

# Dam Breach Parameters

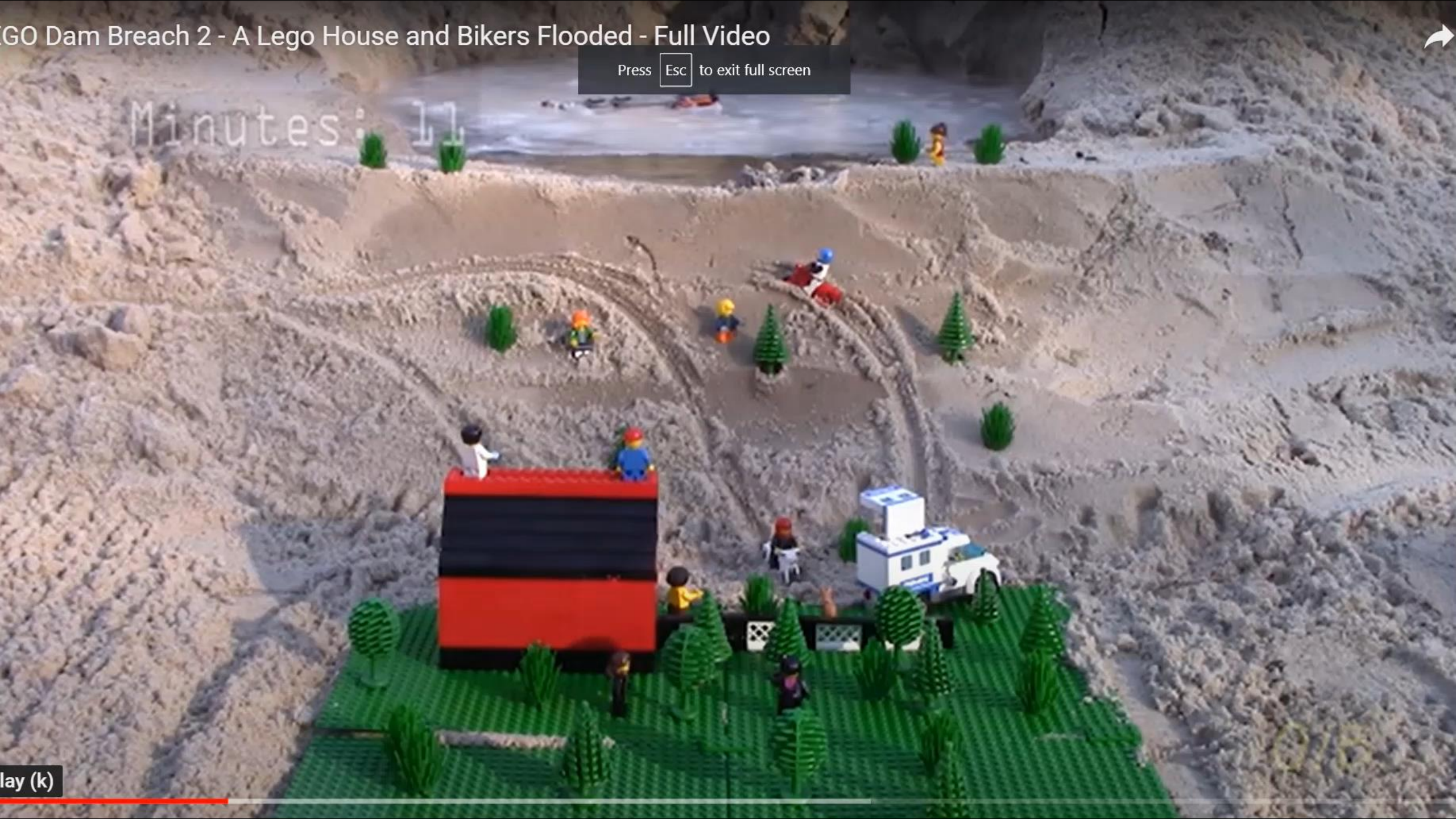


Stanford Gibson, PhD

# GO Dam Breach 2 - A Lego House and Bikers Flooded - Full Video

Press Esc to exit full screen

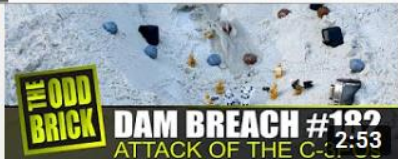
Minutes: 11





LEGO Dam Breach #183 - Construction Crew Surprise

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LEGO Dam Breach #181 - Will Woody be Saved from the...

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LEGO Dam Breach #180 - These are Not the Droids You...

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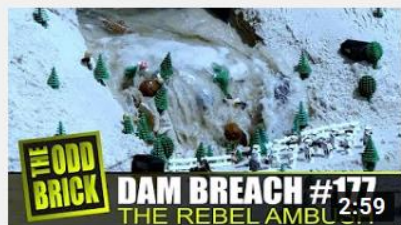
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LEGO Dam Breach 177 - The Rebel Ambush

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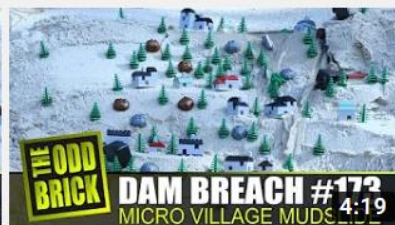
LEGO Dam Breach #175 - The Fantastic T-Rex

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LEGO Dam Breach #174 - The A-Wing Base

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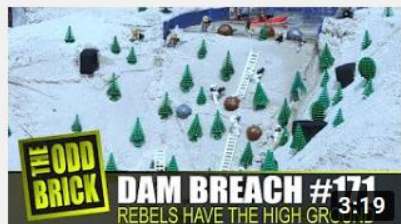
LEGO Dam Breach #173 - Micro Village Mudslide

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LEGO Dam Breach #171 - Rebels Have the High Ground

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LEGO Dam Breach #170 - Sand Castle Battle

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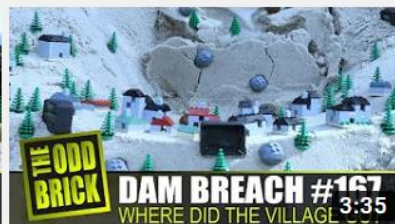
LEGO Dam Breach #169 - Will the House Survive This...

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LEGO Dam Breach #168 - Dinosaur Extinction

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LEGO Dam Breach #167 - Where did the Village Go?

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LEGO Dam Breach #166 - Sand Castle Collapse

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LEGO Dam Breach #164 - Raptors in Fantastic Beasts?



LEGO Dam Breach #163 - Piggy Farm Returns



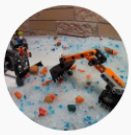
LEGO Dam Breach #162 - Grindelwald's Escape



LEGO Slow Motion #02 - Monster Truck Crashes



LEGO Dam Breach #161 - A House Washes Away



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Epic Attempt Of The Lego Minifigures To Stop The Leg...

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LEGO Dam Breach: LEGO City Explore New Big Sand Castle!

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LEGO DAM BREACH - MINE FLOODING AND COLLAPSE

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LEGO DAM BURST, FLOODING SECRET...

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1.8M views • 9 months ago



LEGO SAND WALL BREAK

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Lego Dam Breach Airport

1.7M views • 1 year ago



LEGO DAM BREACH - DESTRUCTION OF THE SAN...

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Dam Breach - LEGO Construction Site

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LEGO DAM BREACH - BRIDGE COLLAPSE

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LEGO Dam Breach - Dinosaur Attack LEGO Minifigures on...

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LEGO Dam Breach SAND CASTLE

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LEGO DAM BREACH - CITY COLLAPSE

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# NRCS Embankment Failure Research

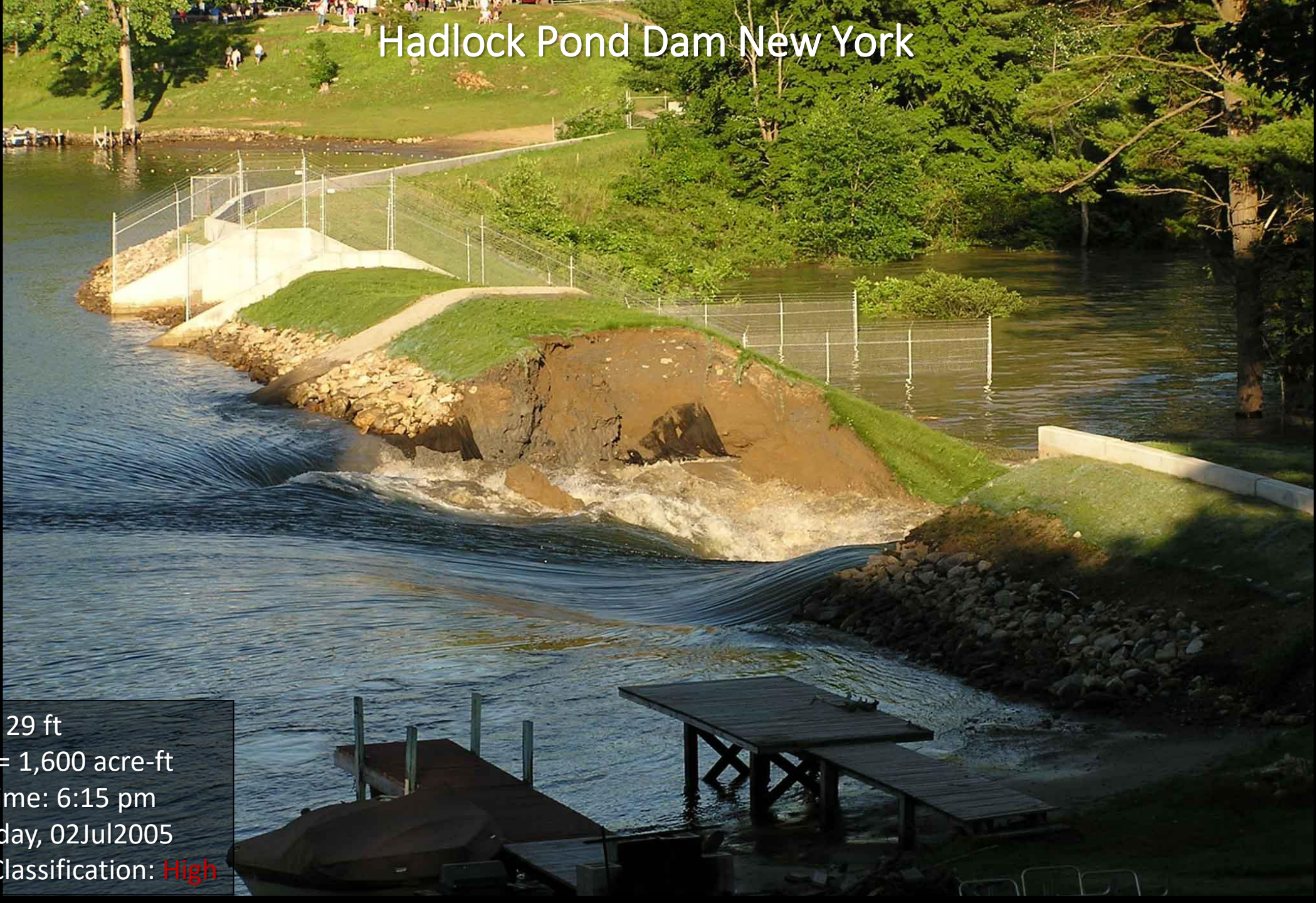
(SIMBA/WinDAMB model)

Temple, Darrel M., and Hanson, Gregory J., "Earth Dam Overtopping and Breach Outflow," Presented at the ASCE World Water & Environmental Resources Congress 2005, EWRI, Anchorage, AK, 15-19 May 2005.



# Hadlock Pond Dam New York

Height = 29 ft  
Storage = 1,600 acre-ft  
Failure time: 6:15 pm  
Saturday, 02Jul2005  
Hazard Classification: High



# Taum Sauk Upper Dam, Missouri

Height = 94 ft

Storage = 1,600 acre-ft

Failure time: 5:20 am

Wednesday, 14Dec2005

Hazard Classification: **High**

USGS

LMR

14Dec2005



# Herald-Zeitung



## Dunlap Dam Failure

Guadalupe Blanco River Authority May 15, 2019 1

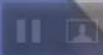


05-14-2019 08:05:46

Press  to exit full screen

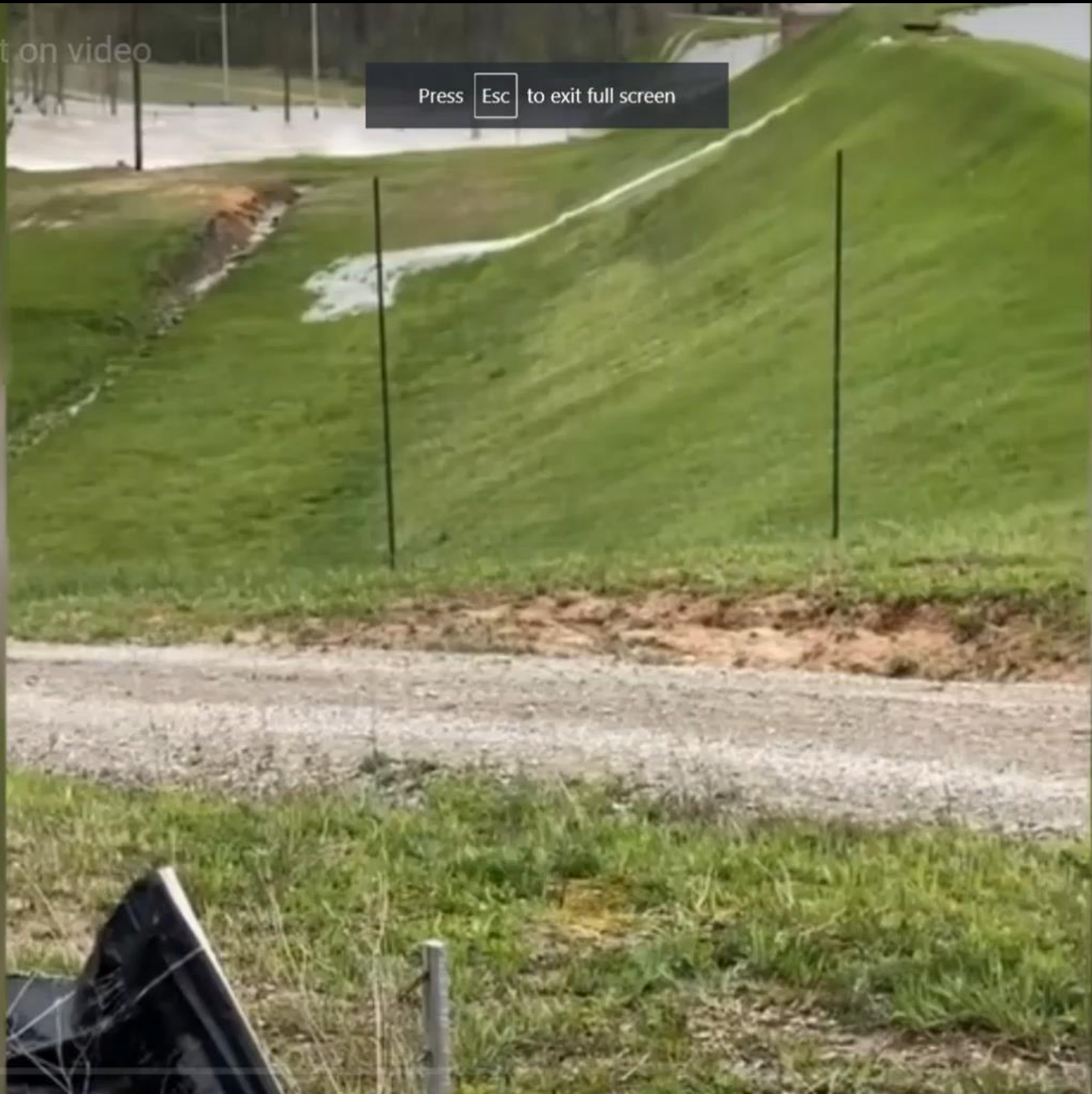


Dunlap\_Dam



Michigan dam failure caught on video

Press Esc to exit full screen



# 1. User Entered Data

## -Parameter Estimation

User Entered Data

# 2. Simplified Physical

Simplified Physical

# 3. DL Breach

Physical Breaching (DLBreach)

User Entered Data

Simplified Physical

Physical Breaching (DLBreach)

# 1. User Entered Data

## -Parameter Estimation

User Entered Data

# 2. Simplified Physical

Simplified Physical

# 3. DL Breach

Physical Breaching (DLBreach)

User Entered Data

Simplified Physical

Physical Breaching (DLBreach)

# HEC-RAS Breach Data

Dam (Inline Structure) Breach Data

Inline Structure: **Bald Eagle Cr. Lock Haven 81454** [Delete this Breach ...] [Delete all Breaches ...]

☒ **Breach This Structure**

Breach Method: **User Entered Data**

Center Station: 5250

Final Bottom Width: 446

Final Bottom Elevation: 585

Left Side Slope: 0.9

Right Side Slope: 0.9

Breach Weir Coef: 2.6

Breach Formation Time (hrs): 3.2

Failure Mode: **Piping**

Piping Coefficient: 0.5

Initial Piping Elev: 620

Trigger Failure at: **WS Elev**

Starting WS: 676.8

Breach Plot | Breach Progression | Simplified Physical | Physical Breaching (DLBreach) | Parameter Calculator | Breach Repair (optional)

**User Entered Data**

Simplified Physical  
Physical Breaching (DLBreach)

**Legend**

- Ground
- Bank Sta
- Final Breach

Elevation (ft)

Station (ft)

OK Cancel

Which of these parameters are hard?

# HEC-RAS Breach Data Input

- Location: Centerline of breach
- Type: Overtopping or Piping
- Size: Bottom elevation, **width** and side slopes
- **Time**: to maximum size and progression type
- Triggering situation:
  - ✓ Pool elevation
  - ✓ Pool elevation + Duration
  - ✓ Clock time

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Starting WS: 676.8

Before we get into the tough parameters...lets take on some of these others.

## Dam (Inline Structure) Breach Data

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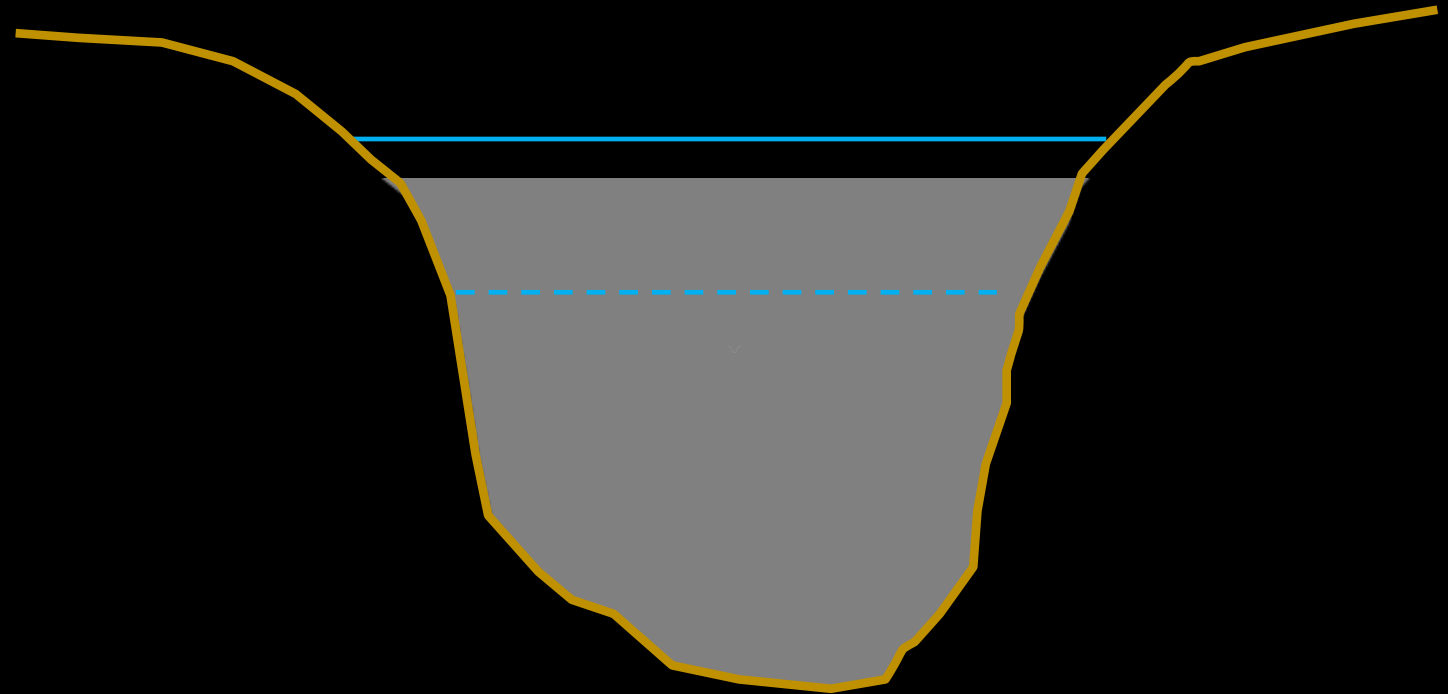
Piping Coefficient: 0.5

Initial Piping Elev: 620

Trigger Failure at: WS Elev

Starting WS 676.8

But First...A Couple Other Features



## Dam (Inline Structure) Breach Data

Inline Structure **Bald Eagle Cr. Lock Haven**

☒ **Breach This Structure**

Breach Method:

User Entered Data

Center Station: 5250

Final Bottom Width: 446

Final Bottom Elevation: 585

Left Side Slope: 0.9

Right Side Slope: 0.9

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Breach Formation Time (hrs): 3.2

Failure Mode: Piping

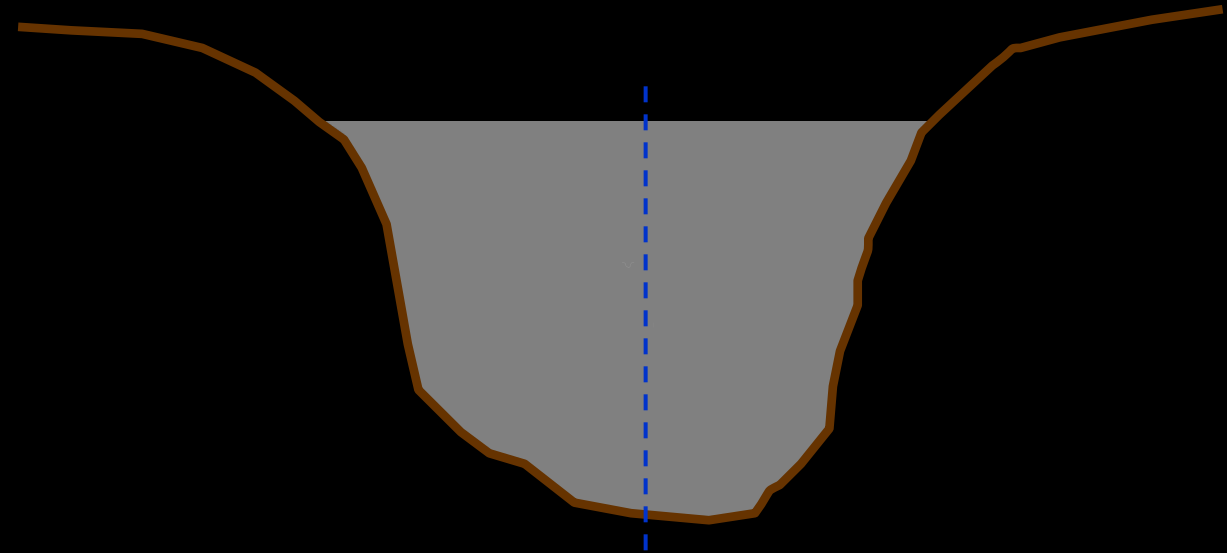
Piping Coefficient: 0.5

Initial Piping Elev: 620

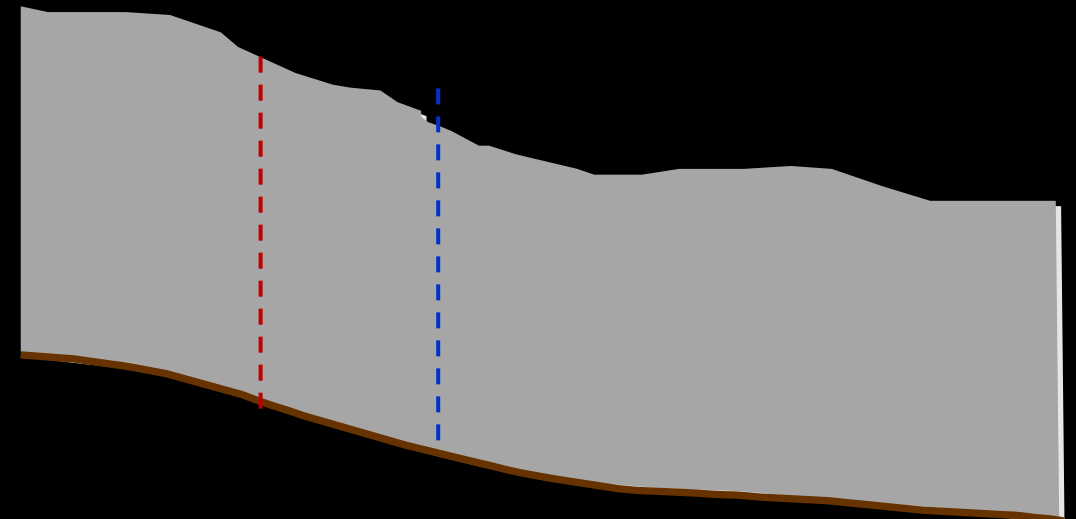
Trigger Failure at: WS Elev

Starting WS 676.8

This doesn't matter much for a dam



But levees can be more complicated



Where?

# 1. User Entered Data

-Parameter Estimation

User Entered Data

# 2. Simplified Physical

Simplified Physical

# 3. DL Breach

Physical Breaching (DLBreach)

User Entered Data

Simplified Physical

Physical Breaching (DLBreach)

# Estimating the Breach Parameters

- **Literature And Guidance**

- Existing COE guidance
- Prediction of embankment dam breach parameters: *USBR (1998) Dam Safety Research Report*

- **Empirical Methods -**

- MacDonald and Langridge-Monopolis (1984)
- Froehlich (1995b)
- Von Thun and Gillette (1990)
- Xu and Zhang (2009)

# USBR

## Technology Review (1998)

### Prediction of Embankment Dam Breach Parameters

*A Literature Review and Needs Assessment*

**DSO-98-004**



Water Resources Research Laboratory

July 1998

# Suggested Breach Parameters

Where: HD = Height of the dam.

L = Length of the dam crest.

Dam Type	Average Breach Width ( $B_{ave}$ )	Horizontal Component of Breach Side Slope (H) (H:V)	Failure Time, $t_f$ (hours)	Agency
Earthen/Rockfill	(0.5 to 3.0) x HD	0 to 1.0	0.5 to 4.0	USACE 1980
	(1.0 to 5.0) x HD	0 to 1.0	0.1 to 1.0	FERC
	(2.0 to 5.0) x HD	0 to 1.0 (slightly larger)	0.1 to 1.0	NWS
	(0.5 to 5.0) x HD*	0 to 1.0	0.1 to 4.0*	USACE 2007
Concrete Gravity	Multiple Monoliths	Vertical	0.1 to 0.5	USACE 1980
	Usually $\leq 0.5 L$	Vertical	0.1 to 0.3	FERC
	Usually $\leq 0.5 L$	Vertical	0.1 to 0.2	NWS
	Multiple Monoliths	Vertical	0.1 to 0.5	USACE 2007
Concrete Arch	Entire Dam	Valley wall slope	$\leq 0.1$	USACE 1980
	Entire Dam	0 to valley walls	$\leq 0.1$	FERC
	(0.8 x L) to L	0 to valley walls	$\leq 0.1$	NWS
	(0.8 x L) to L	0 to valley walls	$\leq 0.1$	USACE 2007
Slag/Refuse	(0.8 x L) to L	1.0 to 2.0	0.1 to 0.3	FERC
	(0.8 x L) to L		$\leq 0.1$	NWS

**\*Note:** Dams that have very large volumes of water, and have long dam crest lengths, will continue to erode for long durations (i.e., as long as a significant amount of water is flowing through the breach), and may therefore have longer breach widths and times than what is shown in Table 3. HD = height of the dam; L = length of the dam crest; FERC - Federal Energy Regulatory Commission; NWS - National Weather Service

# Breach Parameter Calculator

Dam (Inline Structure) Breach Data

Inline Structure: Nittany River Weir Reach 41.75

☐ Breach This Structure

Breach Method: User Entered Data

Center Station: 7150

Final Bottom Width: 912

Final Bottom Elevation: 5523

Left Side Slope: 0.5

Right Side Slope: 0.5

Breach Weir Coef: 2.6

Breach Formation Time (hrs): 2.92

Failure Mode: Overtopping

Piping Coefficient: 0.5

Initial Piping Elev:

Trigger Failure at: WS Elev

Starting WS: 5639.5

Breach Plot | Breach Progression | Simplified Physical | Physical Breaching (DLBreach) | **Parameter Calculator**

Input Data

Top of Dam Elevation (ft): 5644.5

Pool Elevation at Failure (ft): 5639.5

Breach Bottom Elevation (ft): 5523

Pool Volume at Failure (acre-ft): 240000

Failure mode: Overtopping

MacDonald

Dam Crest Width (ft): 50

Earth Fill Type: Non-homogeneous or Rockfill

Slope of US Dam Face Z1 (H:V): 2.6

Slope of DS Dam Face Z2 (H:V): 2.6

Xu Zhang (and Von Thun)

Dam Type: Dam with corewall

Dam Erodibility: Medium

Method	Breach Bottom Width (ft)	Side Slopes (H:V)	Breach Development Time (hrs)	
MacDonald et al	912	0.5	2.92	Select
Froehlich (1995)	675	1.4	3.04	Select
Froehlich (2008)	562	1	2.60	Select
Von Thun & Gillete	411	0.5	0.96	Select
Xu & Zhang	575	1.08	4.85 *	Select

\* Note: the breach development time from the Xu Zhang equation includes more of the initial erosion period and post erosion than what is used in the HEC-RAS breach formation time.

OK Cancel

# MacDonald and Langridge-Monopolis (1984)

## Earthfill

$$V_{eroded} = 0.0216 * (V_{out} * h_w)^{0.769}$$

$$t_f = 0.0179 * (V_{eroded})^{0.364}$$

$$BREACH_{size} = f(V_{eroded})$$

## Non-earthfill

$$V_{eroded} = 0.00348 * (V_{out} * h_w)^{0.852}$$

Input Data			
Top of Dam Elevation (ft):	<input type="text" value="5644.5"/>	Breach Bottom Elevation (ft):	<input type="text" value="5523"/>
Pool Elevation at Failure (ft):	<input type="text" value="5639.5"/>	Pool Volume at Failure (acre-ft):	<input type="text" value="240000"/>
Failure mode:			<input type="text" value="Overtopping"/>

$h_w$  = Pool Elev – Breach Bottom Elev

$V_{out}$  = Pool Volume at Failure

(but is total flow volume – including inflow)

# Von Thun & Gillette (1990)

$$B_{avg} = 2.5h_w + C_b$$

$$t_f = 0.02 h_w + 0.25 \text{ (erosion resistant)}$$

$$t_f = 0.015 h_w \text{ (easily erodible)}$$

Reservoir Size, m <sup>3</sup>	C <sub>b</sub> , meters
< 1.23*10 <sup>6</sup>	6.1
1.23*10 <sup>6</sup> - 6.17*10 <sup>6</sup>	18.3
6.17*10 <sup>6</sup> - 1.23*10 <sup>7</sup>	42.7
> 1.23*10 <sup>7</sup>	54.9

Reservoir Size, acre-feet	C <sub>b</sub> , feet
< 1,000	20
1,000-5,000	60
5,000-10,000	140
>10,000	180

## Input Data

Top of Dam Elevation (ft):

5644.5

Breach Bottom Elevation (ft):

5523

Pool Elevation at Failure (ft):

5639.5

Pool Volume at Failure (acre-ft):

240000

Failure mode:

Overtopping



$h_w$  = Pool Elev – Breach Bottom Elev

$V_{out}$  = Pool Volume at Failure

Froehlich (1995)

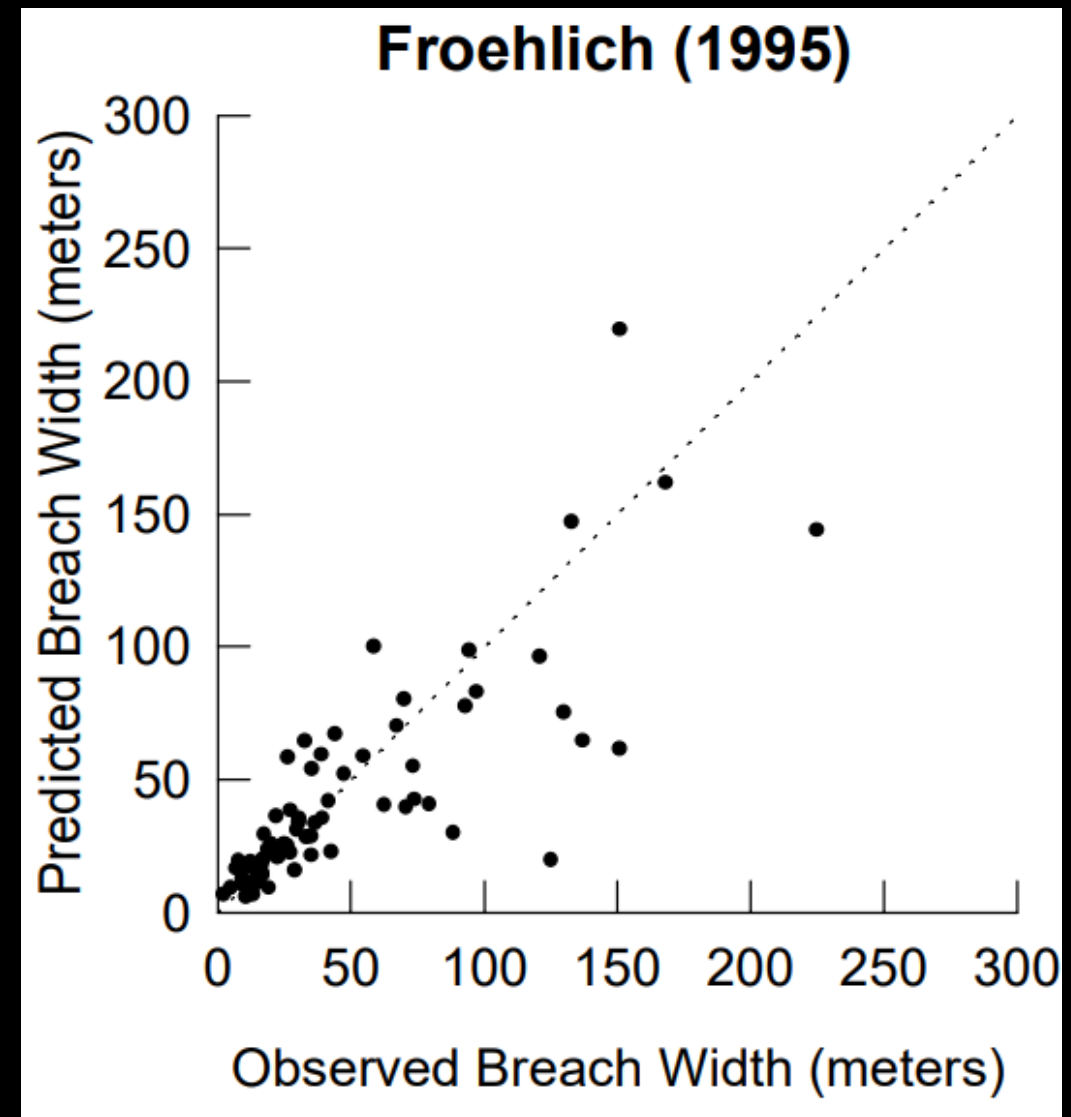
$$B_{avg} = 0.1803 K_o V_w^{0.32} h_b^{0.19}$$

$$t_f = 0.00254 V_w^{0.53} h_b^{-0.90}$$

Froehlich (2008)

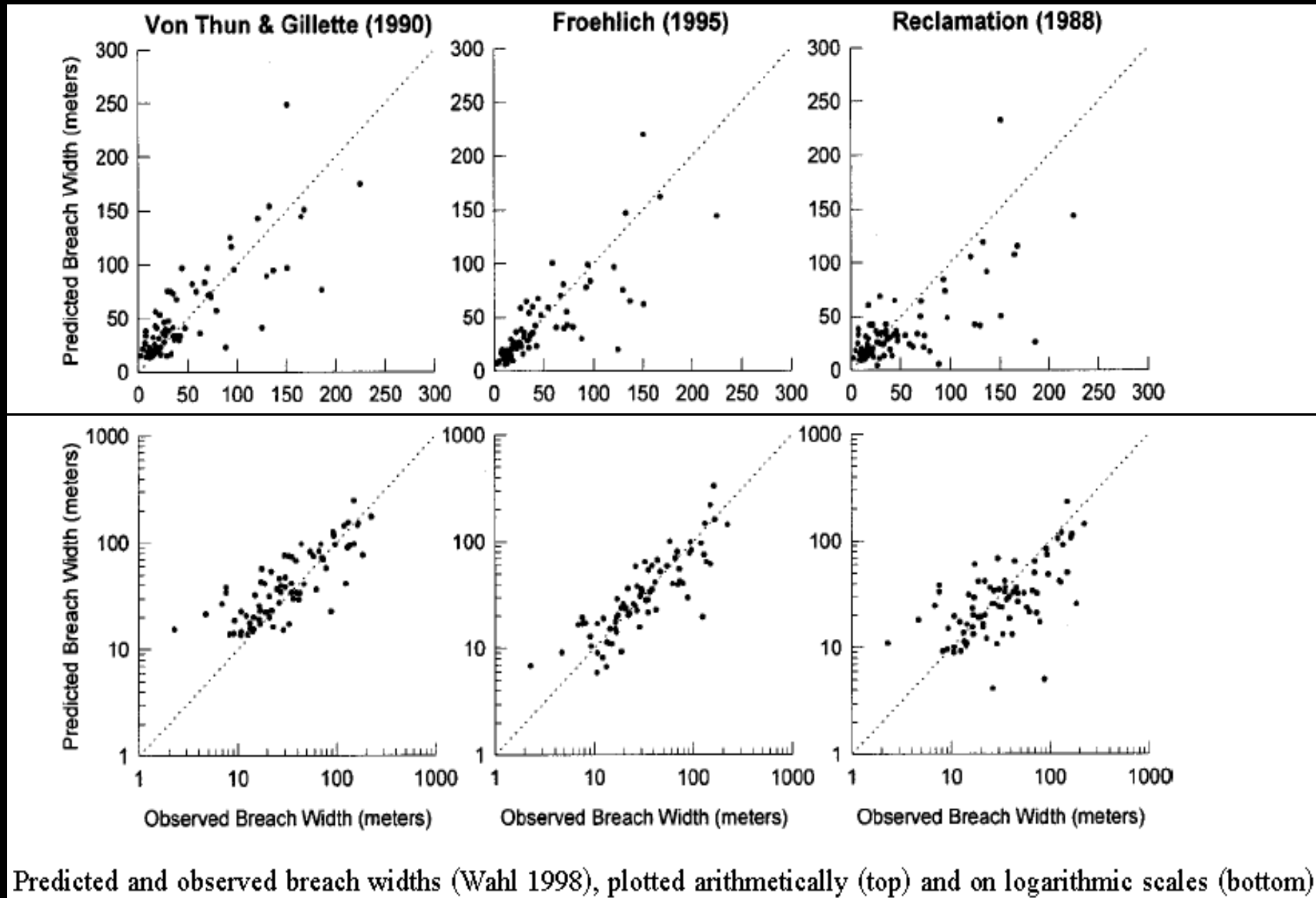
$$B_{avg} = 0.27 K_o V_w^{0.32} h_b^{0.04}$$

$$t_f = 20.18 V_w^{0.5} h_b^{-1.0}$$



$V_w$  = Volume of water at h

# Width Comparison



# Four Important Ideas

1. Do not mix-and-match width and breach time.

Method	Breach Bottom Width (ft)	Side Slopes (H:V)	Breach Development Time (hrs)	
MacDonald et al	122	0.5	2.57	Select
Froehlich (1995)	628	1.4	3.44	Select
Froehlich (2008)	544	1	3.04	Select
Von Thun & Gillete	363	0.5	0.81	Select
Xu & Zhang	499	1.06	5.05 *	Select

# Four Important Ideas

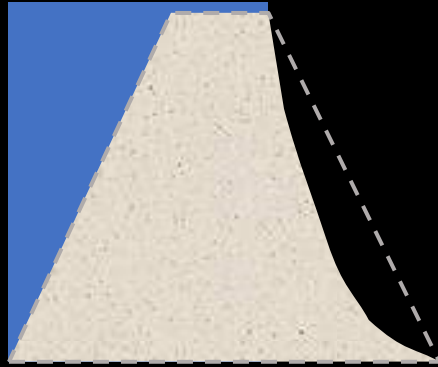
1. Do not mix-and-match width and breach time.
2. Xu & Zang has a different breach development time.

Method	Breach Bottom Width (ft)	Side Slopes (H:V)	Breach Development Time (hrs)	
MacDonald et al	122	0.5	2.57	Select
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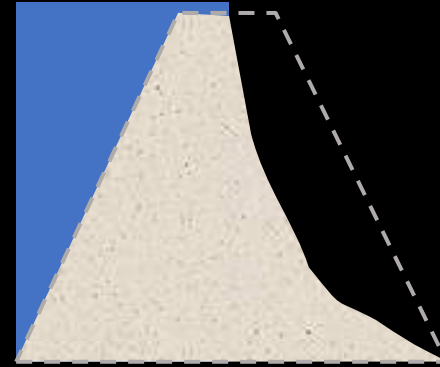
# Thought Experiment

When does the clock start for “breach time” in HEC-RAS?

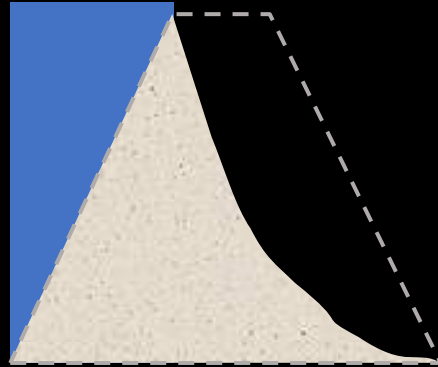
1.)



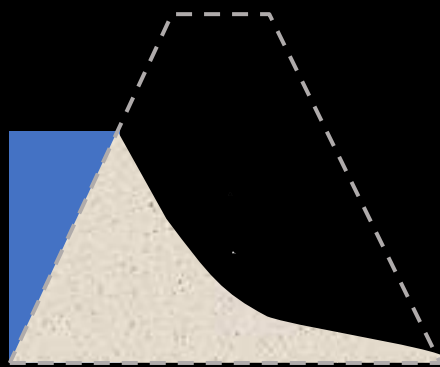
2.)



3.)



4.)



# Four Important Ideas

1. Do not mix-and-match width and breach time.
2. Xu & Zang has a different breach development time.
3. Does the breach progression make physical sense?
  - Does it keep eroding at low head?
  - Does it stop eroding with despite high head and velocity?

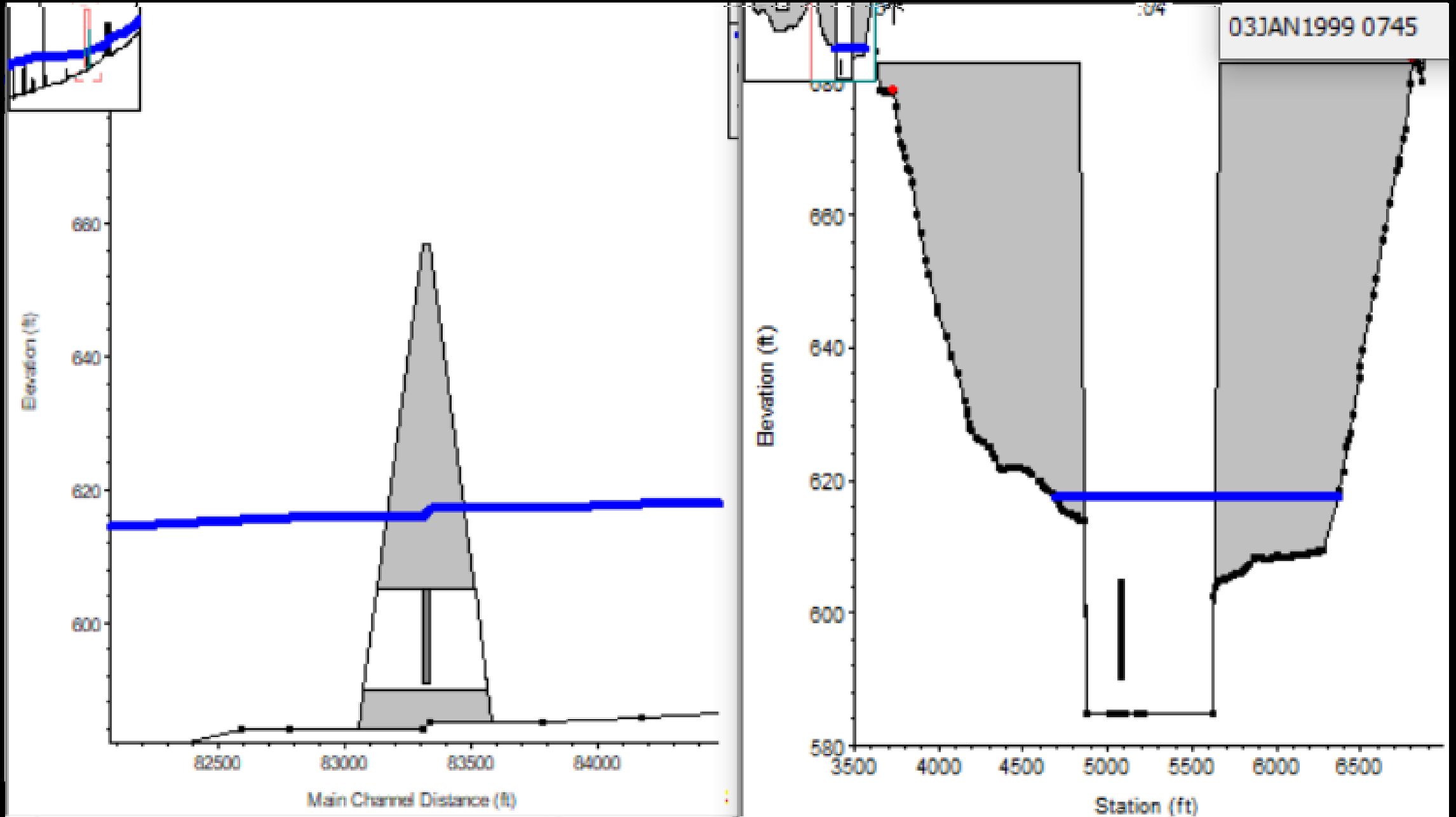
# Evaluate Breach Progress

- Breach parameter estimation is uncertain.

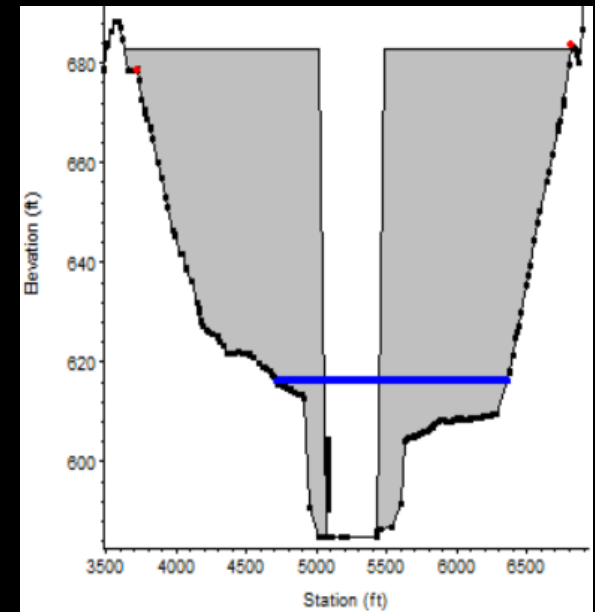
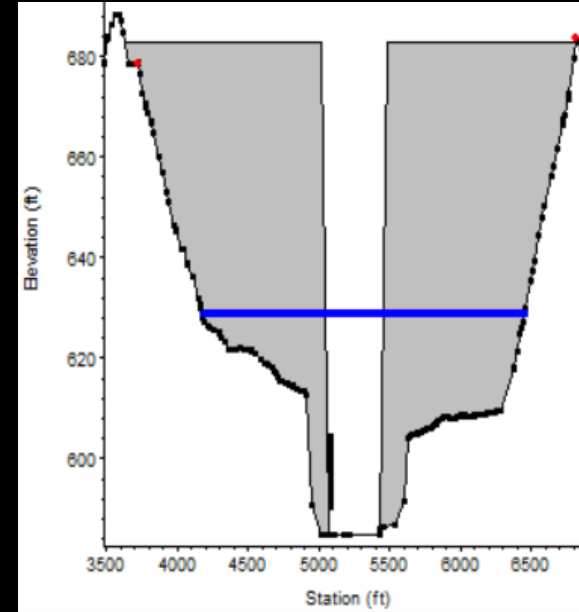
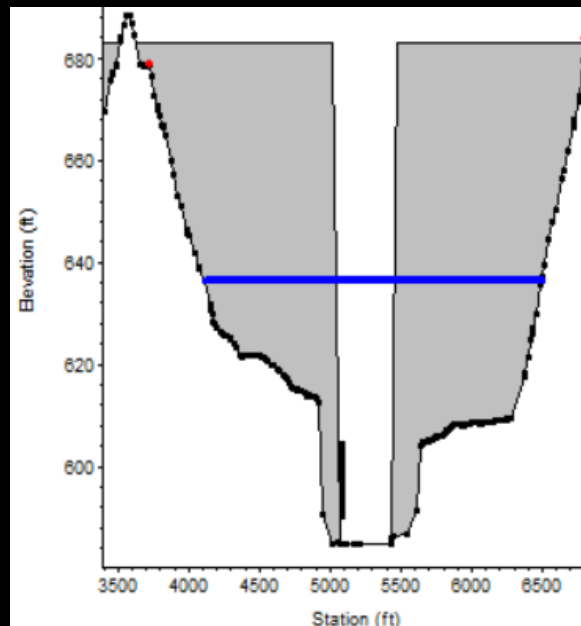
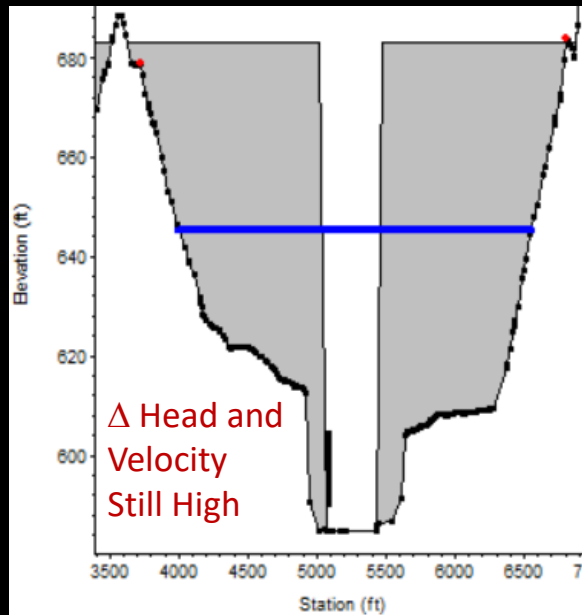
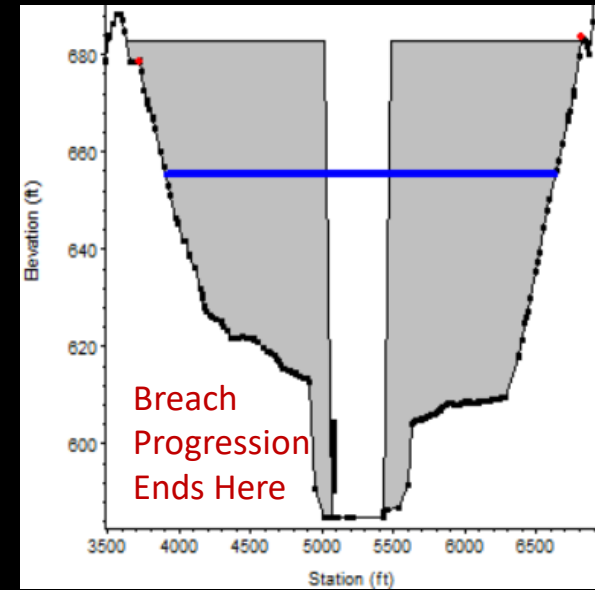
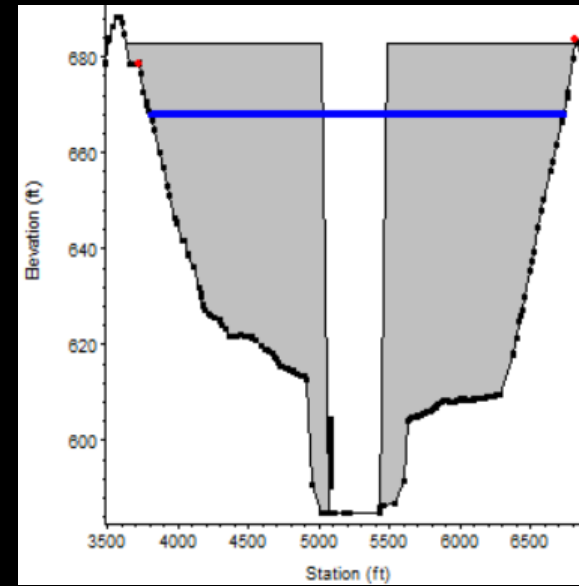
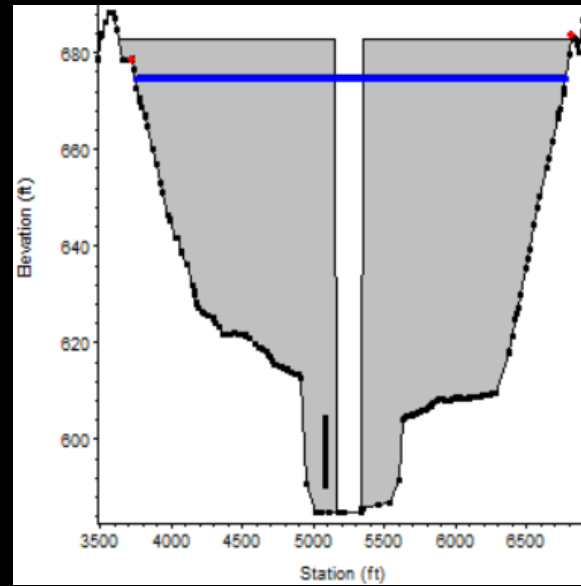
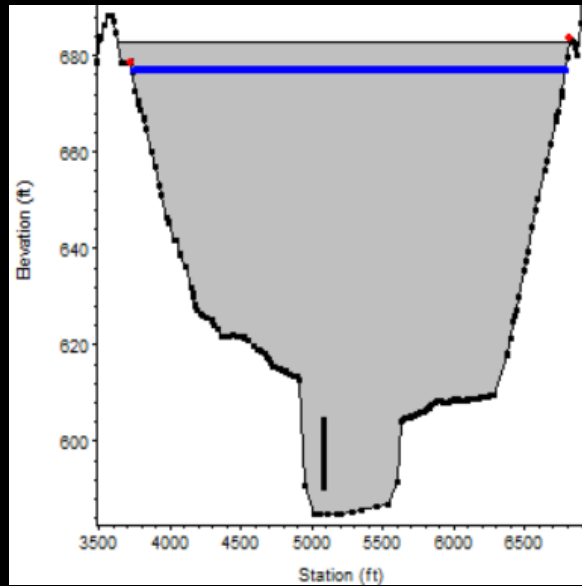
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Froehlich (2008)	544	1	3.04	Select
Von Thun & Gillete	363	0.5	0.81	Select
Xu & Zhang	499	1.06	5.05 *	Select

- We can use HEC-RAS hydraulic results to inform our decision on breach parameter selection.

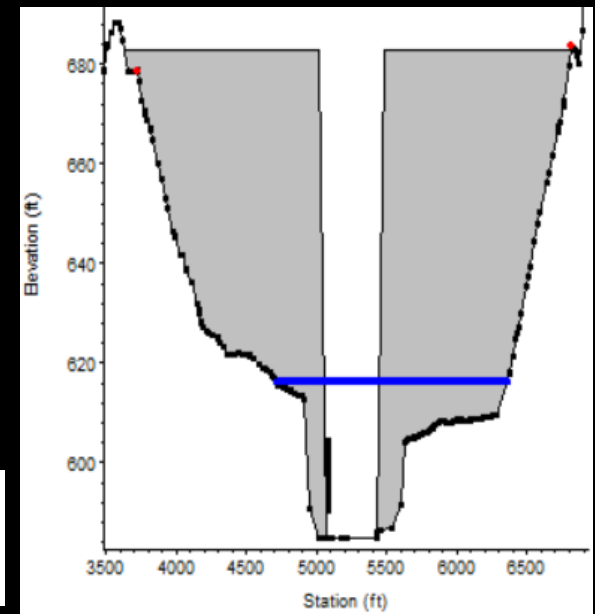
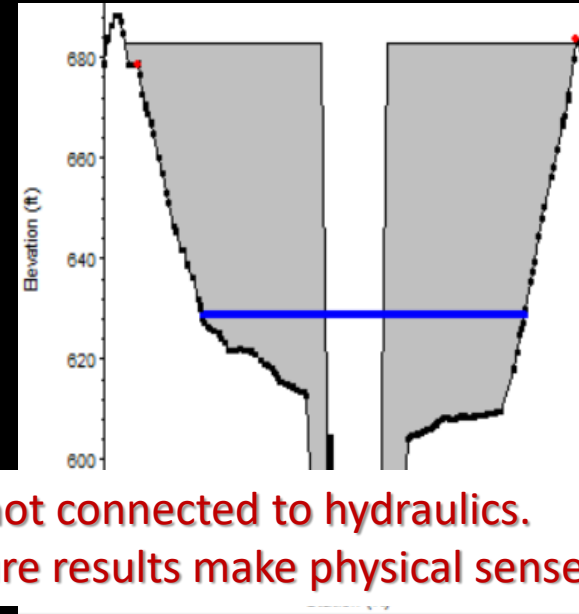
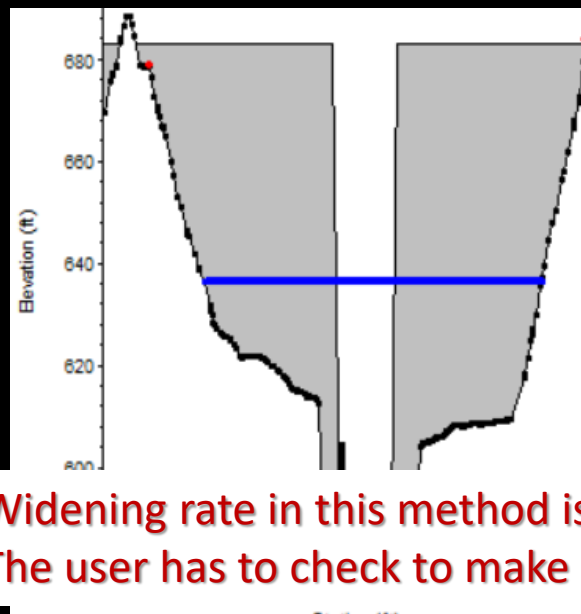
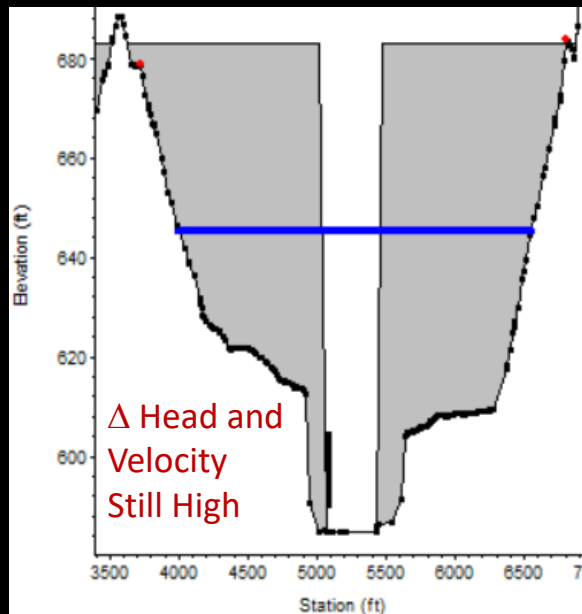
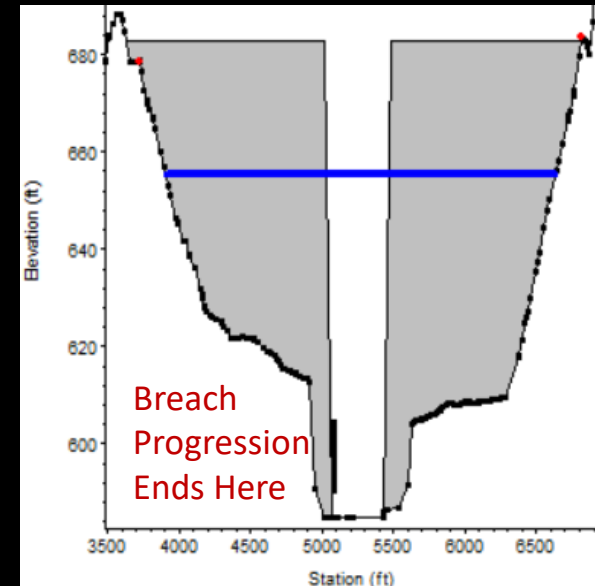
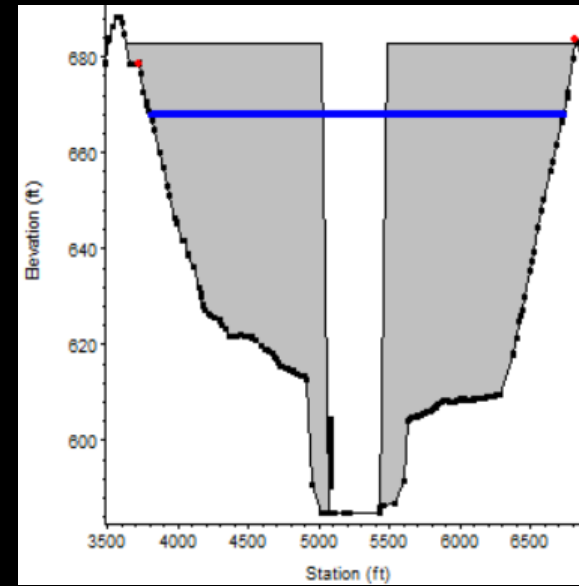
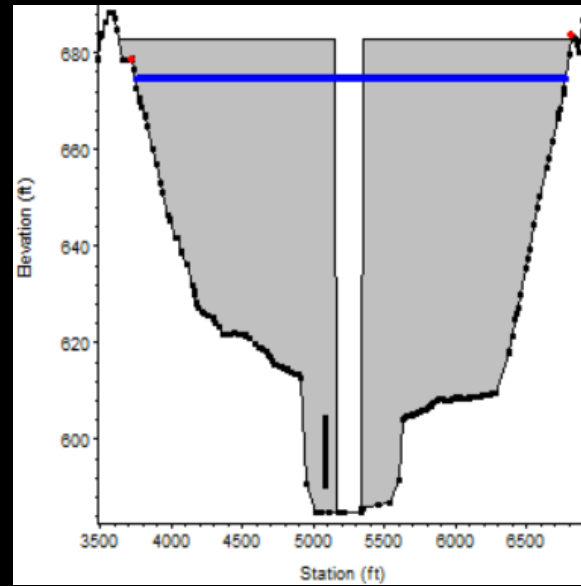
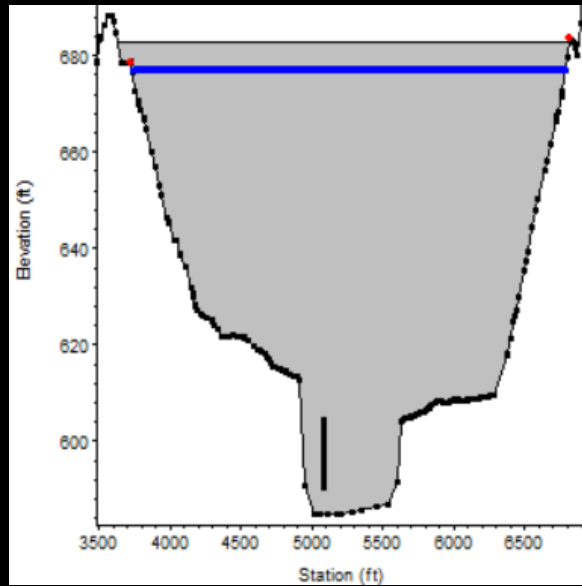
# Profile and Breach Plots



# Do you have any concerns about this simulation?



# This Simulation Continues to Widen for High $\Delta$ Head and Velocities



Breach  
Progression  
Ends Here

$\Delta$  Head and  
Velocity  
Still High

Widening rate in this method is not connected to hydraulics.  
The user has to check to make sure results make physical sense.

# 1. User Entered Data

## -Parameter Estimation

User Entered Data

# 2. Simplified Physical

Simplified Physical

# 3. DL Breach

Physical Breaching (DLBreach)

User Entered Data

Simplified Physical

Physical Breaching (DLBreach)

# Dam (Inline Structure) Breach Data

Inline Structure Bald Eagle Loc Hav 81500



Delete this Breach ...

Delete all Breaches ...

☒ Breach This Structure

Breach Method: Simplified Physical

Center Station: 3900

Max Possible Bottom Width: 1800

Min Possible Bottom Elev: 592

Left Side Slope: 2

Right Side Slope: 2

Breach Weir Coef: 2.6

Breach Formation Time (hrs): 1

Failure Mode: Piping

Piping Coefficient: 0.6

Initial Piping Elev: 620

Initial Piping Diameter: 1

☐ Mass Wasting Feature:

Trigger Failure at: WS Elev

Starting WS 668.1

Breach Plot | Breach Progression | Simplified Physical

Breach Repair (optional) | Parameter Calculator

Overtopping Downcutting

	Velocity (ft/s)	Downcutting Rate (ft/hr)
1	0	0
2	2	0
3	3	1
4	4	2
5	5	5
6	7	15
7	10	30
8	20	50
9	30	100
10		
11		
12		
13		
14		
15		
16		
17		
18		
19		
20		
21		
22		
23		
24		
25		

Widening Relationship

	Velocity (ft/s)	Widening Rate (ft/hr)
1	0	0
2	2	0
3	3	1
4	4	2
5	5	5
6	7	15
7	10	30
8	20	50
9	30	100
10		
11		
12		
13		
14		
15		
16		
17		
18		
19		
20		
21		
22		
23		
24		
25		

OK

Cancel

# Where do these rates come from?



Prepared by:  
**MMC MCX**

## Appendix 3.X.X – Application of Simplified Physical Breach Method in HEC-RAS

FY17 MMC Technical Manual SOP

July 2018

# Draft

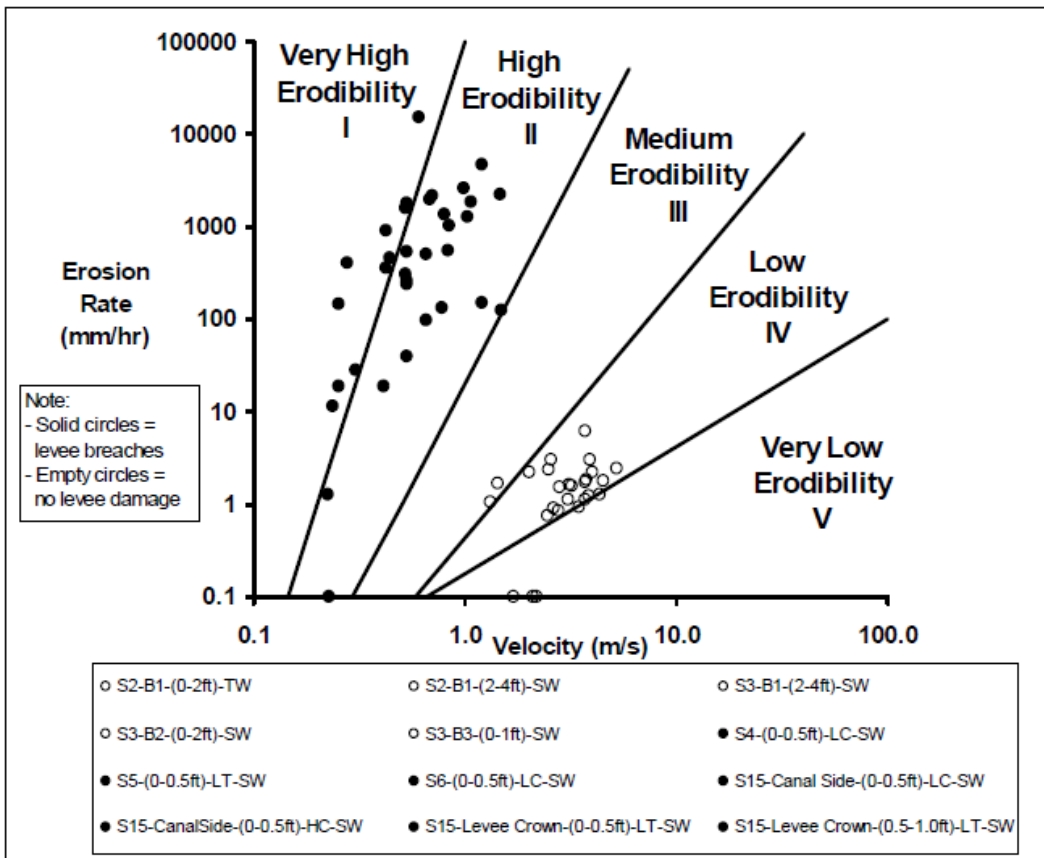
### FOR OFFICIAL USE ONLY (FOUO)

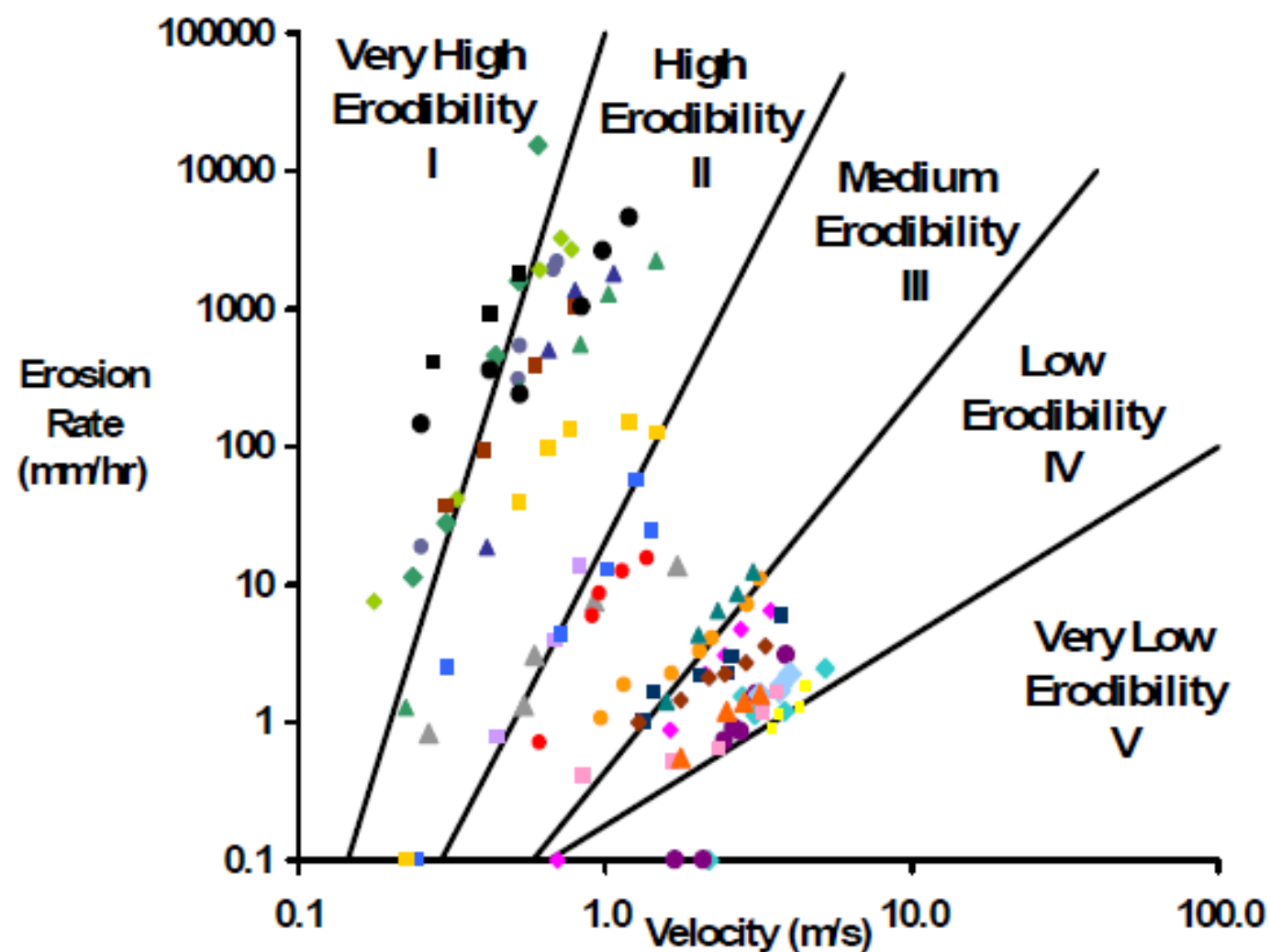
This document is FOR OFFICIAL USE ONLY (FOUO). It contains information that may be exempt from public release under the Freedom of Information Act (5 USC 552). It is to be controlled, stored, handled, transmitted, distributed, and disposed of in accordance with USACE policy relating to FOUO information and it is not to be released to the public or other personnel who do not have a valid "need to know" without prior written approval of an authorized USACE official.

The guidance is under development.  
There are some historic values that can help.

## Technical Memorandum for Record

**Date:** May 31, 2013 (Revised July 2, 2013)  
**From:** Chris Bahner, P.E., D. WRE  
**Subject:** Updated Levee Breach Characteristics for MMC SOP





◆ S1-B1-(0-2ft)-TW	▲ S1-B1-(2-4ft)-SW	◆ S2-B1-(0-2ft)-TW
● S2-B1-(2-4ft)-SW	◆ S3-B1-(2-4ft)-SW	■ S3-B2-(0-2ft)-SW
■ S3-B3-(0-1ft)-SW	◆ S4-(0-0.5ft)-LC-SW	■ S4-(0-0.5ft)-HC-SW
▲ S5-(0-0.5ft)-LT-SW	● S6-(0-0.5ft)-LC-SW	◆ S7-B1-(0-2ft)-TW
● S7-B1-(2-4ft)-SW	● S8-B1-(0-2ft)-TW	■ S8-B1-(2-4ft)-L1-SW
▲ S8-B1-(2-4ft)-L2-SW	◆ S11-(0-0.5ft)-LC-TW	■ S11-(0-0.5ft)-HC-TW
■ S12-B1-(0-2ft)-TW	▲ S12-B1-(2-4ft)-SW	▲ S15-Canal Side-(0-0.5ft)-LC-SW
■ S15-CanalSide-(0-0.5ft)-HC-SW	● S15-Levee Crown-(0-0.5ft)-LT-SW	■ S15-Levee Crown-(0.5-1.0ft)-LT-SW

# 1. User Entered Data

## -Parameter Estimation

User Entered Data

# 2. Simplified Physical

Simplified Physical

# 3. DL Breach

Physical Breaching (DLBreach)

User Entered Data

Simplified Physical

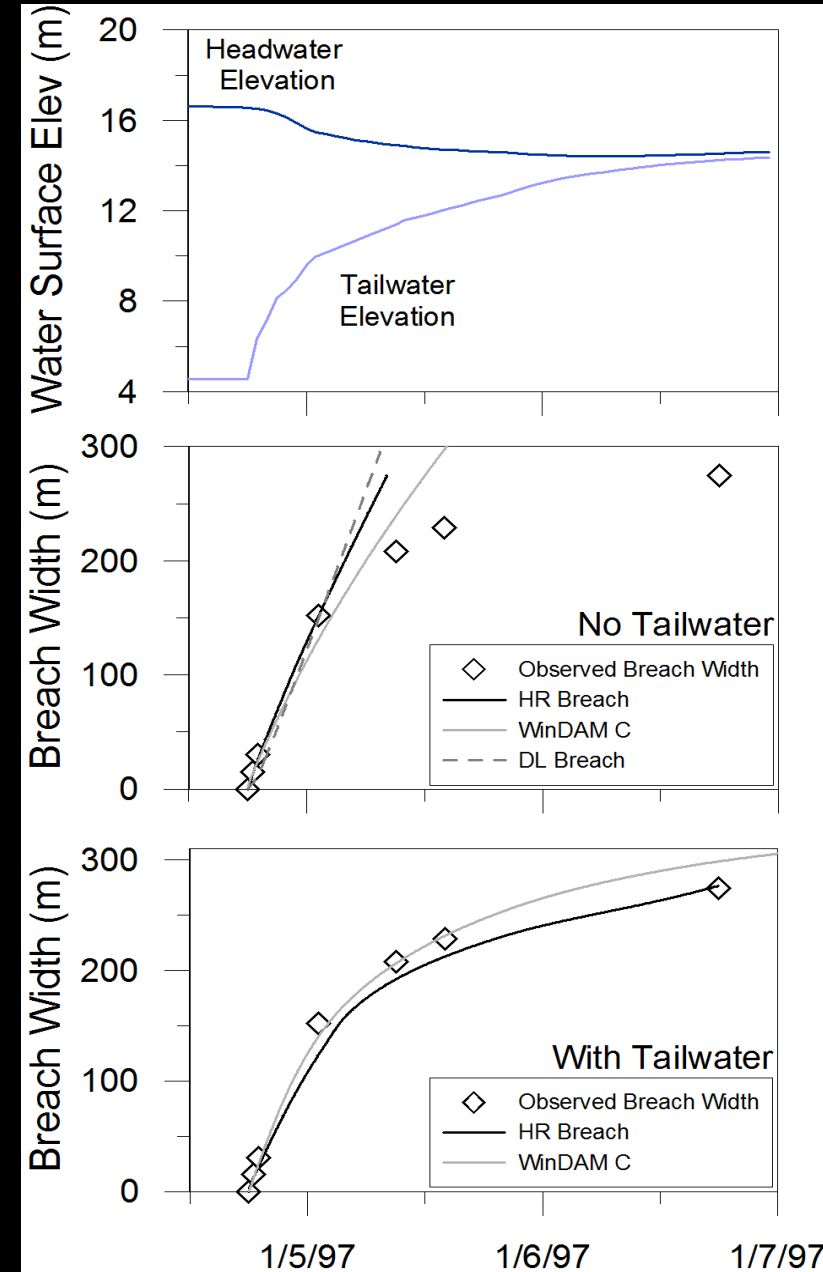
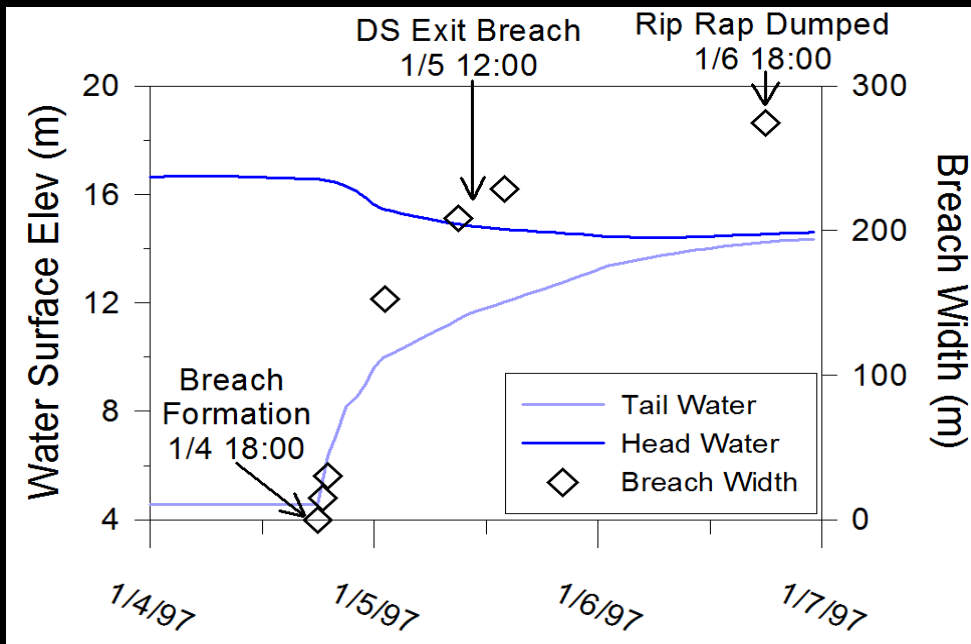
Physical Breaching (DLBreach)

# Process Models:

- ARS SIMBA/WinDAMB
- HR-BREACH (HR Wallingford)
- DL Breach (Dr Weiming Wu)

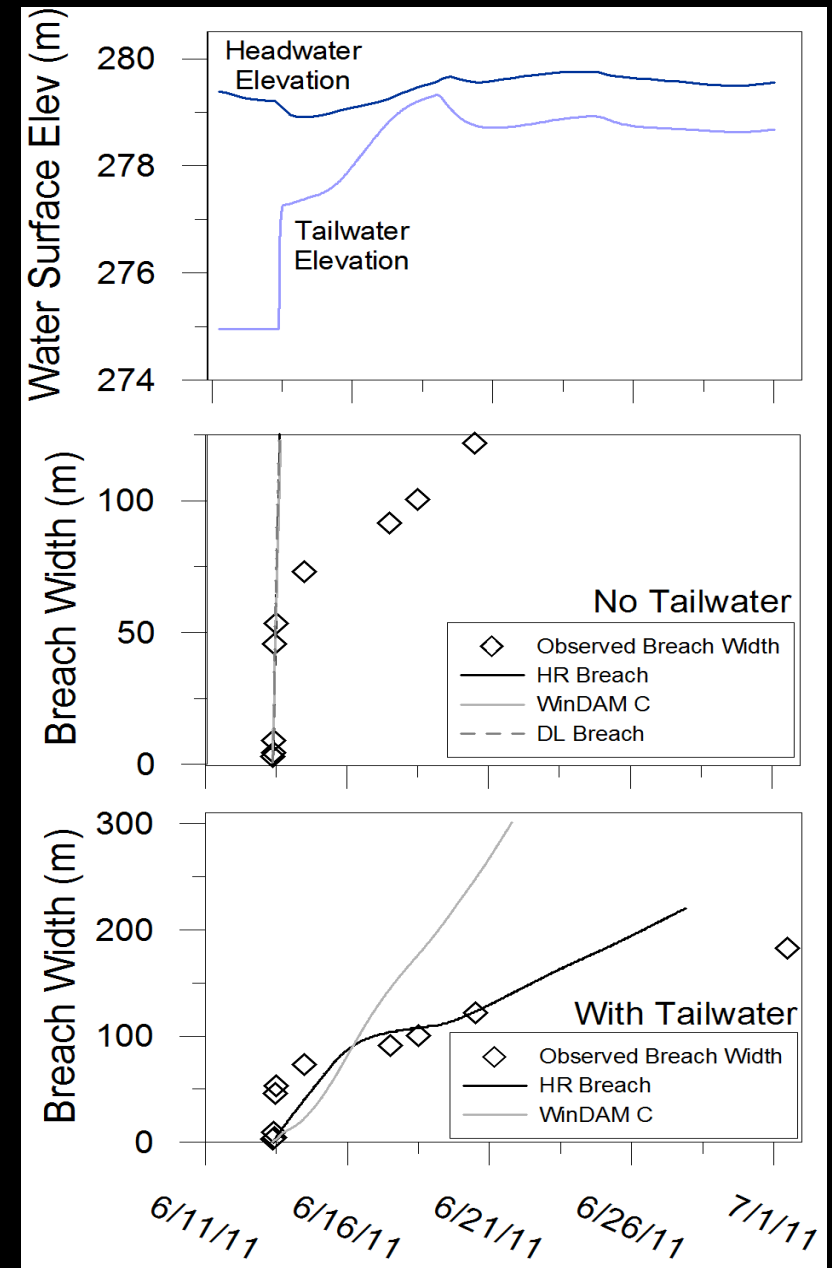
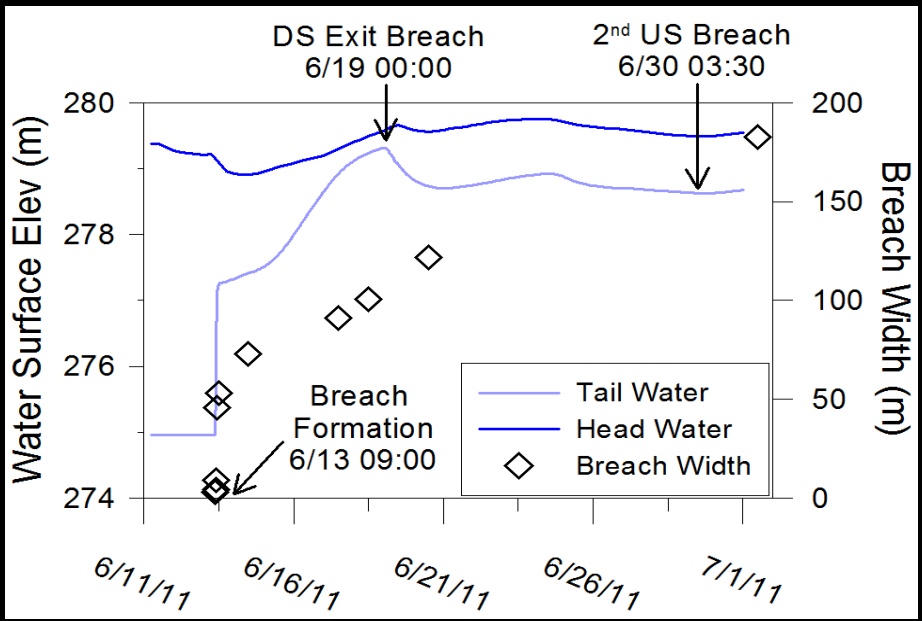
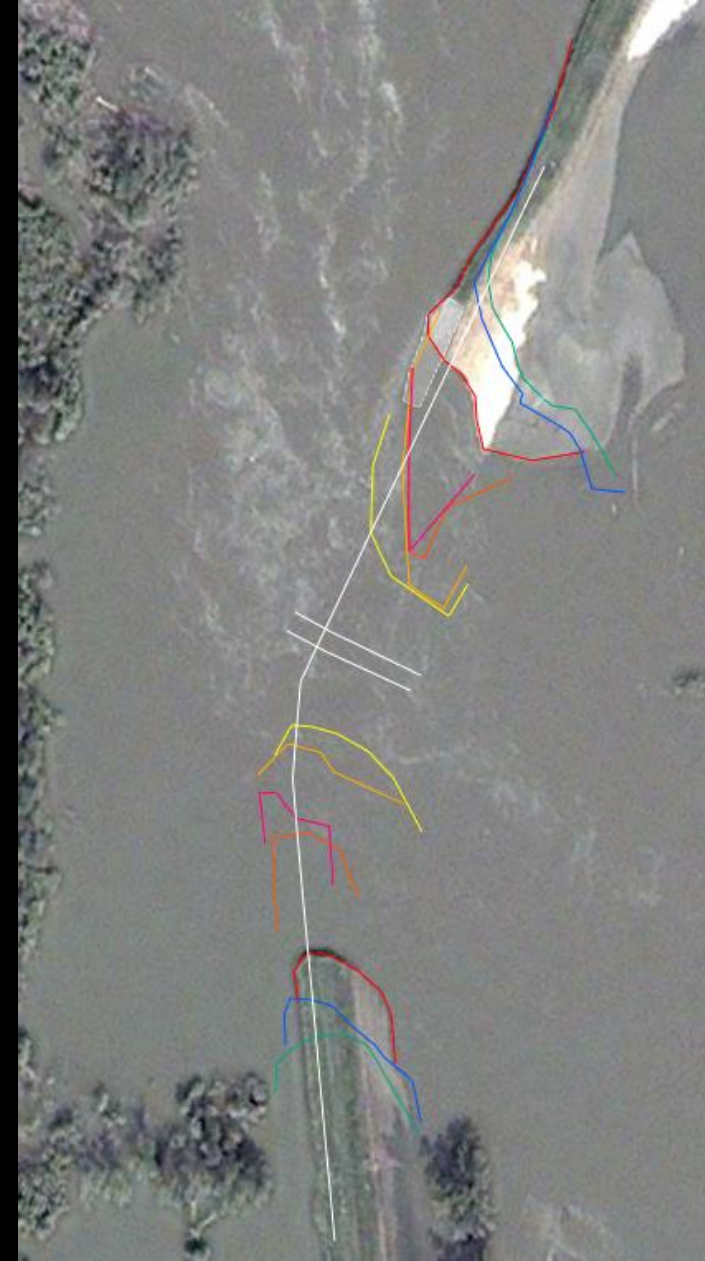
# Sutter Bypass Levee Breach

From: Risher, P. and Gibson, S. (2016) "Applying Mechanistic Dam Breach Models to Historic Levee Breaches," Proceedings FloodRisk 2016.



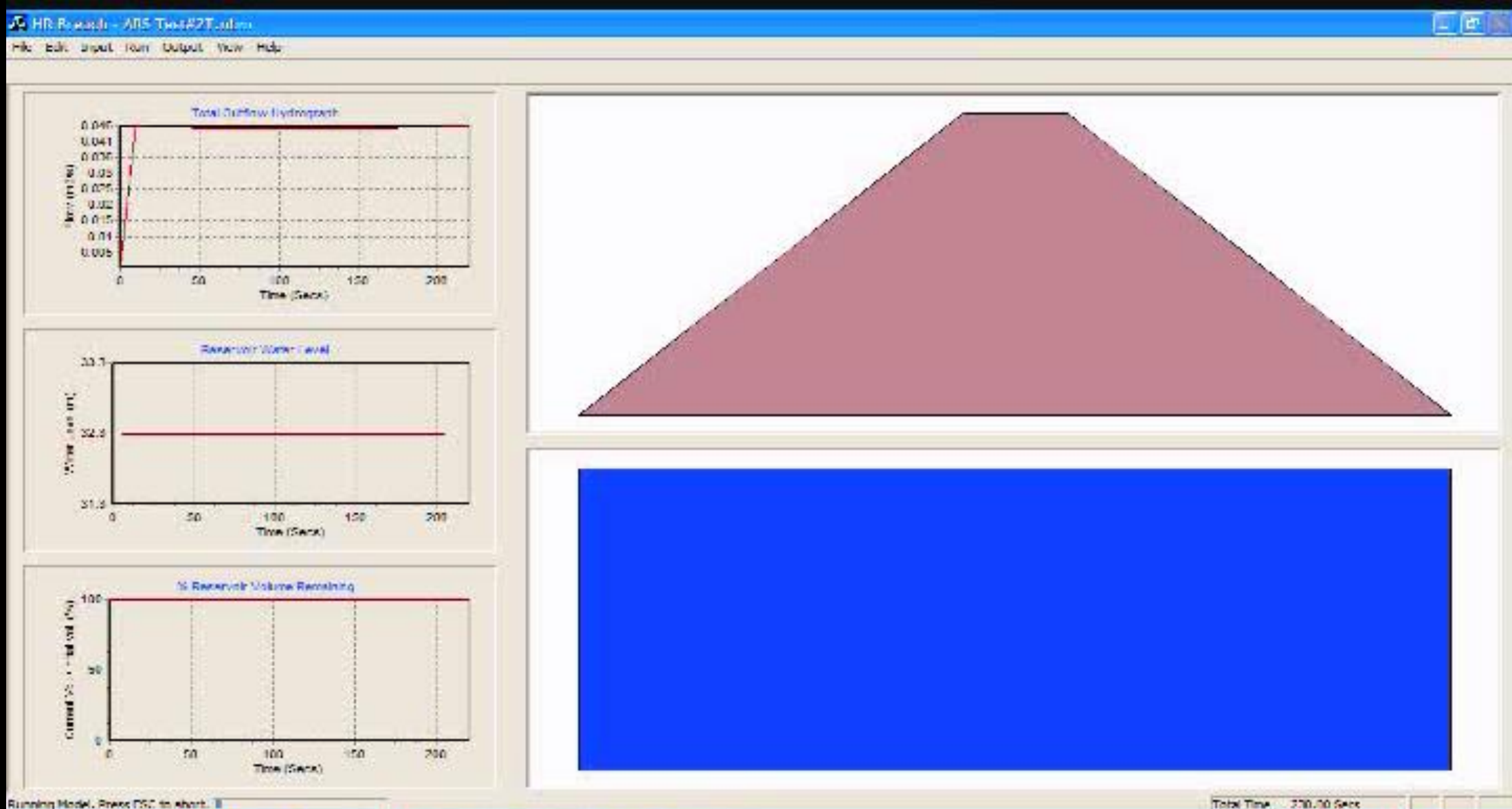
# Hamburg Breach (Missouri River)

From: Risher, P. and Gibson, S. (2016) "Applying Mechanistic Dam Breach Models to Historic Levee Breaches," Proceedings FloodRisk 2016.



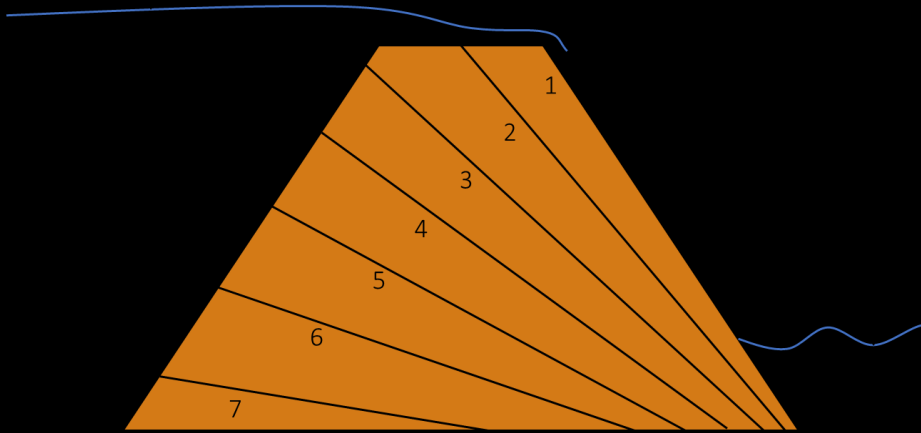
# HR BREACH

- Erosion through homogeneous or simple composite dams
- Channel erosion, head cutting and side slope instability + piping
- Storage routing with quasi-steady hydraulics thru the breach

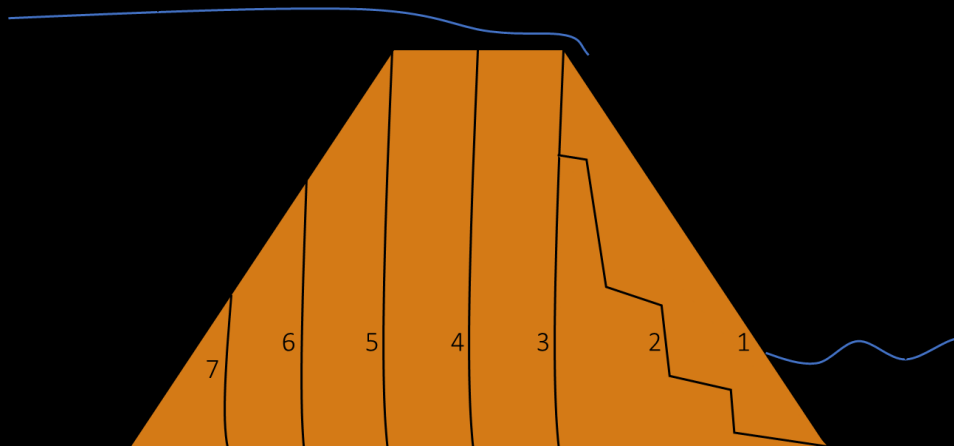


# DLBreach

Surface



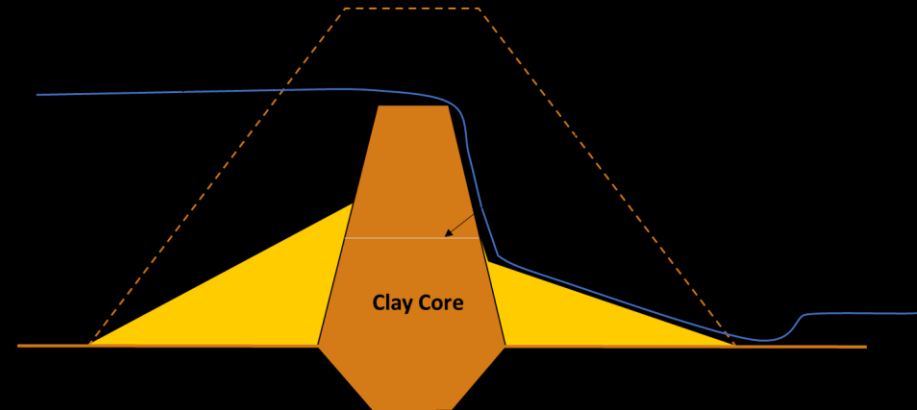
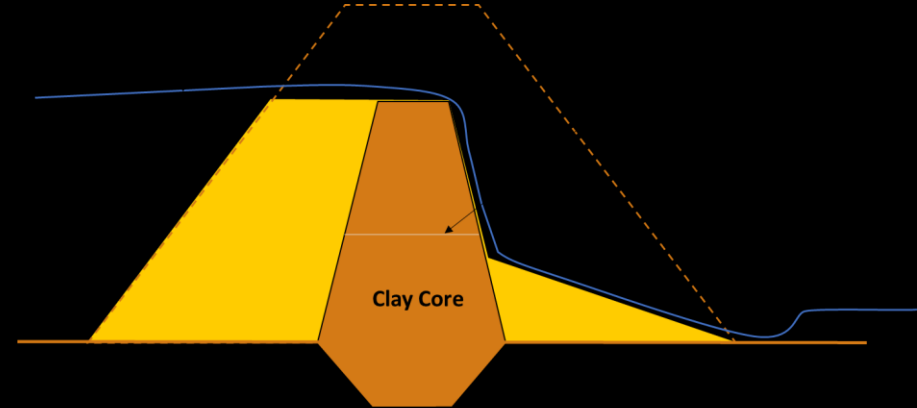
Headcut



Surface Erosion

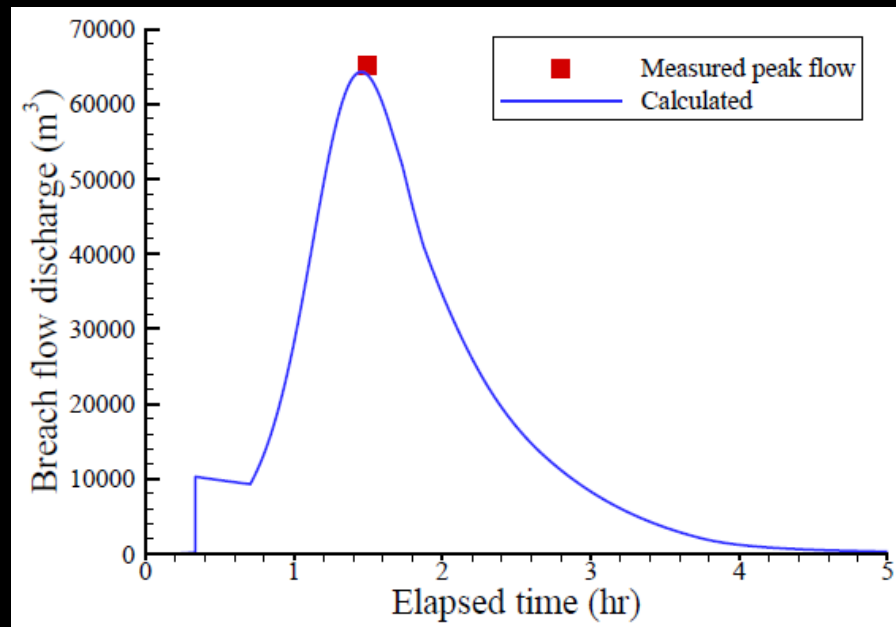
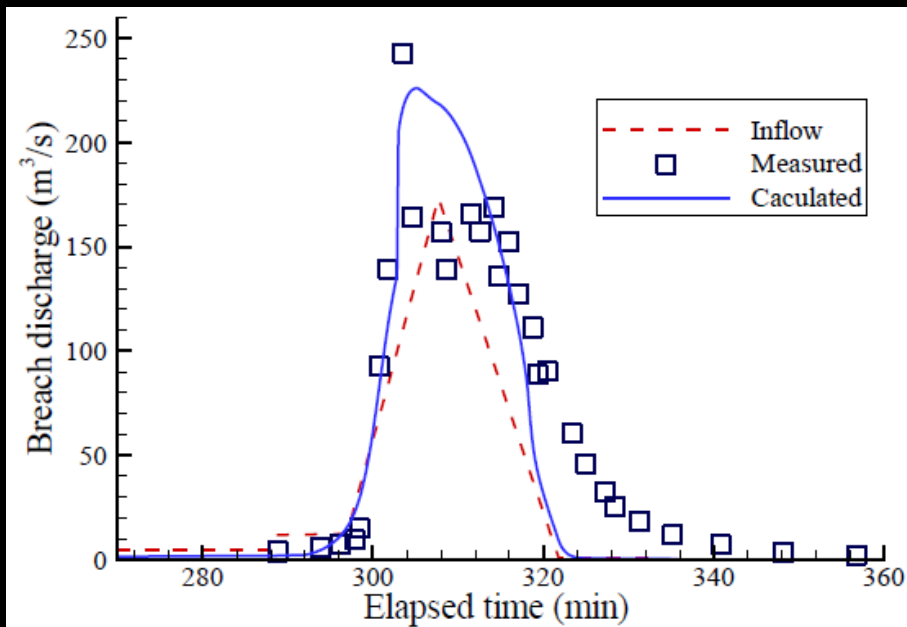
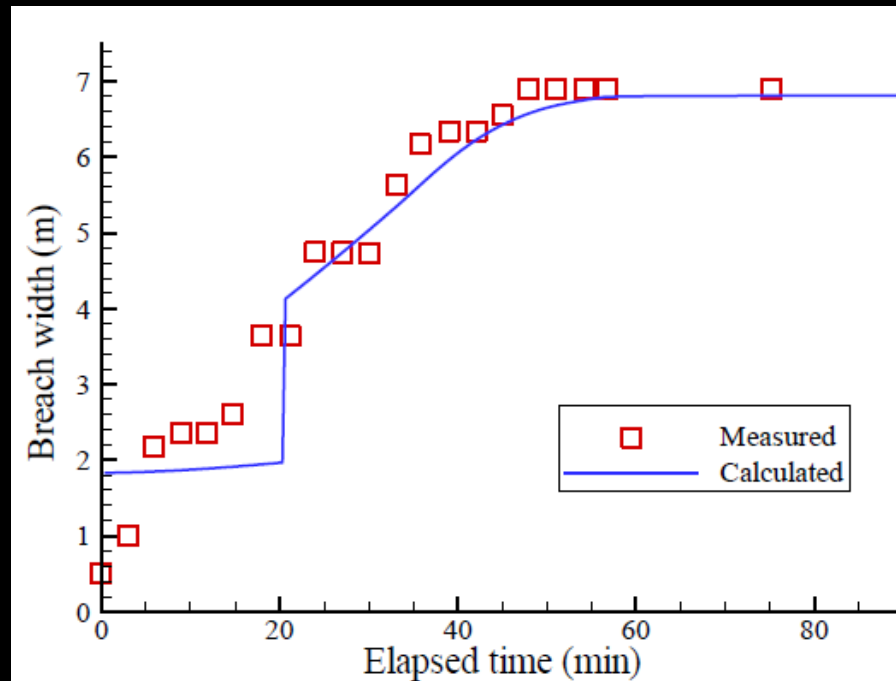
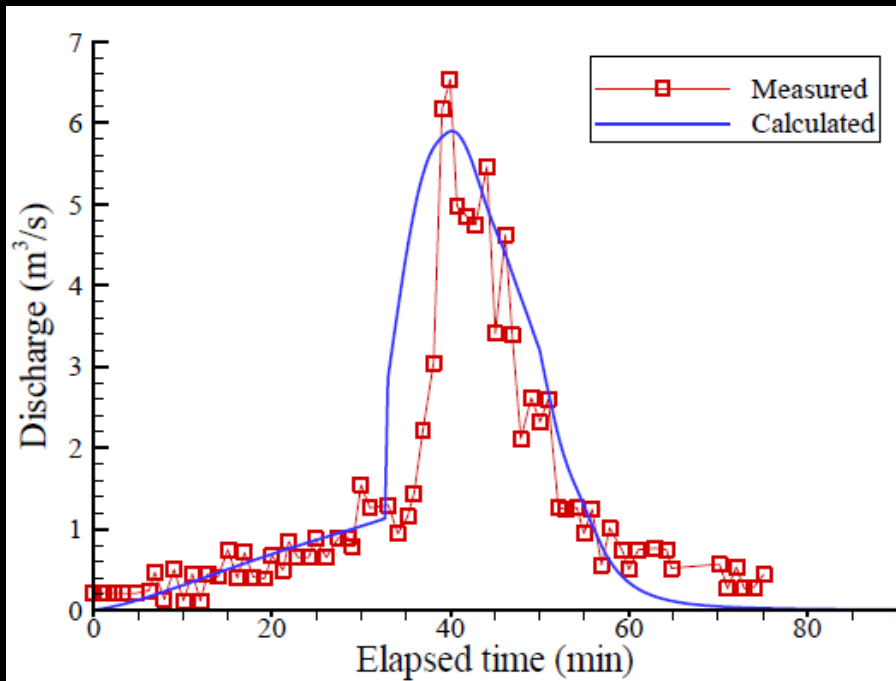
Headcut Erosion

Composite (Zoned) Structure



Composite

# DLBreach



# DLBreach

Dam (Inline Structure) Breach Data

Inline Structure: **Teton Reach 1 2.5** [Down Arrow] [Up Arrow] [Delete this Breach ...] [Delete all Breaches ...]

☒ **Breach This Structure**

Breach Method: **Physical Breaching (DLBreach)**

Center Station: 1000

Max Possible Bottom Width: 250

Min Possible Bottom Elev: 0

Left Side Slope: 0

Right Side Slope: 0

Breach Weir Coef: 1.7

Breach Formation Time (hrs):

Failure Mode: **Piping**

Piping Coefficient: 0.05

Initial Piping Elev: 48

Initial Piping Diameter: 0.1

☐ Mass Wasting Feature:

Trigger Failure at: **Set Time**

Start Date: 01JAN2000

Start Time: 2400

**Breach Plot** | Breach Progression | Simplified Physical | **Physical Breaching (DLBreach)** | Parameter Calculator | Breach Repair (options)

Erosion Model (Overtopping Only):

Surface Erosion  
Headcut Erosion  
Composite (Zoned) Structure

Embankment Width: 10.5

Slope (H:B) Roughness:

US Slope: 0.3333 0.016

Flat Top: 0.016

DD Slope: 0.4 0.016

Soil Parameters:

Soil Type: **Cohesive**

Sediment Diameter: 0.00003

Porosity: 0.3

Specific Gravity: 2.65

Clay Content: 0.3

Cohesion: 25000.

Friction Angle: 0.65

Adaptation Length:

Erodibility (kd): 8.

Critical Shear Stress: 0.15

Breach Direction: **One Way**

☒ Model a cover layer

Clay Cover and Core Parameters:

Parameters	Cover	Core
Core Height:		
Core Crest Width:		
Core US Slope:		
Core DS Slope:		
Core Center Location:		
Core Manning n:		
Soil Type:		<b>Cohesionless</b>
Sediment Diameter:		
Porosity:		
Specific Gravity:		
Clay Content:		
Cohesion:		
Friction Angle:		
Erodibility (kd):		
Critical Shear Stress:		
Top Thickness:		
US Slope Thickness:		
DS Slope Thickness:		

OK Cancel

Which of these parameters are hard?

# Four Important Ideas

1. Do not mix-and-match width and breach time.
2. Xu & Zang has a different breach development time.
3. Does the breach progression make physical sense?
4. Test sensitivity...on important result.  
(e.g. Arrival Time/Max Stage vs Breach Geometry)  
-Try multiple methods



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**US Army Corps  
of Engineers**  
Hydrologic Engineering Center

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# Using HEC-RAS for Dam Break Studies

**August 2014**

<https://www.hec.usace.army.mil/publications/TrainingDocuments/TD-39.pdf>

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Hydraulic Laboratory Report HL-2011-09

## Physical Hydraulic Modeling of Canal Breaches

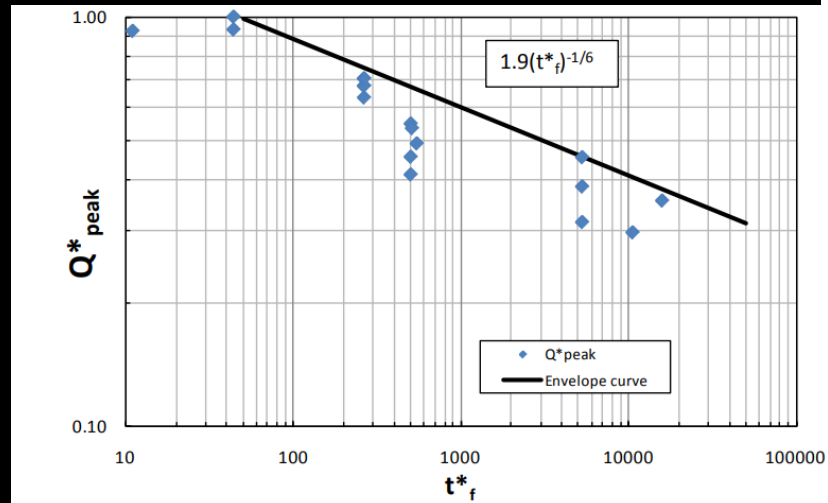


Figure 28. — Dimensionless peak discharge as a function of dimensionless breach development time.

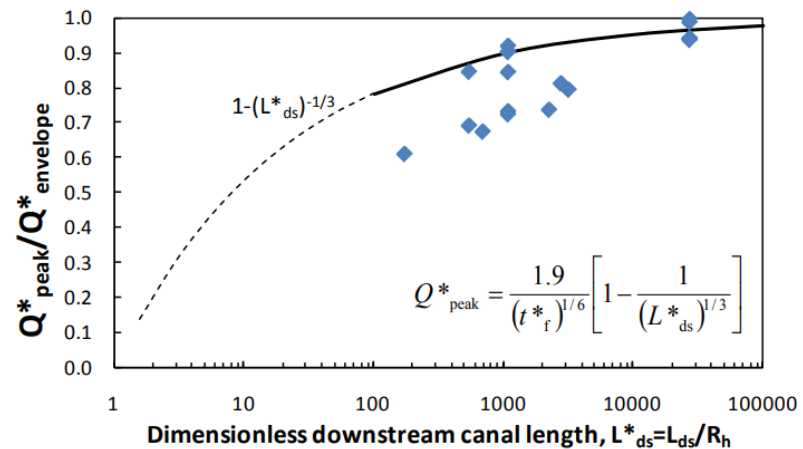


Figure 29. — Effect of downstream canal reach length on peak breach outflow.



U.S. Department of the Interior  
Bureau of Reclamation  
Technical Service Center  
Hydraulic Investigations and Laboratory Services Group  
Denver, Colorado

September 2011