Workshop 2D Bridge Hydraulics

Introduction

This workshop will help students learn how to use HEC-RAS to model bridges inside of 2D Flow Areas and review and understand pertinent 2D Bridge output.

NOTE: While this data is from an actual river system, the model and results of this workshop do not represent current or future conditions of the river. The United States Army Corps of Engineers has granted access to the information in this model for instructional purposes only. Do not copy, forward, or release the information without United States Army Corps of Engineers approval.

Background

Figure 1 shows the extent of an existing 2D Flow model. The Terrain data is from Bald Eagle Creek. For this workshop you will be modeling just a small portion of Bald Eagle creek, which contains one bridge crossing. You will be starting with an existing model of the area that does not contain the bridge. The goal of this workshop is to add the bridge data; run the model; review the output; and answer some questions about the model.



Figure 1. Terrain and existing 2D Model for Bald Eagle Creek Bridge workshop.

Tasks

1. Open the Project and Run the 2D No Bridge Plan

Open the Project and run the only plan, which is called "2D No Bridge".

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<u>File Edit R</u> u	un <u>V</u> iew <u>O</u> ptions <u>G</u> IS Tools <u>H</u> elp Debug	
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Project:	2D Bridge Workshop	d:\\WS-2D Bridge Hydraulics\2DBridgeWorkshop.prj
Plan:	2D No Bridge	d:\\WS-2D Bridge Hydraulics\2DBridgeWorkshop.p01
Geometry:	Single 2D Area No Bridge	d:\\WS-2D Bridge Hydraulics\2DBridgeWorkshop.g01
Steady Flow:		
Unsteady Flow:	Single Event	d:\\WS-2D Bridge Hydraulics\2DBridgeWorkshop.u02
Description:	The United States Army Corps of Engineers has granted	access to the information in this model for 👔 Beta Version US Customary Units

2. Make a Copy of the Geometry File

Go to the Geometric Data editor and make a copy of the current Geometry data using the "**Save As**" option. Label the new geometry file something like: "**Single 2D Area with Bridge**".

3. Add the Bridge into the Geometry Data.

You can add a 2D Bridge with either RAS Mapper or the Geometry editor. However, RAS Mapper only allows you to lay out the geospatial centerline of the 2D Bridge, and to give it a name. All of the actual bridge data must be entered from the Geometry editor. So, for this workshop we will do the whole thing in the geometry editor. To add a bridge to the 2D Flow Area from the Geometry editor, perform the following steps:

a. Zoom into the bridge location (see below), then select the **SA/2D Area Conn** tool. Draw the bridge centerline from left to right, while looking downstream, as shown in Figure below:



Figure 2. Using the SA/2D Area Conn to draw the bridge centerline.

b. Left click on the Bridge centerline and select "**Edit Internal Connection Cell Spacing**". This will bring up the editor shown below. Put in a minimum cell spacing = 30 ft, and a maximum cell spacing = 30 ft.

HEC-RAS							
	Enter Breaklin	e Cell Spacing					
Minimum (blank Maximum (blan	Minimum (blank for mesh spacing) Maximum (blank for mesh spacing)						
ОК	Cancel	Defaults	Help				

Figure 3. Internal Connection Cell Spacing editor.

b. Next, left click on the 2D Bridge centerline and select "Enforce Internal Connection as Breakline in 2D Flow Area". You mesh should look like this:



Figure 4. 2D Bridge Mesh after Breakline Enforcement.

d. Right click on the 2D Bridge centerline and select "Edit Connection. This will bring up the SA/2D Area Conn editor as shown below. Change the Structure Type to Bridge (Internal to 2D Flow Area).

v Connecti File View	on Data Editor - Single 2D Area No Bridge Options Help				 _	×
Connection: Description	Bridge	Apply Data	Breach (plan dat	a)		
Connections From:	2D Flow Area: BaldEagleCr 2D Flow Area: BaldEagleCr	Set SA/2D	Weir Length:	n/a		
Overflow Cor	putation Method Equation Domain 💿 Use Weir Equation		Centerline GIS Cod	ords		
Structure Type Plot:	Bridge (Internal to 2D Flow Area) US Inside Bridge	• • • •	Cut profile from ter	rain D Cells		
Pier Stoping Abutment Bridge Modeling Approach HTab Param.		No Da	ata for Plot			

Figure 5. SA/2D Area Conn editor, with Bridge Structure Type selected.

e. Select the **Deck/Roadway** editor on the left and enter the data as shown in the Figure below:

Deck/Roadway Data Editor								
Dis	tance			Wid	th	We	eir Coef	
20. 40.						2.6		
Clear	De	Row	Ins Row			Сор	y US to DS	
	Up	ostream				Downstrea	am	
Stat	tion	high cho	rd low ch	ord	Station	high chord	low chord	•
1 0		570.4	566.6		0	570.4	566.6	
2 289.3	3	571.6	567.8		289.3	571.6	567.8	
3								
4								
5								
6								
7								
8								
U.S Emban	kment	SS	0		D.S Em	bankment S	s 0	
-Weir Data	a — —							
Max Submergence: 0.98 Min Weir Flow El:								
Weir Cres Broad Ogee	<u>t Shar</u> d Cres	<u>be</u> ted						
	OK Cancel							
Enter distance between upstream cross section and deck/roadway. (ft)								

Figure 6. 2D Bridge Deck and Roadway data editor with data shown.

Once you have entered all the data, close the editor. In order to get the editor to update the drawing, click on the window or move the window. The editor should now look like this:



Figure 7. SA/2D Area Conn with 2D Bridge Deck/Roadway data added.

f. The next step is to add the bridge piers. Open the **Pier** editor on the left and enter the pier data. There are two piers, they are each 2 ft wide. One is a station 125, the other is at station 185. See the data in the Figure below:

Pier Data Editor
Add Copy Delete Pier # 1 💌 🖡 🇊
Del Row Centerline Station Upstream 125 Ins Row Centerline Station Downstream 125 Floating Pier Debris All On All Off All On All Off Apply floating debris to this pier Set Wd/Ht for all Debris Width:
Upstream Downstream
Pier Width Elevation Pier Width Elevation
1 2 545 2 545
2 2 570 2 570
3
3 4
3 4 5

Figure 8. Pier Editor with the data for the first pier.

g. Next, add the **Bridge Modeling Approach** data as shown below:

Conne	Connection Bridge Modeling Approach Editor						
Low Use	Flow Methods Compute F Energy (Standard Ste	p)					
0	Momentum	Coef Drag Cd					
0	Tarnell (Class A only)	Pier Shape K	2				
•	Highest Energy Answer						
⊢High CE ⊙p	High Flow Methods C Energy Only (Standard Step) Pressure and/or Weir Submerged Inlet Cd (Blank for table) Submerged Inlet + Outlet Cd Max Low Chord (Blank for default)						
	OK Cancel Help						
Use pr	Use pressure and/or weir method for high flow						

Figure 9. Bridge Modeling Approach Editor.

Combined 1D River and Floodplain with 2D Area behind the Levees

h. Next select the **HTab Param** button to enter the Hydraulic Tables parameters. See below:

Connection Hydraulic Property Table Parameters								
Number of points on free	80							
Number of submerged cu	60							
Number of points on eac	50							
Apply number o	Apply number of points to all Connections							
Head water maximum elevation: 575								
Tail water maximum elevation (Optional):								
Maximum Flow (Recommended):								
	ОК	Cancel						

Figure 10. Htab Parameters to control how the Bridge Curves get built.

h. Next. Close the editor. This is required in order for the software to automatically locate the two cross sections outside of the bridge. The bridge and the 1D cross section locations should look like what is shown in the Figure below:



Figure 11. Final location of 1D Bridge Cross Sections after Bridge Data is entered.

I. Go back into the SA/2D Area Conn editor and inspect all of the 1D cross sections that will be used to create the Family of Bridge curves. Next go to the **Options** menu and select **External and Internal Bridge Cross Sections**. This will bring up the editor shown below. This editor allows you to change the cross section data (just in case the terrain data is not accurate); change the main channel bank station locations (the default is the ends of the cross sections); and to enter Manning's n data for the cross sections. Entering Manning's n data for the cross sections is required and must be done in this editor. You must enter a Manning's n value at Station zero. This Manning's n values will remain in affect until you change it at another station. At least one Manning's n value is required at the first station. For this workshop we will only enter a single Manning's n value of .04 at the first station (See figure below):

Br	idge	Cross Se	ctions																		
		Up	ostream Outs	ide			U	lpstream Insid	de			Downstream Inside					Downstream Outside				
Main Channel Bank Stations Left Bank Sta Right Bank Sta 0 289.2				Main Channel Bank Stations Left Bank Sta Right Bank Sta 0 289.2				Main C Left Bank)	hannel Bank Sta Rig 289.3	Stations ht Bank Sta 2	_		Main (Left Ban 0	Channel Bank k Sta Rig 289.	Stations ht Bank Sta 2						
		Cross Se	ection X-Y Co	ordinates			Cross Se	ection X-Y Co	ordinates			Cross S	ection X-Y Co	ordinates			Cross Se	ection X-Y Co	ordinates		
		Station	Elevation	Mann n	_		Station	Elevation	Mann n	-		Station	Elevation	Mann n			Station	Elevation	Mann n		
	1	0	565.23	.04		1	0	569.4	.04		1	0	568.73	.04		1	0	563.25	.04		
	2	2.64	565.27			2	9.58	569.53			2	5.02	568.76			2	3.23	563.32			
	3	5.66	565.25			3	15.61	569.67			3	8.36	568.76			3	6.25	563.16			
	4	8.67	565.2			4	27.68	570.01			4	11.38	568.78			4	10.87	562.67			
	5	11.74	565.27			5	30.69	569.88			5	14.4	568.83			5	15.3	562.15			
	6	14.38	565.43			6	33.71	569.53			6	17.41	568.94			6	18.32	561.9			
	7	17.11	565.44			7	34.24	569.51			7	20.43	568.95			7	19.06	561.88			
	8	17.72	565.42			8	36.01	569.39			8	23.45	568.86			8	20.77	561.78			
	9	26.77	564.93			9	36.73	569.37			9	26.46	568.67			9	24.35	561.7			
	10	29.79	564.9			10	46.93	569.87			10	32.01	568.44			10	27.24	561.52			
	11	32.81	564.83			11	51.81	570.08			11	35.62	568.26			11	30.38	561.25			
	12	33.48	564.77		-	12	54.83	569.89		•	12	38.53	568.15		-	12	36.42	560.61		-	
																	OK	Cano	el He	lp	

Figure 12. Internal and External cross sections editor with Manning's n values for all of the 1D cross sections.

After you have enter all of the bridge data and Manning's n values, close the editor and **SAVE THE GEOMETRY DATA**.

4. Make a new Plan using the "Single 2D Area with Bridge" Geometry.

Go to the **Unsteady Flow Analysis** window (**Run: Unsteady Flow Analysis**). Make a new Plan file using the **Save Plan As** option from the **File** menu under the **Unsteady Flow Analysis** editor. Label the Plan something like: "**2D Bridge Run**". Give the Plan a relevant short ID, like "**2D Bridge**". No special computational options will be needed for this run, as we will use the same computational options and time step that were used for the No Bridge plan. See the Unsteady Flow Analysis window below:

11

」」Unsteady Flow Analysis	×
<u>File</u> Options Help	
Plan: 2D Bridge	Short ID: 2D Bridge
Geometry File: Sing	gle 2D Area with Bridge 💌
Unsteady Flow File: Sing	jle Event 🗸
Programs to Run	an Description
Image: Construct of the second se	this example a single 2D flow area is used to model the attre system, including the Dam. The computational mesh as modyfied in the area of the dam to align the cell faces ong the top of the dam. A SA/2D Connection was added side of the 2D flow area to represent the Dam, including e top of dam, overflow spillway, and low flow gates. This
Simulation Time Window Starting Date: 01JAN Ending Date: 03JAN	1999 Starting Time: 1200 1999 Ending Time: 0000
Computation Settings	
Computation Interval: 5 Seco	nd 💌 Hydrograph Output Interval: 1 Minute 💌
Mapping Output Interval: 10 Min	ute 💌 Detailed Output Interval: 1 Hour 💌
Project DSS Filename: 💽 d: \HEC	Classes\RAS Classes\#352 Advanced 2D Modeling w
	debug parameters
]	
[Compute

Figure 13. Unsteady Flow Analysis window for 2D Bridge run.

6. Perform the Computations and Review the Model Output.

Run the new 2D Bridge model Plan. When the model has finished running, review the output for the new run by looking at the results in RAS-Mapper. Compare the no bridge to the 2D with Bridge plan by plotting a profile plot of the water surface elevations for the user defined profile line called "**Channel Centerline**". Also inspect the map view and animate the water surface elevations, as wells as looking at the velocities.

- 7. Answer the following Questions:
 - A. How do the results from the with and without Bridge Plans compare? What is the water surface difference between the two Plans upstream of the bridge?

B. What is the maximum difference in Water Surface from the headwater side (Upstream of Bridge) to the Tailwater side (downstream of bridge) for the 2D Bridge Plan? How did you find this answer? (i.e. what plot did you use).

C. Does the Bridge go into Pressure Flow?

D. Is the left roadway approach, bridge deck, or right roadway approach overtopped? Do you think that it is correct, given the terrain data? How could you make the model more accurate with respect to this question?