...A Couple Other Features

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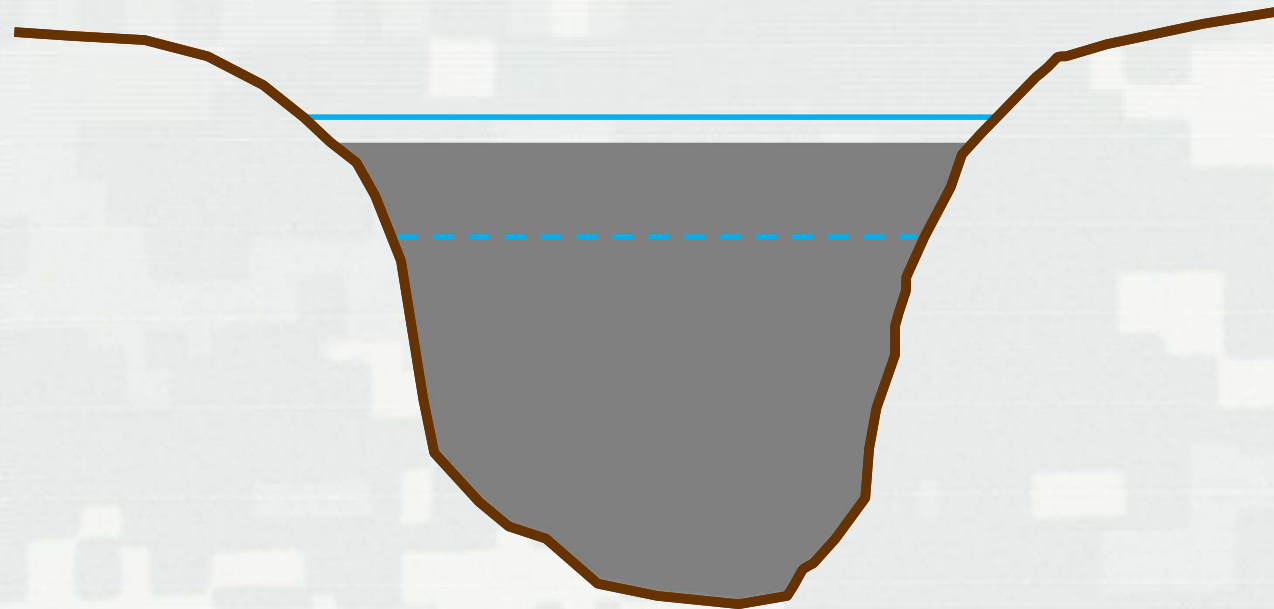
Failure Mode: Piping

Piping Coefficient: 0.5

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Trigger Failure at: WS Elev

Starting WS 676.8



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But First...A Couple Other Features

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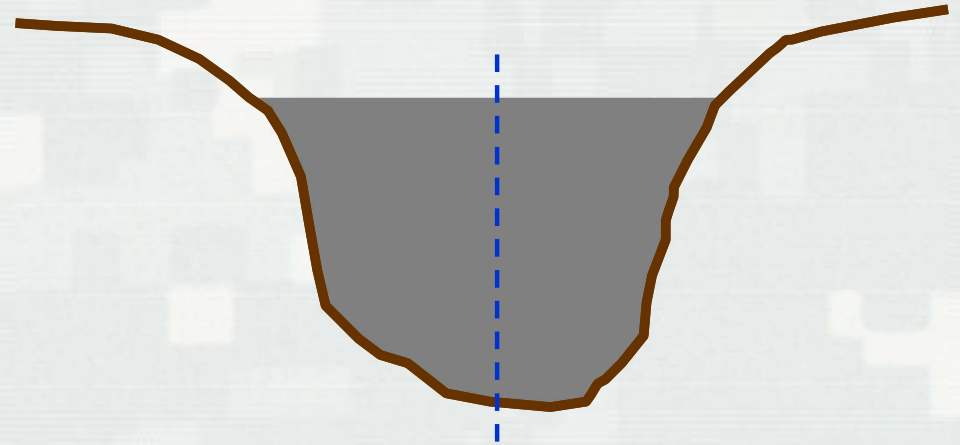
Piping Coefficient: 0.5

Initial Piping Elev: 620

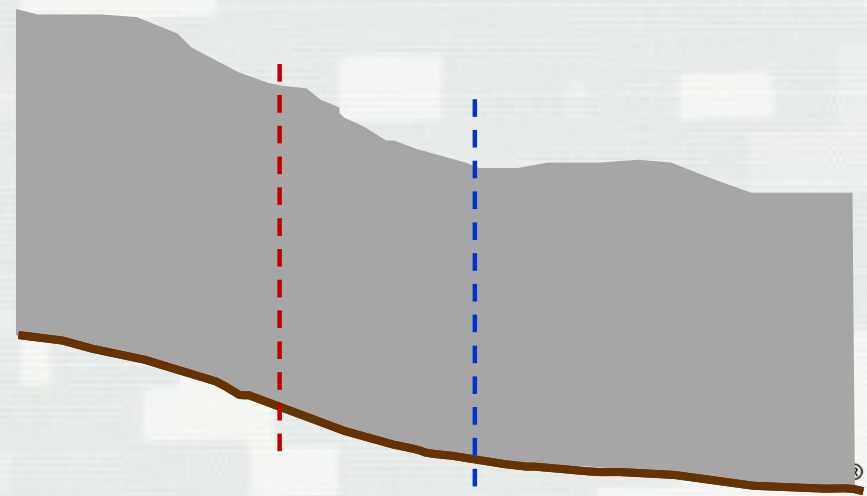
Trigger Failure at: WS Elev

Starting WS 676.8

This doesn't matter much for a dam



But levees can be more complicated



Where

Three Approaches

1. User Entered Data
-Parameter Estimation

2. Simplified Physical

3. DL Breach



Estimating the Breach Parameters

■ Literature And Guidance

- ▶ Existing COE guidance
- ▶ Prediction of embankment dam breach parameters: *USBR (1998) Dam Safety Research Report*

■ Regression Equations-

- ▶ MacDonald and Langridge-Monopolis (1984)
- ▶ Froehlich (1995b)
- ▶ Von Thun and Gillette (1990)
- ▶ Xu and Zhang (2009)



Technology Review (1998)

Prediction of Embankment Dam Breach Parameters

A Literature Review and Needs Assessment

DSO-98-004



Water Resources Research Laboratory

July 1998



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Suggested Breach Parameters

Dam Type	Average Breach Width (B_{ave})	Horizontal Component of Breach Side Slope (H) (H:V)	Failure Time, t_f (hours)	Agency
Earthen/Rockfill	(0.5 to 3.0) x HD	0 to 1.0	0.5 to 4.0	USACE 1980
	(1.0 to 5.0) x HD	0 to 1.0	0.1 to 1.0	FERC
	(2.0 to 5.0) x HD	0 to 1.0 (slightly larger)	0.1 to 1.0	NWS
	(0.5 to 5.0) x HD*	0 to 1.0	0.1 to 4.0*	USACE 2007
Concrete Gravity	Multiple Monoliths	Vertical	0.1 to 0.5	USACE 1980
	Usually $\leq 0.5 L$	Vertical	0.1 to 0.3	FERC
	Usually $\leq 0.5 L$	Vertical	0.1 to 0.2	NWS
	Multiple Monoliths	Vertical	0.1 to 0.5	USACE 2007
Concrete Arch	Entire Dam	Valley wall slope	≤ 0.1	USACE 1980
	Entire Dam	0 to valley walls	≤ 0.1	FERC
	(0.8 x L) to L	0 to valley walls	≤ 0.1	NWS
	(0.8 x L) to L	0 to valley walls	≤ 0.1	USACE 2007
Slag/Refuse	(0.8 x L) to L	1.0 to 2.0	0.1 to 0.3	FERC
	(0.8 x L) to L		≤ 0.1	NWS

***Note:** Dams that have very large volumes of water, and have long dam crest lengths, will continue to erode for long durations (i.e., as long as a significant amount of water is flowing through the breach), and may therefore have longer breach widths and times than what is shown in Table 3. HD = height of the dam; L = length of the dam crest; FERC - Federal Energy Regulatory Commission; NWS - National Weather Service

Where: HD = Height of the dam.

L = Length of the dam crest.

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Dam Type	Average Breach Width (B_{ave})	Horizontal Component of Breach Side Slope (H) (H:V)	Failure Time, t_f (hours)	Agency
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	(2.0 to 5.0) x HD	0 to 1.0 (slightly larger)	0.1 to 1.0	NWS
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	(0.8 x L) to L		≤ 0.1	NWS

You can start by trying 1h and 4 hrs and seeing if it makes a difference

***Note:** Dams that have very large volumes of water, and have long dam crest lengths, will continue to erode for long durations (i.e., as long as a significant amount of water is flowing through the breach), and may therefore have longer breach widths and times than what is shown in Table 3. HD = height of the dam; L = length of the dam crest; FERC - Federal Energy Regulatory Commission; NWS - National Weather Service

Where: HD = Height of the dam.

L = Length of the dam crest.

Breach Parameter Calculator

Dam (Inline Structure) Breach Data

Inline Structure: Cherry Creek 1 1.01

Breach This Structure

Breach Method: User Entered Data

Center Station: 7150

Final Bottom Width: 912

Final Bottom Elevation: 5523

Left Side Slope: 0.5

Right Side Slope: 0.5

Breach Weir Coef: 2.6

Breach Formation Time (hrs): 2.92

Failure Mode: Overtopping

Piping Coefficient: 0.5

Initial Piping Elev:

Trigger Failure at: WS Elev

Starting WS: 5639.5

Breach Plot | Breach Progression | Simplified Physical | Breach Repair (optional) | **Parameter Calculator**

Input Data

Top of Dam Elevation (ft): 5644.5

Breach Bottom Elevation (ft): 5523

Pool Elevation and Failure (ft): 5639.5

Pool Volume at Failure (acre-ft): 240000

Failure mode: Overtopping

MacDonald

Dam Crest Width (ft): 50

Slope of DS Dam Face Z1 (H:V): 2.6

Earth Fill Type: Non-homogeneous or Rockfill

Slope of DS Dam Face Z2 (H:V): 2.6

Xu Zhang (and Von Thun)

Dam Type: Dam with corewall

Dam Erodibility: Medium

Method	Breach Bottom Width (ft)	Side Slopes (H:V)	Breach Development Time (hrs)	
MacDonald et al	912	0.5	2.92	Select
Froehlich (1995)	675	1.4	3.04	Select
Froehlich (2008)	562	1	2.60	Select
Von Thun & Gillete	411	0.5	0.96	Select
Xu & Zhang	576	1.08	4.85 *	Select

* Note: the breach development time from the Xu Zhang equation includes more of the initial erosion period and post erosion than what is used in the HEC-RAS breach formation time.

OK Cancel

MacDonald and Langridge-Monopolis (1984)

Earthfill

$$V_{eroded} = 0.0261 (V_{out} * h_w)^{0.769}$$

$$t_f = 0.0179 (V_{eroded})^{0.364}$$

$$BREACH_{size} = f(V_{eroded})$$

Non-earthfill

$$V_{eroded} = 0.00348 (V_{out} * h_w)^{0.852}$$

Input Data

Top of Dam Elevation (ft):

Breach Bottom Elevation (ft):

Pool Elevation at Failure (ft):

Pool Volume at Failure (acre-ft):

Failure mode:



h_w = Pool Elev – Breach Bottom Elev

V_{out} = Pool Volume at Failure

(but is **total flow** volume – including inflow)



Von Thun & Gillette (1990)

$$B_{avg} = 2.5h_w + C_b$$

$$t_f = 0.02h_w + 0.25 \quad (\text{erosion resistant})$$

$$t_f = 0.015h_w \quad (\text{easily erodible})$$

Reservoir Size, m ³	C _b , meters
< 1.23*10 ⁶	6.1
1.23*10 ⁶ - 6.17*10 ⁶	18.3
6.17*10 ⁶ - 1.23*10 ⁷	42.7
> 1.23*10 ⁷	54.9

Reservoir Size, acre-feet	C _b , feet
< 1,000	20
1,000-5,000	60
5,000-10,000	140
>10,000	180

Input Data

Top of Dam Elevation (ft):

5644.5

Breach Bottom Elevation (ft):

5523

Pool Elevation at Failure (ft):

5639.5

Pool Volume at Failure (acre-ft):

240000

Failure mode:

Overtopping

$h_w = \text{Pool Elev} - \text{Breach Bottom Elev}$

$V_{out} = \text{Pool Volume at Failure}$



Froehlich (1995)

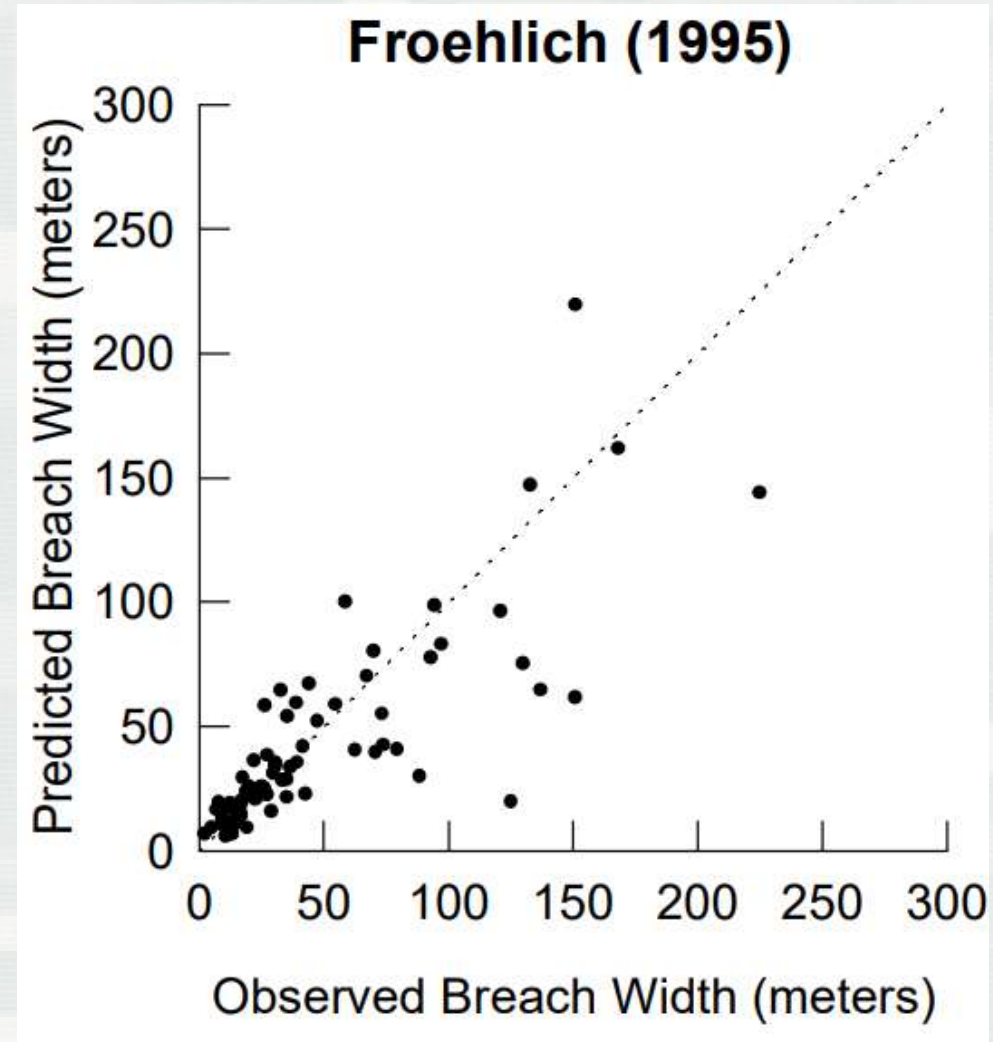
$$B_{ave} = 0.1803 K_o V_w^{0.32} h_b^{0.19}$$

$$t_f = 0.00254 V_w^{0.53} h_b^{-0.90}$$

Froehlich (2008)

$$B_{ave} = 0.27 K_o V_w^{0.32} h_b^{0.04}$$

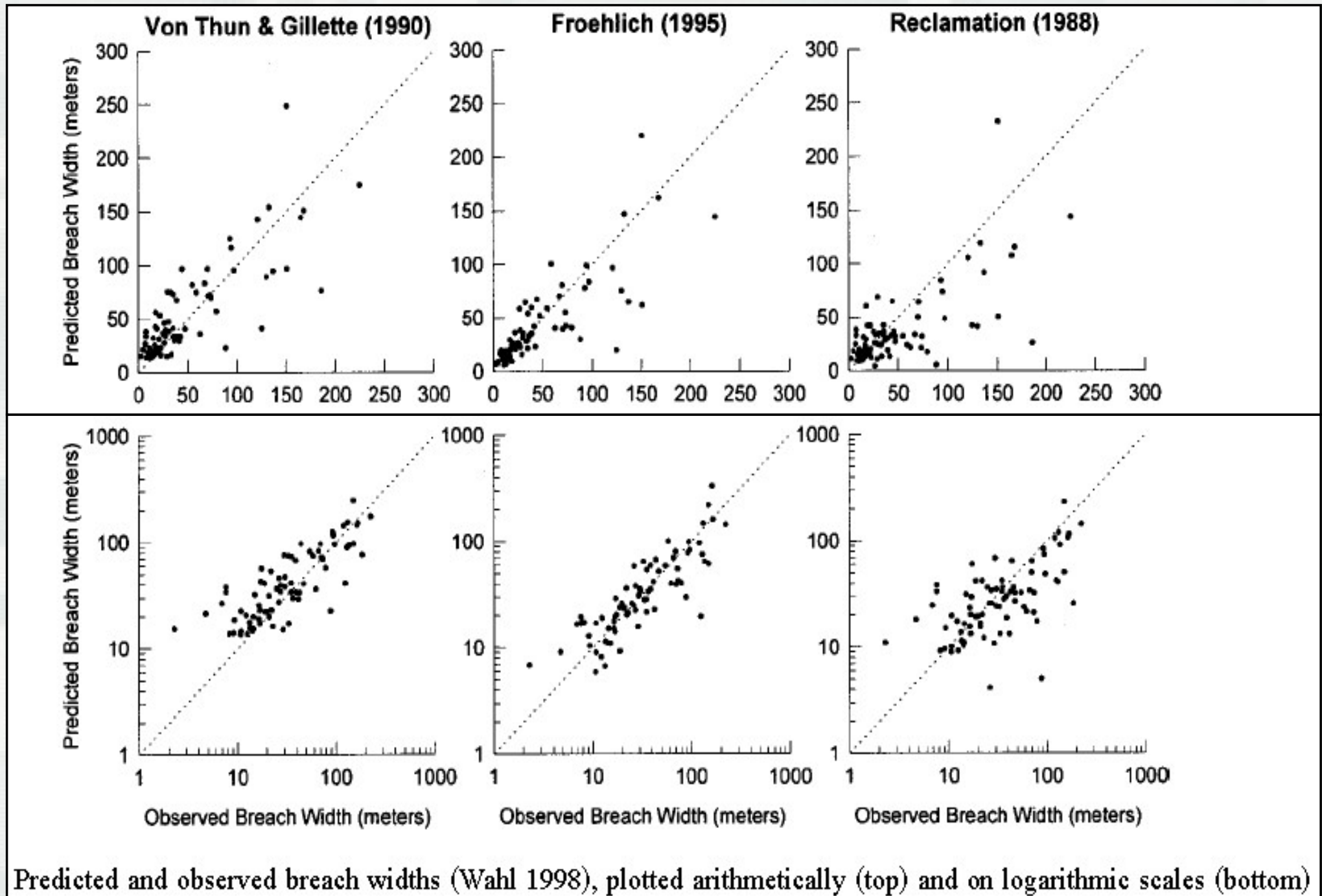
$$t_f = 20.18 V_w^{0.5} h_b^{-1.0}$$



V_w = Volume of water at h



Width Comparison



Four Important Ideas

1. Do not mix-and-match width and breach time.

Method	Breach Bottom Width (ft)	Side Slopes (H:V)	Breach Development Time (hrs)	
MacDonald et al	122	0.5	2.57	Select
Froehlich (1995)	628	1.4	3.44	Select
Froehlich (2008)	544	1	3.04	Select
Von Thun & Gillete	363	0.5	0.81	Select
Xu & Zhang	499	1.06	5.05 *	Select



Four Important Ideas

1. Do not mix-and-match width and breach time.

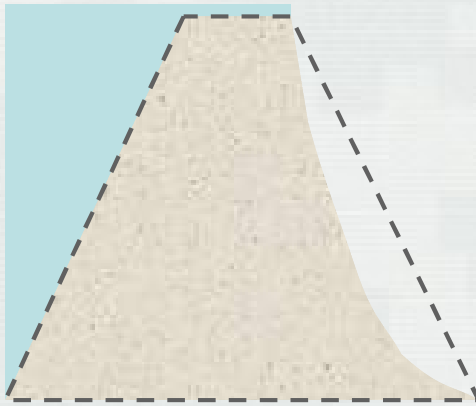
2. Xu & Zang has a different breach development time.

Method	Breach Bottom Width (ft)	Side Slopes (H:V)	Breach Development Time (hrs)	
MacDonald et al	122	0.5	2.57	Select
Froehlich (1995)	628	1.4	3.44	Select
Froehlich (2008)	544	1	3.04	Select
Von Thun & Gillete	363	0.5	0.81	Select
Xu & Zhang	499	1.06	5.05*	Select

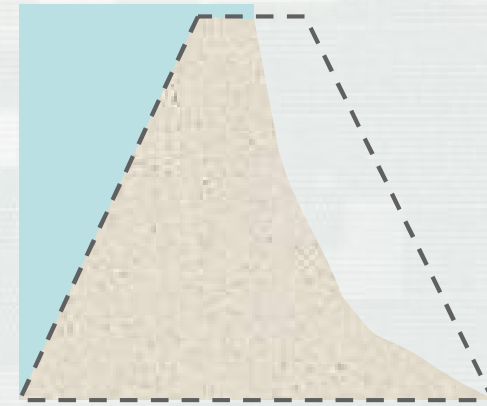
Thought Experiment

When does the clock start for “breach time” in HEC-RAS?

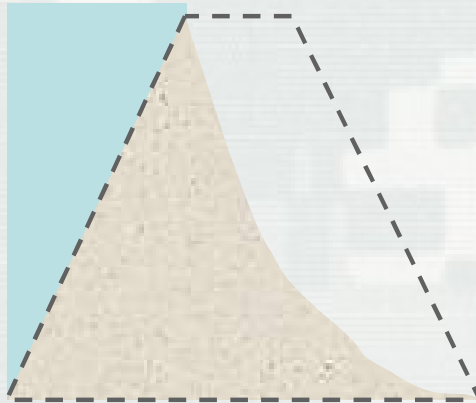
1.)



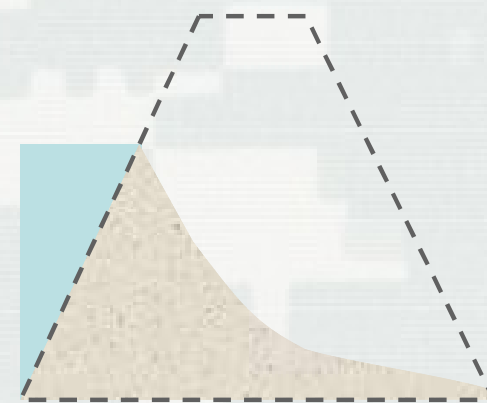
2.)



3.)



4.)



Four Important Ideas

1. Do not mix-and-match width and breach time.
2. Xu & Zang has a different breach development time.
3. Does the breach progression make physical sense?
 - Does it keep eroding at low head?
 - Does it stop eroding with despite high head and velocity?



Evaluate Breach Progress

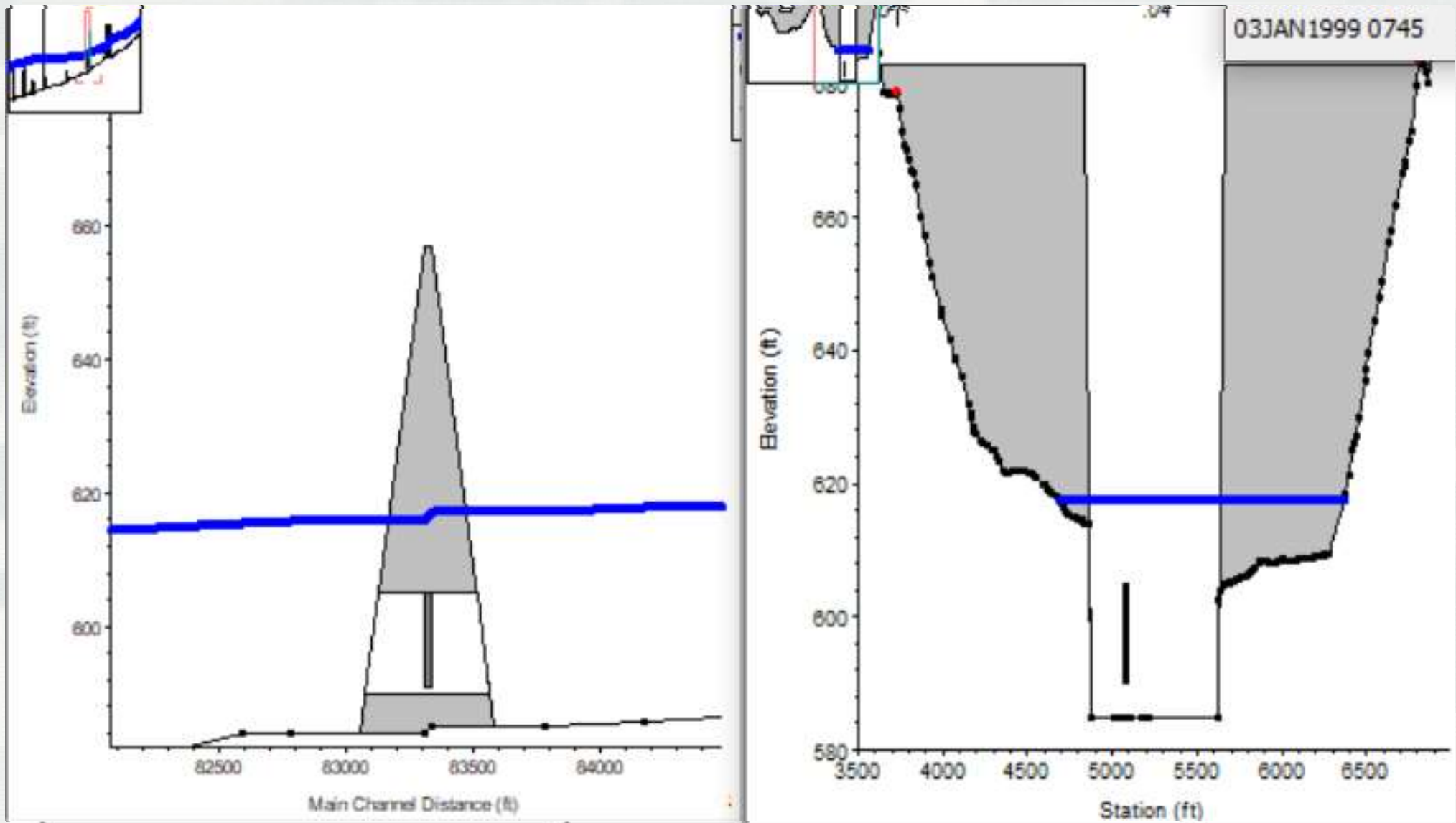
- Breach parameter estimation is uncertain.

Method	Breach Bottom Width (ft)	Side Slopes (H:V)	Breach Development Time (hrs)	
MacDonald et al	122	0.5	2.57	Select
Froehlich (1995)	628	1.4	3.44	Select
Froehlich (2008)	544	1	3.04	Select
Von Thun & Gillete	363	0.5	0.81	Select
Xu & Zhang	499	1.06	5.05 *	Select

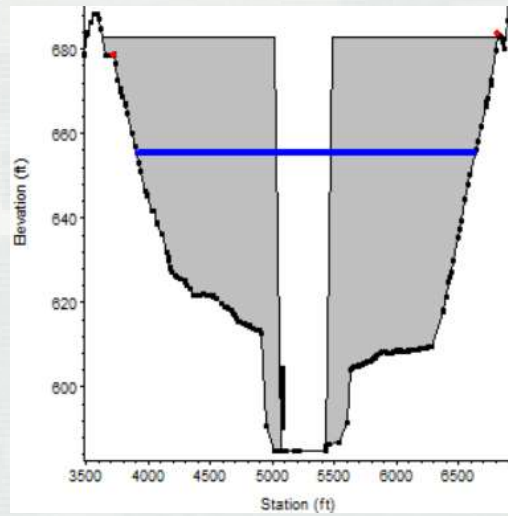
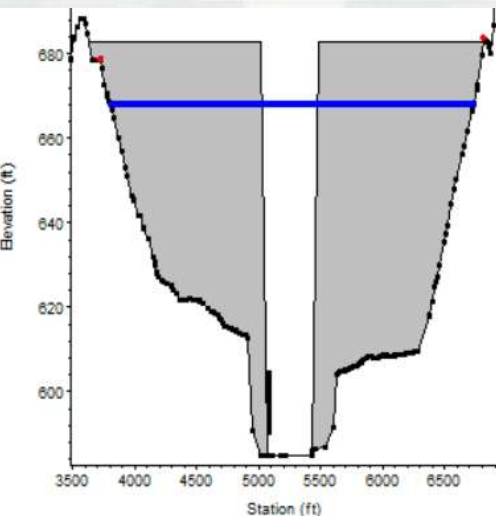
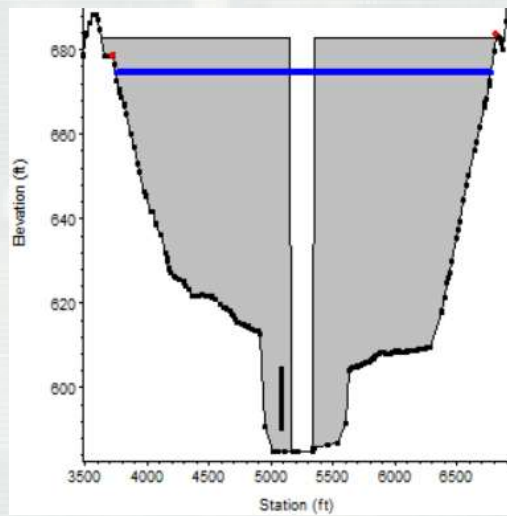
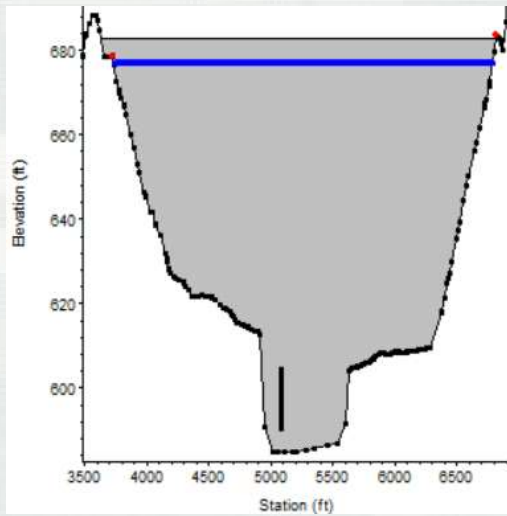
- We can use HEC-RAS hydraulic results to inform our decision on breach parameter selection.



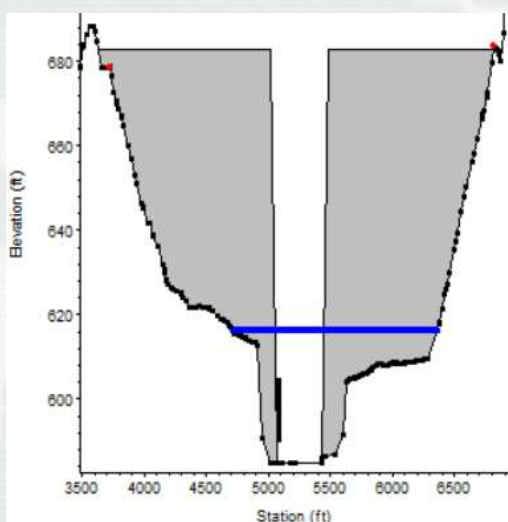
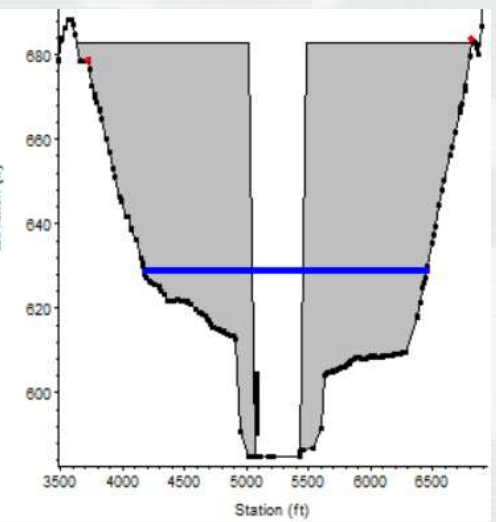
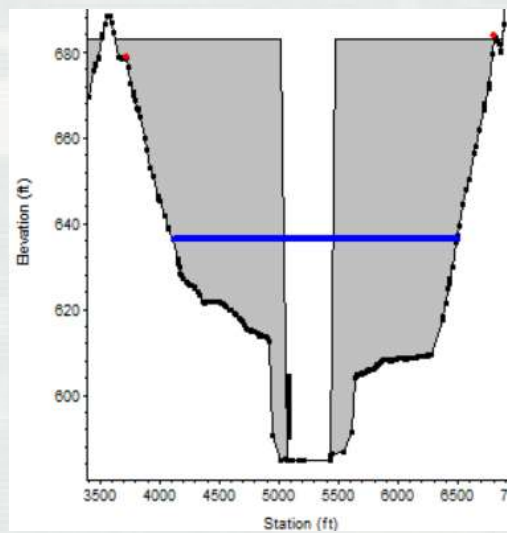
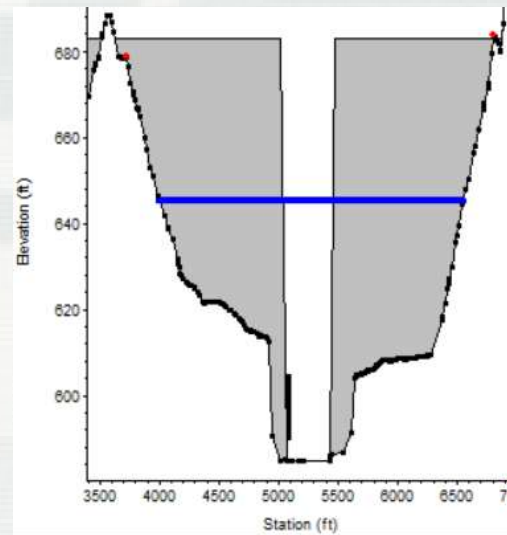
Profile and Breach Plots



Do you have any concerns about this simulation?



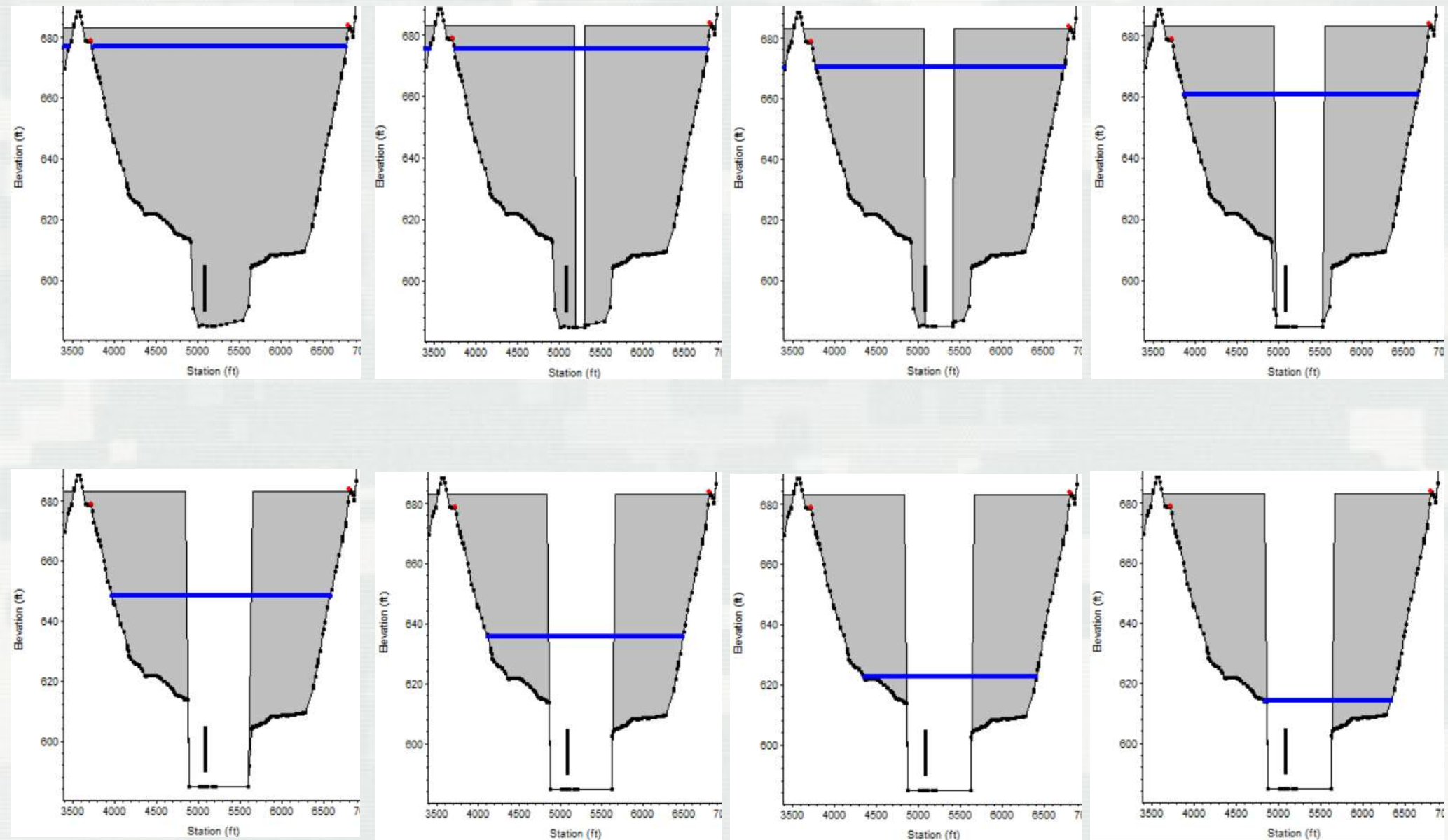
Breach Progression Ends Here



Δ Head and Velocity Still High

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This Simulation Continues to Widen for High Δ Head and Velocities



Widening rate in this method is not connected to hydraulics.
The user must check to make sure results make physical sense.

Three Approaches

1. User Entered Data
-Parameter Estimation

2. Simplified Physical

3. DL Breach



Dam (Inline Structure) Breach Data

Inline Structure Bald Eagle Loc Hav 81500 ▼ ↓ ↑ Delete this Breach ... Delete all Breaches ...

Breach This Structure

Breach Method: Simplified Physical ▼

Breach Plot | Breach Progression Simplified Physical | Breach Repair (optional) | Parameter Calculator |

Center Station: 3900

Max Possible Bottom Width: 1800

Min Possible Bottom Elev: 592

Left Side Slope: 2

Right Side Slope: 2

Breach Weir Coef: 2.6

Breach Formation Time (hrs): 1

Failure Mode: Piping ▼

Piping Coefficient: 0.6

Initial Piping Elev: 620

Initial Piping Diameter: 1

Mass Wasting Feature:

Trigger Failure at: WS Elev ▼

Starting WS 668.1

Overtopping Downcutting

	Velocity (ft/s)	Downcutting Rate (ft/hr)
1	0	0
2	2	0
3	3	1
4	4	2
5	5	5
6	7	15
7	10	30
8	20	50
9	30	100
10		
11		
12		
13		
14		
15		
16		
17		
18		
19		
20		
21		
22		
23		
24		
25		

Widening Relationship

	Velocity (ft/s)	Widening Rate (ft/hr)
1	0	0
2	2	0
3	3	1
4	4	2
5	5	5
6	7	15
7	10	30
8	20	50
9	30	100
10		
11		
12		
13		
14		
15		
16		
17		
18		
19		
20		
21		
22		
23		
24		
25		

OK

Cancel

Where do these rates come from?



Prepared by:
MMC MCX

Appendix 3.X.X – Application of Simplified Physical Breach Method in HEC-RAS

FY17 MMC Technical Manual SOP

July 2018

Draft

FOR OFFICIAL USE ONLY (FOUO)

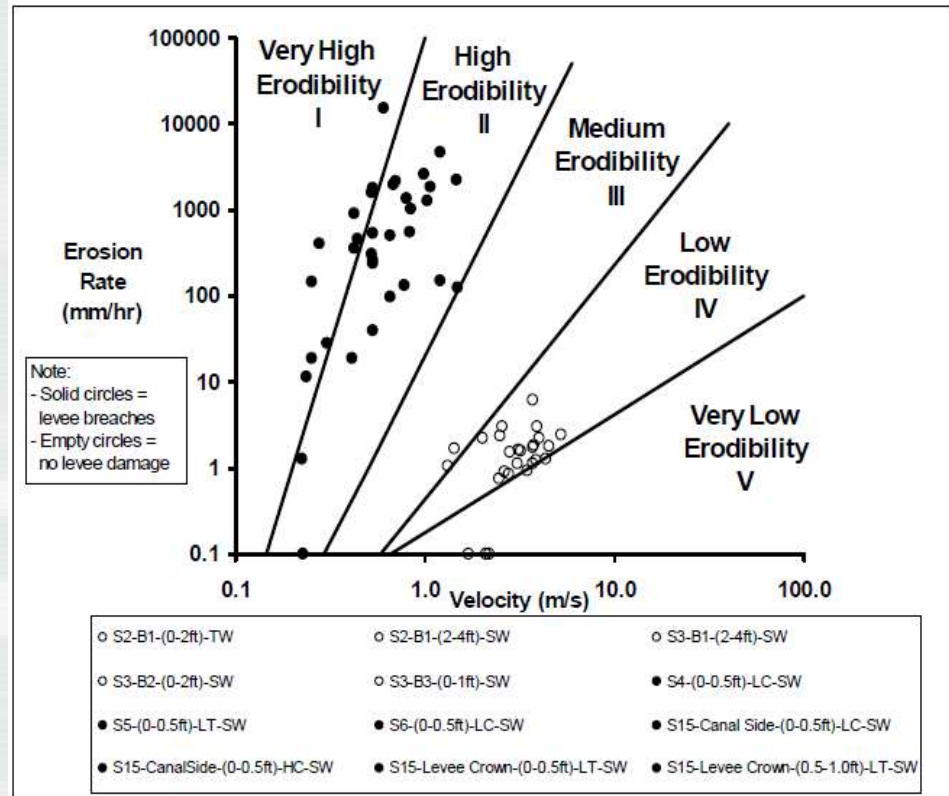
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Technical Memorandum for Record

Date: May 31, 2013 (Revised July 2, 2013)

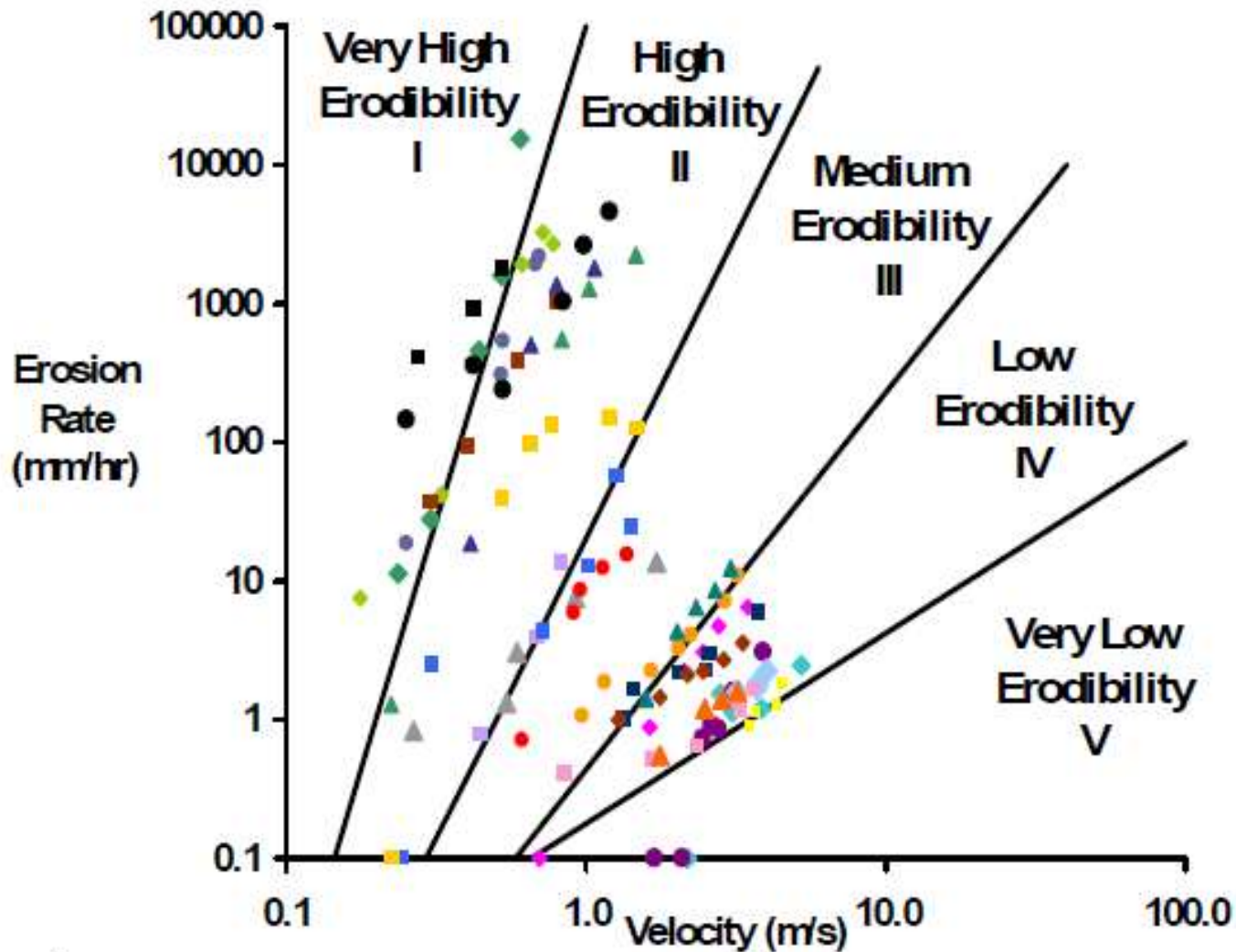
From: Chris Bahner, P.E., D. WRE

Subject: Updated Levee Breach Characteristics for MMC SOP



The guidance is under development.
There are some historic values that can help.

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- | | | |
|----------------------------------|-----------------------------------|-------------------------------------|
| ◆ S1-B1-(0-2ft)-TW | ▲ S1-B1-(2-4ft)-SW | ◆ S2-B1-(0-2ft)-TW |
| ● S2-B1-(2-4ft)-SW | ◆ S3-B1-(2-4ft)-SW | ■ S3-B2-(0-2ft)-SW |
| ● S3-B3-(0-1ft)-SW | ◆ S4-(0-0.5ft)-LC-SW | ◆ S4-(0-0.5ft)-HC-SW |
| ▲ S5-(0-0.5ft)-LT-SW | ● S6-(0-0.5ft)-LC-SW | ● S7-B1-(0-2ft)-TW |
| ● S7-B1-(2-4ft)-SW | ● S8-B1-(0-2ft)-TW | ◆ S8-B1-(2-4ft)-L1-SW |
| ▲ S8-B1-(2-4ft)-L2-SW | ◆ S11-(0-0.5ft)-LC-TW | ■ S11-(0-0.5ft)-HC-TW |
| ■ S12-B1-(0-2ft)-TW | ▲ S12-B1-(2-4ft)-SW | ▲ S15-Canal Side-(0-0.5ft)-LC-SW |
| ● S15-Canal Side-(0-0.5ft)-HC-SW | ● S15-Levee Crown-(0-0.5ft)-LT-SW | ■ S15-Levee Crown-(0.5-1.0ft)-LT-SW |



Three Approaches

1. User Entered Data
-Parameter Estimation

2. Simplified Physical

3. DL Breach



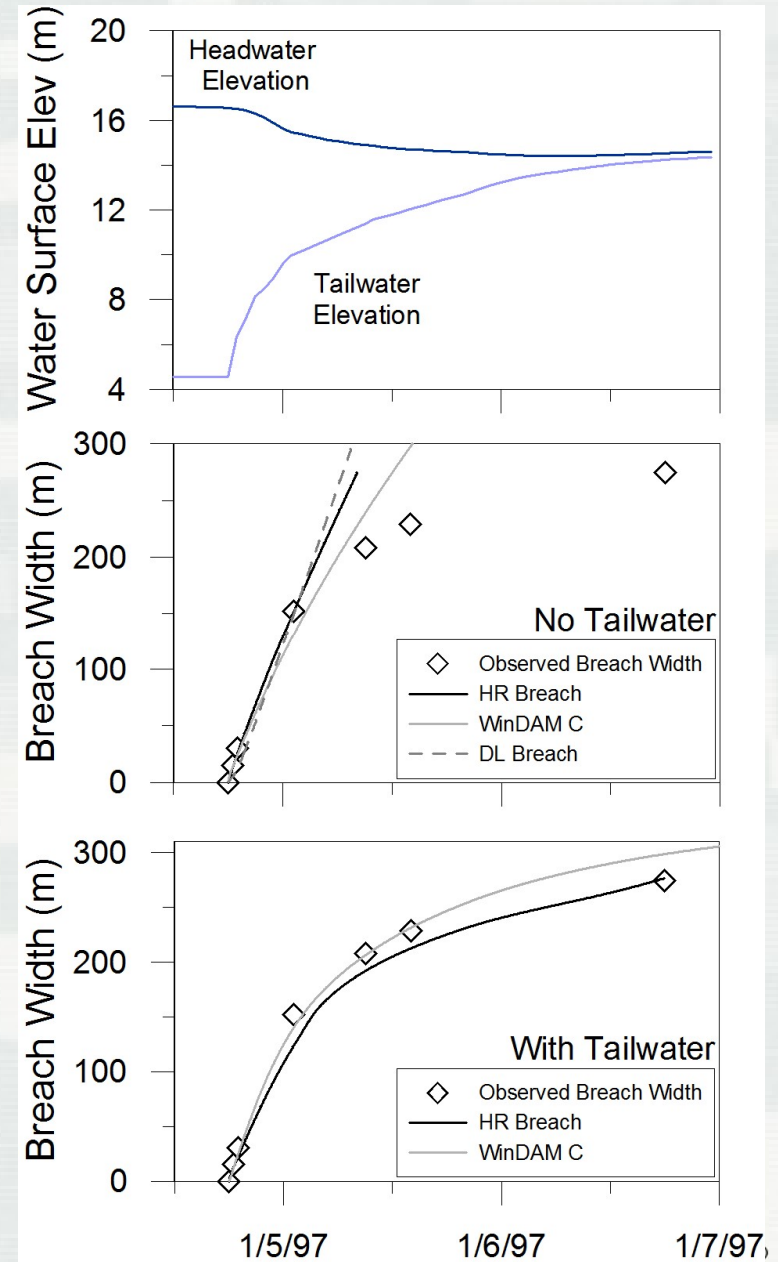
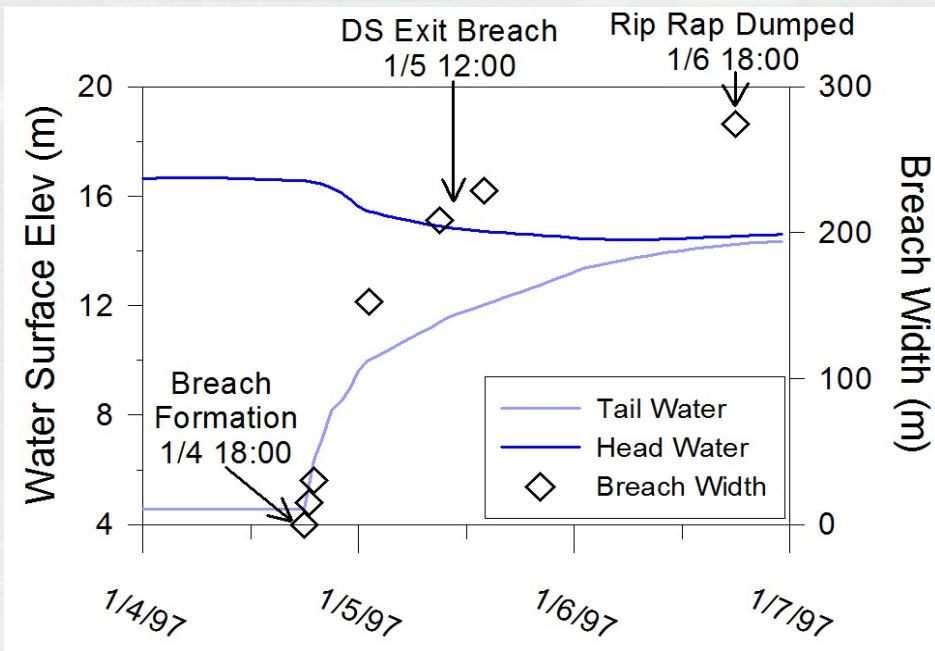
Estimating the Breach Evolution

Process Models:

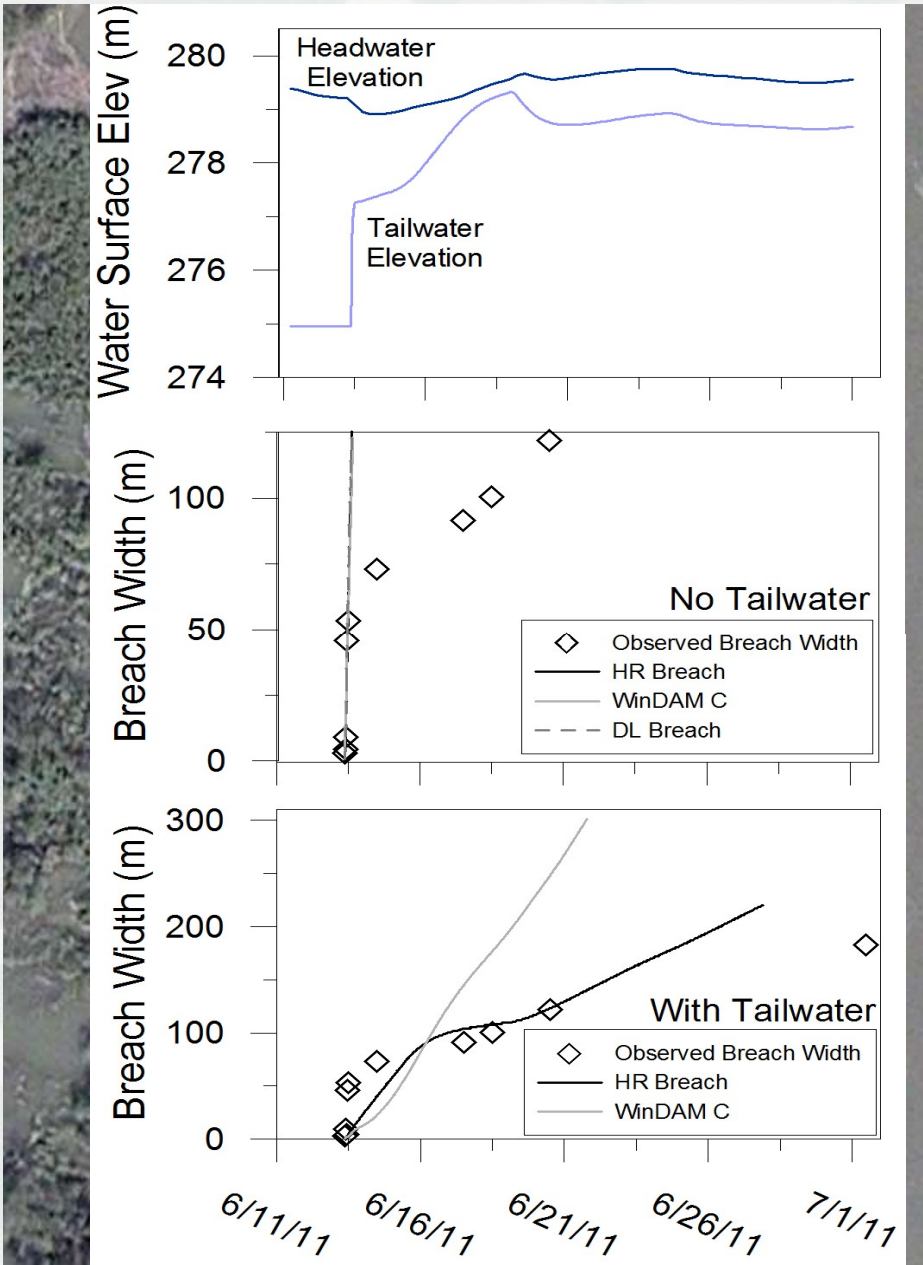
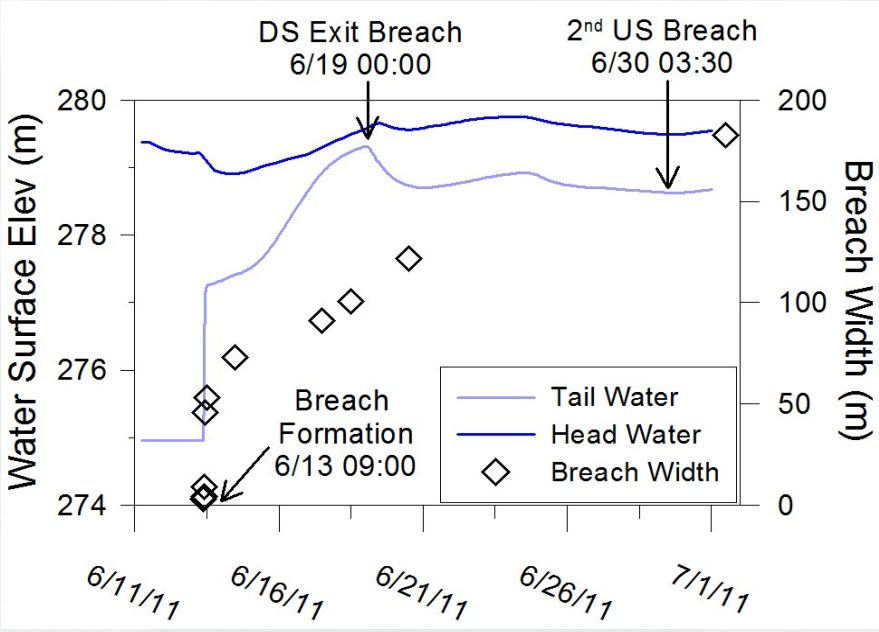
- ▶ ARS SIMBA/WinDAMB
- ▶ HR-BREACH (HR Wallingford)
- ▶ DL Breach (Dr Weiming Wu)



Sutter Bypass Levee Breach



Hamburg Breach (Missouri River)

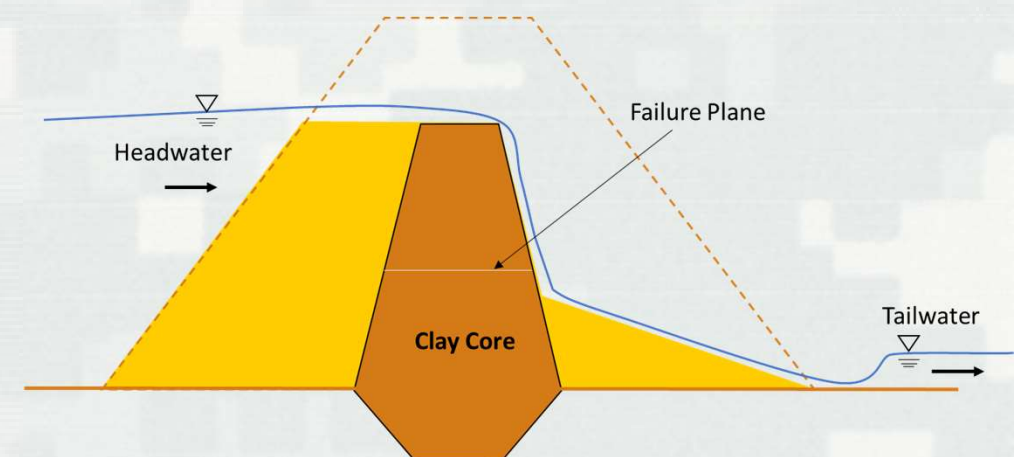
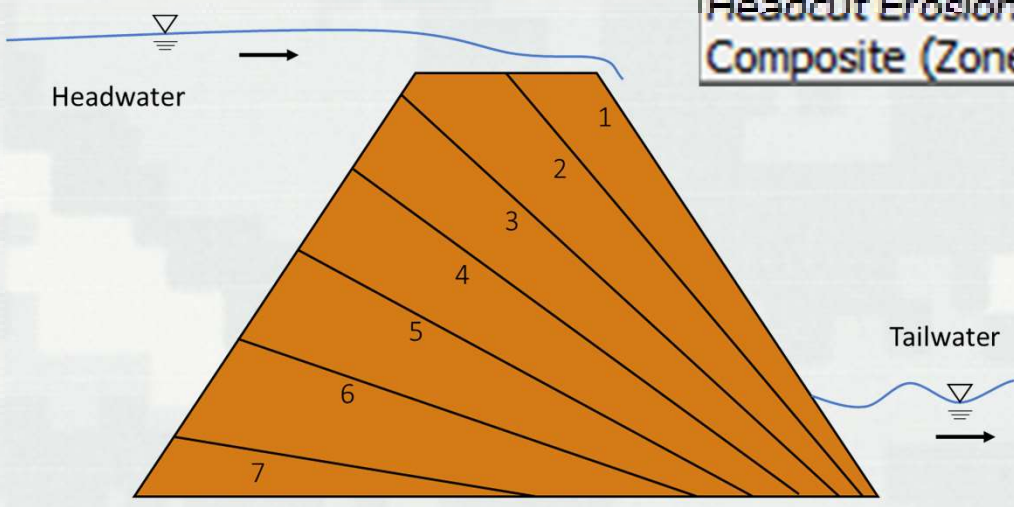


From: Risher, P. and Gibson, S. (2016) "Applying Mechanistic Dam Breach Models to Historic Levee Breaches," Proceedings FloodRisk 2016.

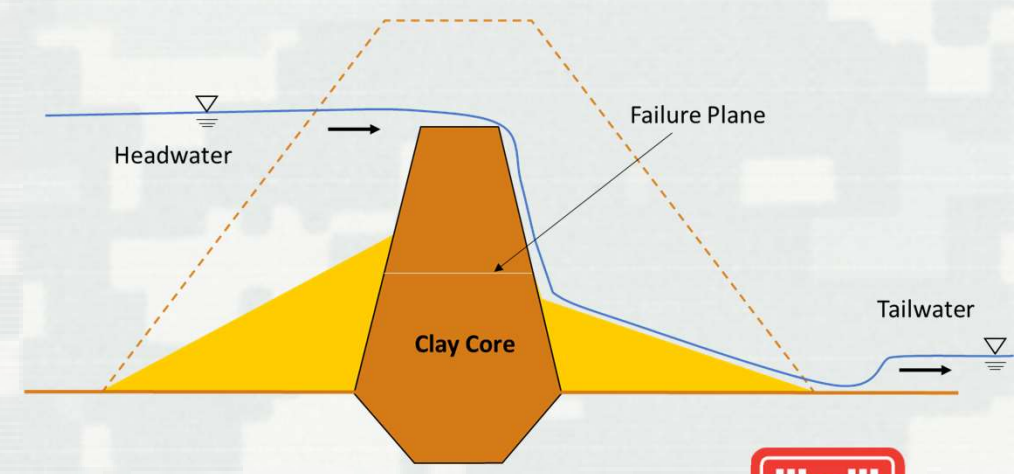
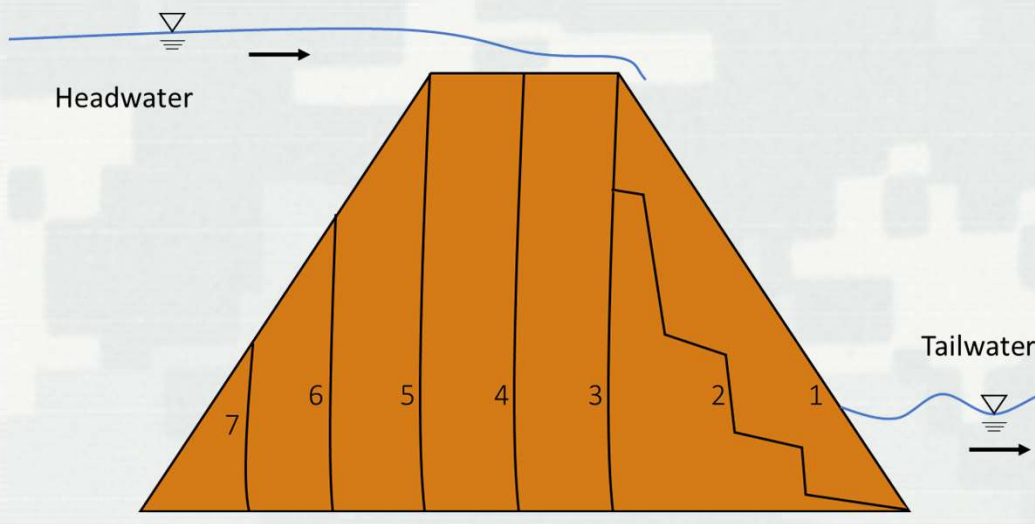
DLBreach

Surface Erosion
Headcut Erosion
Composite (Zoned) Structure

Surface



Headcut

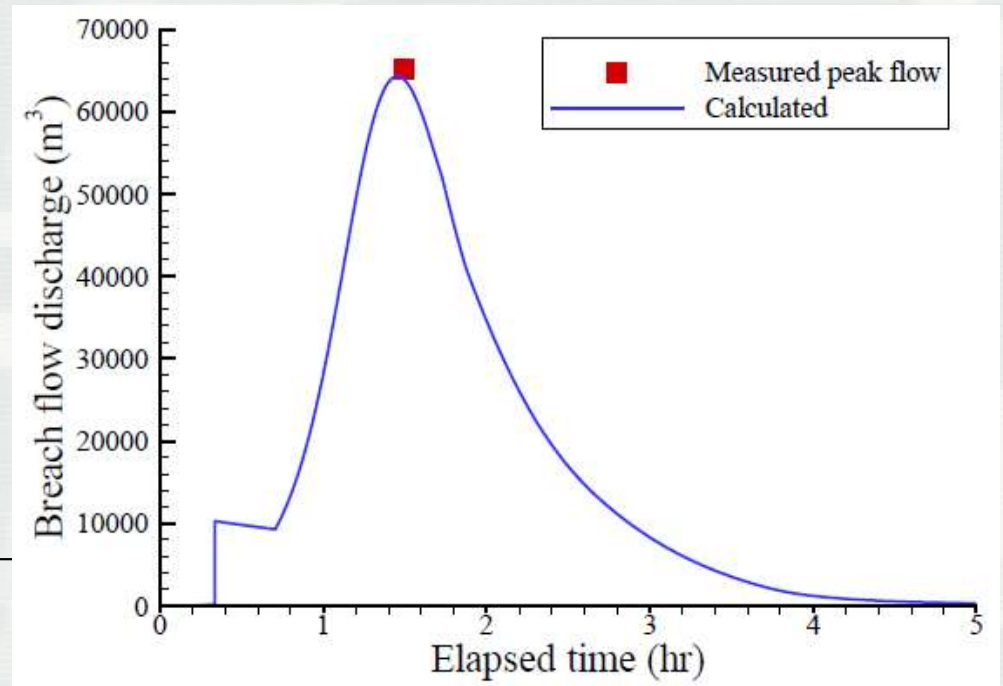
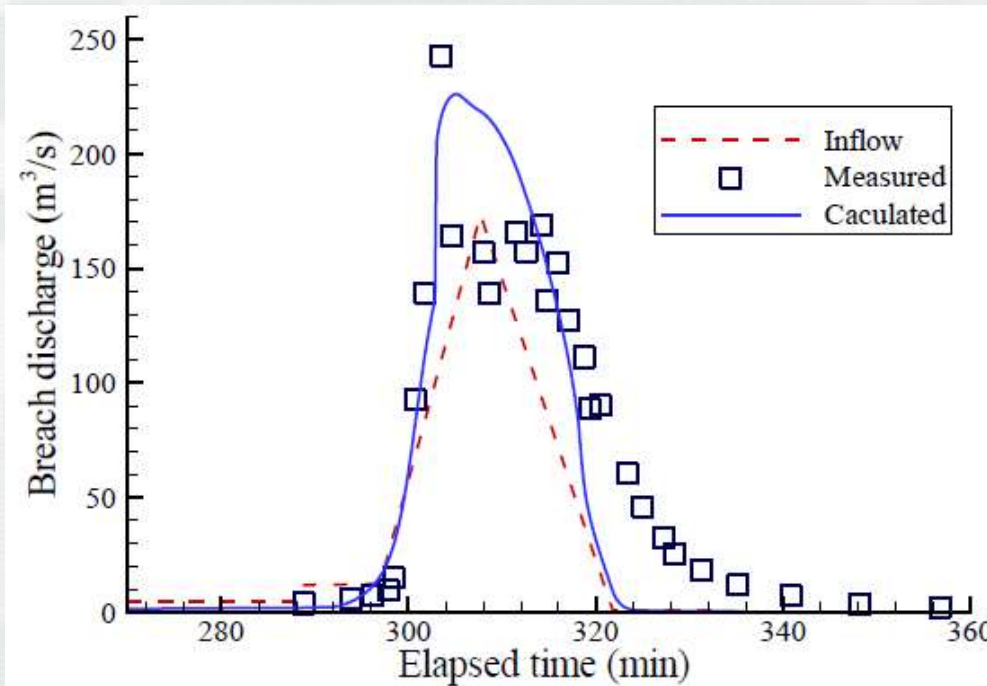
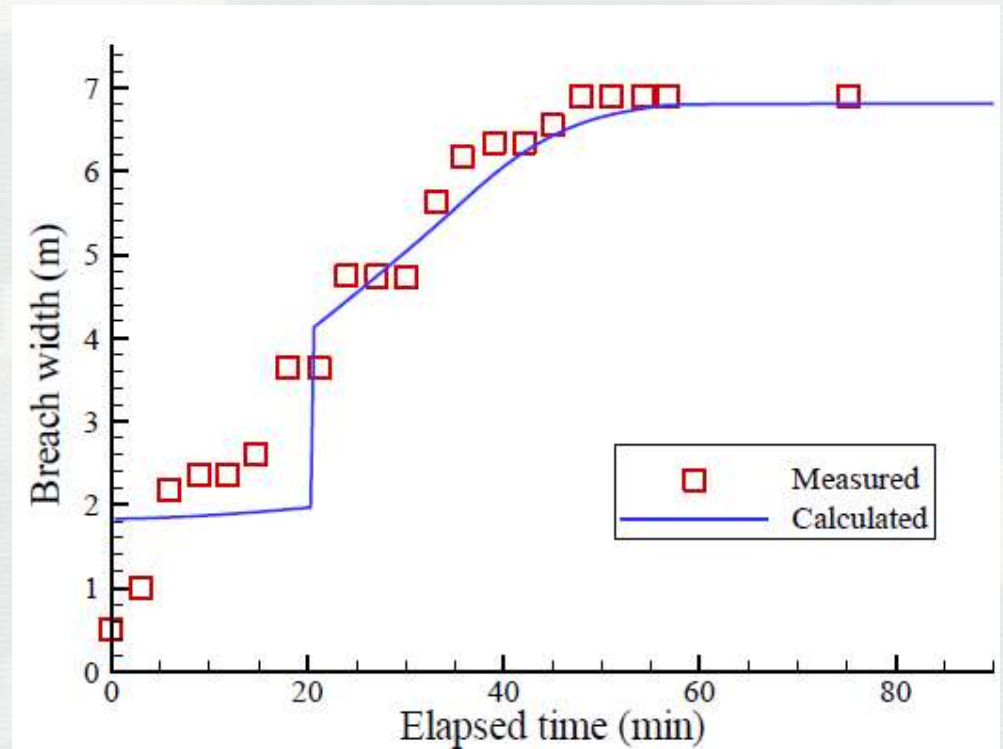
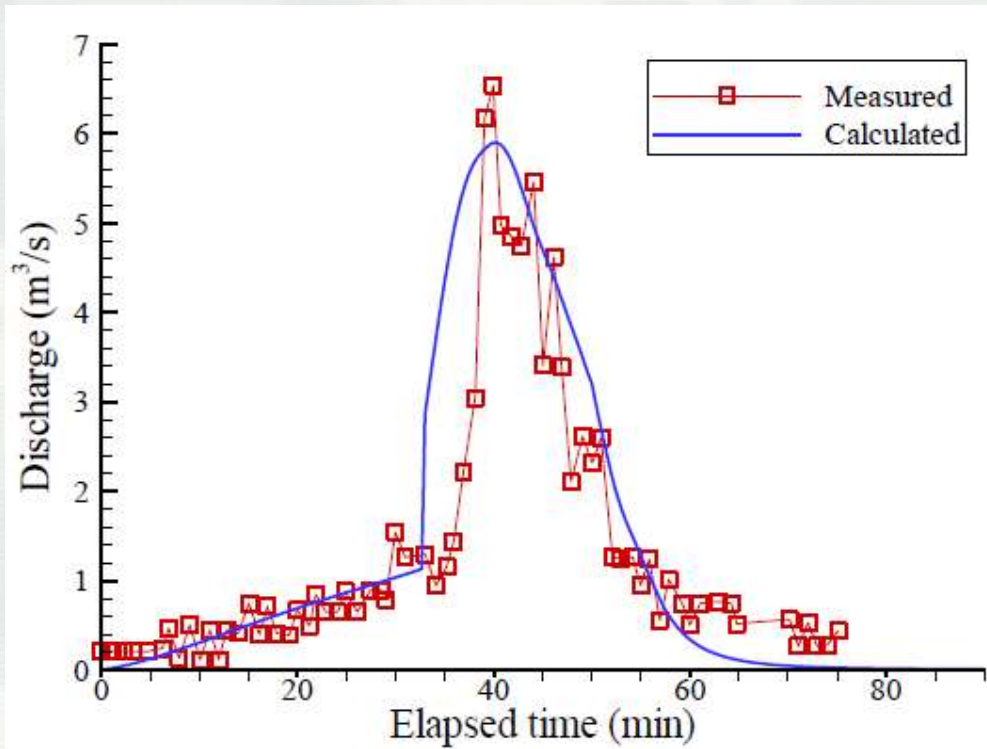


Composite



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DLBreach



DLBreach

Dam (Inline Structure) Breach Data

Inline Structure: Teton Reach 1 2.5

Breach This Structure

Breach Method: Physical Breaching (DLBreach)

Center Station: 1000

Max Possible Bottom Width: 250

Min Possible Bottom Elev: 0

Left Side Slope: 0

Right Side Slope: 0

Breach Weir Coef: 1.7

Breach Formation Time (hrs):

Failure Mode: Piping

Piping Coefficient: 0.05

Initial Piping Elev: 48

Initial Piping Diameter: 0.1

Mass Wasting Feature:

Trigger Failure at: Set Time

Start Date: 01JAN2000

Start Time: 2400

Breach Plot | Breach Progression | Simplified Physical | Physical Breaching (DLBreach) | Parameter Calculator | Breach Repair (optional)

Erosion Model (Overtopping Only):

Surface Erosion

Headcut Erosion

Composite (Zoned) Structure

Embankment Width: 10.5

US Slope: Slope (H:B) 0.3333 Roughness: 0.016

Flat Top: 0.016

DD Slope: 0.4 0.016

Soil Parameters:

Soil Type: Cohesive

Sediment Diameter: 0.00003

Porosity: 0.3

Specific Gravity: 2.65

Clay Content: 0.3

Cohesion: 25000.

Friction Angle: 0.65

Adaptation Length:

Erodibility (kd): 8.

Critical Shear Stress: 0.15

Breach Direction: One Way

Model a cover layer

Clay Cover and Core Parameters:

Parameters	Cover	Core
Core Height:		
Core Crest Width:		
Core US Slope:		
Core DS Slope:		
Core Center Location:		
Core Manning n:		
Soil Type:		Cohesionless
Sediment Diameter:		
Porosity:		
Specific Gravity:		
Clay Content:		
Cohesion:		
Friction Angle:		
Erodibility (kd):		
Critical Shear Stress:		
Top Thickness:		
US Slope Thickness:		
DS Slope Thickness:		

OK Cancel

Which of these parameters are hard?

Four Important Ideas

1. Do not mix-and-match width and breach time.
2. Xu & Zang has a different breach development time.
3. Does the breach progression make physical sense?
4. Test sensitivity...on important result.
(e.g. Arrival Time/Max Stage vs Breach Geometry)
-Try multiple methods





**US Army Corps
of Engineers**
Hydrologic Engineering Center

Using HEC-RAS for Dam Break Studies

August 2014

<https://www.hec.usace.army.mil/publications/TrainingDocuments/TD-39.pdf>



USBR Levee Breach Lab Studies

Hydraulic Laboratory Report HL-2011-09

Physical Hydraulic Modeling of Canal Breaches

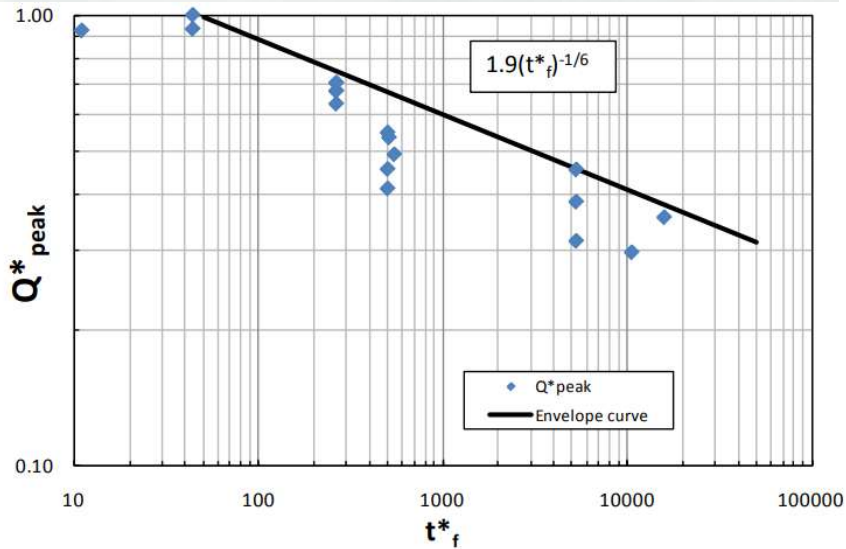


Figure 28. — Dimensionless peak discharge as a function of dimensionless breach development time.

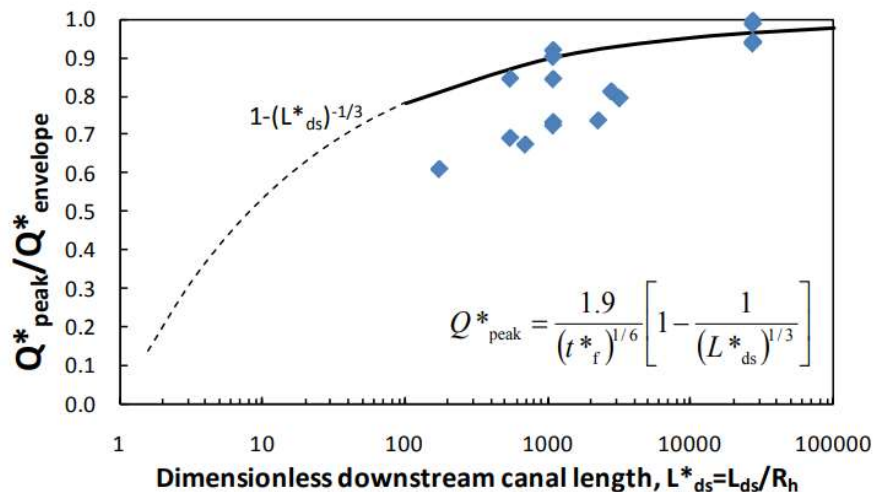


Figure 29. — Effect of downstream canal reach length on peak breach outflow.



U.S. Department of the Interior
Bureau of Reclamation
Technical Service Center
Hydraulic Investigations and Laboratory Services Group
Denver, Colorado

September 2011

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A Brief Plug

The RSM

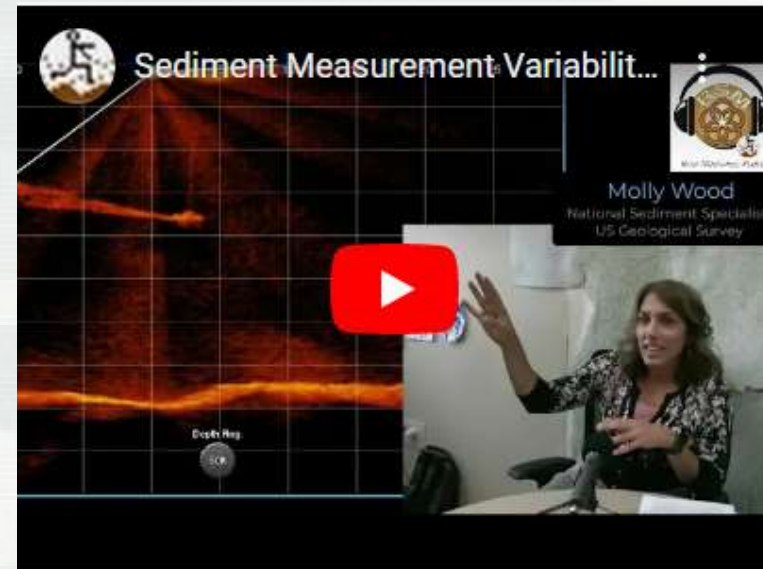
River Mechanics

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- ▶ River Processes
- ▶ Channel Response to Engineering Measures
- ▶ Dam Removal Modeling
- ▶ River Restoration



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