# Combined 1D River and 2D Floodplain/Levee Areas 

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## Overview

- Using a 2D Flow Area to model inside of Levees
- Saint Paul Levee Breach Example
- Using 2D Flow Areas to model overbank areas
(i.e. 1D Channels and 2D Floodplains)
- Carson River Example


## Using a 2D Flow Area to model inside of a Levee system

- Bring in terrain and background map layers into RAS Mapper
- Draw a Polygon for the 2D Flow Area Boundary Inside of the Levee
- Create the 2D Computational Mesh
- View the Mesh Boundary Cells to ensure there are no Mesh Problems
- Modify The Mesh if Needed (add break lines for roads, high ground, etc. Use mesh refinement regions to refine or coarsen areas of the mesh)
- Hook up the 2D Flow Area to a 1D River Reach with Lateral Structures
- Weir Coefficients for Lateral Structures
- Levee Breaching
- Weir and Levee Breach Submergence Issues

From HEC-RAS Mapper Create a Terrain Model and Map Layers


Draw a Polygon for the 2D Flow Area Boundary Inside of the Levee


## Create the 2D Computational Mesh using the 2D Flow Area Editor



View the Mesh to ensure there are no Mesh Problems


Modify The Mesh as Needed


Hooking up a 2D Flow Area to a 1D River Reach with Lateral Structures


Lateral Structure editor


Using Geospatial Coordinates for Lateral Structures


## Lateral Weir/Embankment Editor

| Lateral Weir Embankment |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| -Weir Data | -Embankment Station/Elevation Table |  |  |  |  |
| Weir Width k5. | Insert Row |  | Delete Row | Filter... |  |
| Weir Computations: Standard Weir Eqn $^{\text {a }}$ |  | Station |  |  | - |
| -Standard Weir Equation Parameters | 1 | 0 |  | 717.52 |  |
|  | 2 | 160.9 |  | 17.328 |  |
| Weir flow reference: Water Surface - | 3 | 217.23 |  | 16.861 |  |
| Weir Coefficient (Cd) 2. | 4 | 239.07 |  | 16.825 |  |
|  | 5 | 297.36 |  | 16.253 |  |
|  | 6 | 477.08 |  | 16.183 |  |
|  | 7 | 556.99 |  | 16.042 |  |
| Weir Crest Shape: Broad Crested $^{\text {a }}$ | 8 | 671.42 |  | 16.046 |  |
|  | 9 |  |  |  |  |
|  | 10 |  |  |  |  |
|  | 11 |  |  |  |  |
|  | 12 |  |  |  |  |
|  | 13 |  |  |  |  |
|  | 14 |  |  |  |  |
|  | 15 |  |  |  |  |
| -Weir Stationing Reference- | 16 |  |  |  |  |
| HW - Distance to Upstream XS: 23. | 17 |  |  |  |  |
|  | 18 |  |  |  |  |
|  | 19 |  |  |  |  |
|  | 20 |  |  |  |  |
|  | 21 |  |  |  |  |
| HW Connections ... TW Connections ... | 22 |  |  |  | - |
|  |  |  | K | Canc |  |

Lateral Weir Headwater Connections
(HW)

| HW Lateral Structure Connections |  |  |  | 4 |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| - Computed Default Weir Stationing |  |  |  | $\bigcirc$ User Defined Weir Stationing |  |  |  |
| Default Computed Weir Stationing |  |  |  | User Defined Weir Stationing |  |  |  |
|  | XS RSs | Weir Station | - |  | XS RSs | Weir Station | - |
| 1 | 151436.4 | -22.92 |  | 1 | 151354.9 | 5692 |  |
| 2 | 151354.9 | 156.84 |  | 2 | 151084.4 | 5909 |  |
| 3 | 151084.4 | 373.92 |  | 3 | 150654.0 | 6435 |  |
| 4 | 150654.0 | 803.62 |  | 4 |  |  |  |
| 5 |  |  |  | 5 |  |  |  |
| 6 |  |  |  | 6 |  |  |  |
| 7 |  |  |  | 7 | User Specifie | nnections |  |
| 8 |  |  |  | 8 | Option will n | used |  |
| 9 |  |  |  |  | because the | al structure |  |
| 10 |  |  |  | 10 | has a geo-re | ced |  |
| 11 |  |  |  | 11 | centerine. |  |  |
| 12 |  |  |  | 12 |  |  |  |
| 13 |  |  |  | 13 |  |  |  |
| 14 |  |  |  | 14 |  |  |  |
| 15 |  |  |  | 15 |  |  |  |
| 16 |  |  |  | 16 |  |  |  |
| 17 |  |  |  | 17 |  |  |  |
| 18 |  |  |  | 18 |  |  |  |
| 19 |  |  | $\checkmark$ | 19 |  |  | $\checkmark$ |
|  |  |  |  |  |  | Cance |  |

Lateral Weir Tailwater Connections (TW)

| TW Lateral Structure Connections |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| - Computed Default Weir Stationing |  |  |  | $\bigcirc$ User Defined Weir Stationing |  |  |  |
| Default Computed Weir Stationing |  |  |  | User Defined Weir Stationing |  |  |  |
|  | 2D Face Points | Weir Station | $\wedge$ |  | 2D Face Points | Weir Station | - |
| 1 | 456 | -34.3083 |  | 1 | 454 | 5470.98 |  |
| 2 | 412 | 28.59606 |  | 2 | 411 | 5537.3 |  |
| 3 | 368 | 174.9681 |  | 3 | 368 | 5703.08 |  |
| 4 | 319 | 314.8326 |  | 4 | 319 | 5861.57 |  |
| 5 | 322 | 479.7218 |  | 5 | 322 | 6048.15 |  |
| 6 | 325 | 521.3957 |  | 6 | 325 | 6095.38 |  |
| 7 | 2408 | 663.5559 |  |  | User Specified Co | nections |  |
| 8 | 239 | 725.6677 |  |  | Option will not be | used |  |
| 9 |  |  |  | 9 | because the later | structure |  |
| 10 |  |  |  | 10 | has a geo-referen |  |  |
| 11 |  |  |  | 11 | centerline. |  |  |
| 12 |  |  |  | 12 |  |  |  |
| 13 |  |  |  | 13 |  |  |  |
| 14 |  |  |  | 14 |  |  |  |
| 15 |  |  |  | 15 |  |  |  |
| 16 |  |  |  | 16 |  |  |  |
| 17 |  |  |  | 17 |  |  |  |
| 18 |  |  |  | 18 |  |  |  |
| 19 |  |  | $\checkmark$ | 19 |  |  | $\checkmark$ |
|  |  |  |  |  | OK | Cance |  |

Connected 1D River to 2D Flow Area with


## Weir Coefficients for Lateral Structures

| What is being modeled with <br> the Lateral Structure | Description | Range of Weir <br> Coefficients |
| :--- | :--- | :---: |
| Levee/Roadway - 3ft or <br> higher above natural ground | Broad crested weir shape, flow <br> over Levee/road acts like weir flow | $\mathbf{1 . 5}$ to $\mathbf{2 . 6}$ (2.0 default) <br> SI Units: 0.83 to 1.43 |
| Levee/Roadway - 1 to 3 ft <br> elevated above ground | Broad Crested weir shape, flow <br> over levee/road acts like weir flow, <br> but becomes submerged easily. | $\mathbf{1 . 0}$ to $\mathbf{2 . 0}$ <br> SI Units: 0.55 to 1.1 |
| Natural high ground barrier - <br> 1 to 3 ft high | Does not really act like a weir, but <br> water must flow over high ground <br> to get into 2D area. | $\mathbf{0 . 5}$ to $\mathbf{1 . 0}$ <br> SI Units: 0.28 to 0.55 |
| Non elevated overbank <br> terrain. Lat Structure not <br> elevated above ground | Overland flow escaping the main <br> river. | $\mathbf{0 . 2}$ to 0.5 <br> SI Units: 0.11 to 0.28 |

## Levee Breaching



Simplified Physical Breaching


## Velocity vs. Downcutting and Widening



## Weir and Levee Breach Submergence Issues

- When a lateral structure gets highly submerged, HEC-RAS uses a weir submergence curve to compute the flow reduction over the weir. The curve is very steep (i.e. the flow reduction changes dramatically) between $95 \%$ and $100 \%$ submergence. This can cause oscillations and possible model stability issues. To reduce these oscillations, user can have HEC-RAS use a milder sloping submergence curve by going to the 1D "Computational Options and Tolerances" and setting the field labeled "Weir flow submergence decay exponent" to 3.0.


## Weir Submergence Curves



## Unsteady Flow Computational Options and Tolerances



Using RAS-Mapper
Associate the Terrain to the Geometry


RAS-Mapper
Running the 2D Pre-Processor


## Run the Model and View the Results

Saint Paul Levee Breach Example


- $x$

Lateral Structure Time Series Output


## Lateral Structure Detailed Output



Stage Hydrograph Plots from RASMapper


Velocity Hydrograph Plots from RASMapper


## Using 2D Flow Areas to model Overbank Areas (floodplains)

- Draw a Polygon for the Overbank/Floodplain Area
- The 2D Flow Area boundary should be drawn at a High Ground Separation between the 1D Main Channel and 2D Floodplain
- Create the 2D Computational Mesh
- View the Mesh to ensure there are no Mesh Problems
- Modify The Mesh if Needed (add break lines, points, etc...)
- Hooking up a 2D Flow Area to a 1D River Reach with Lateral Structures
- Overflow Computation Method
- Weir Coefficients for Lateral Structures
- Weir Submergence Issues

1D Channel to 2D Interface Should be at High


## Terrain Contours



Lateral Structure to connect 1D river to 2D overbank areas


Overflow Computation Method


## Weir Coefficients for Lateral Structures

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| Levee/Roadway -1 to 3 ft <br> elevated above ground | Broad Crested weir shape, flow <br> over levee/road acts like weir flow, <br> but becomes submerged easily. | $\mathbf{1 . 0}$ to 2.0 <br> SI Units: 0.55 to 1.1 |
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| Non elevated overbank <br> terrain. Lat Structure not <br> elevated above ground | Overland flow escaping the main <br> river. | $\mathbf{0 . 2}$ to $\mathbf{0 . 5}$ |

## Questions?

