

# **Techniques for Evaluating Long-Term Reservoir Yields**

February 1969

Approved for Public Release. Distribution Unlimited.

**TP-14** 

A	EPORT DOC	UMENTATIC	N PAGE		Form Approved OMB No. 0704-0188
existing data sources, ga burden estimate or any c	thering and maintaining ther aspect of this colle ations Directorate (070 failing to comply with	g the data needed, and ection of information, in 4-0188). Respondents a collection of information	completing and reviewi cluding suggestions for should be aware that n on if it does not display	ng the collection of reducing this burde otwithstanding any	ng the time for reviewing instructions, searching i information. Send comments regarding this en, to the Department of Defense, Executive other provision of law, no person shall be MB control number.
1. REPORT DATE (DD-I	,	2. REPORT TYPE		3. DATES CO	VERED (From - To)
February 1969		Technical Paper			
4. TITLE AND SUBTITL				a. CONTRACT NU	JMBER
Techniques for Eva	luating Long-Ter	m Reservoir Yield	ls		
			5	b. GRANT NUMB	ER
			5	c. PROGRAM ELI	EMENT NUMBER
6. AUTHOR(S)			5	d. PROJECT NUM	IBER
A.J. Fredrich					
			5	e. TASK NUMBER	र
			5	F. WORK UNIT N	UMBER
7. PERFORMING ORG. US Army Corps of Institute for Water Hydrologic Engine 609 Second Street Davis, CA 95616-	Engineers Resources ering Center (HE0			8. perform TP-14	ING ORGANIZATION REPORT NUMBER
9. SPONSORING/MON	TORING AGENCY NA	ME(S) AND ADDRESS	S(ES)	10. SPONSO	R/ MONITOR'S ACRONYM(S)
					R/ MONITOR'S REPORT NUMBER(S)
				11. SPONSO	R/ MONITOR S REPORT NUMBER(S)
14. ABSTRACT The effects of some techniques may be techniques for eval	NOTES CE National Mee basic assumption chose to produce uating long-term	ting on Water Res ns of simplified me the desired results reservoir yield may	ethods of reservoir with the best expo y be divided into t	yield forecast enditure of tim wo general cat	eans, 8 February 1969. ing are demonstrated so that e, funds, and manpower. The egories: (1) non-sequential methods ods which consider both the quantity
of water available a simplified techniqu given yield may be are actually signific yield in a complex	and its time distributes are evaporation under-estimated leant. None of the multiple-purpose	ution. The three to n, seasonal variation by more then forty simplified technic project. The capa	factors about whic on in demand, and y percent if evapor ues are capable of bility now exists t	h assumptions drought durati ation and sease accounting fo p perform these	are frequently made in the ion. Storage requirements to meet a onal variation are ignored when they r all of the factors which affect the e sequential routings by electronic ay be used with generated
supply, safe yield,	probability, statist				nating reservoir operation, water
16. SECURITY CLASSI			17. LIMITATION	18. NUMBER	19a. NAME OF RESPONSIBLE PERSON
a. REPORT U	b. ABSTRACT U	c. THIS PAGE U	OF ABSTRACT UU	OF PAGES 38	19b. TELEPHONE NUMBER

# **Techniques for Evaluating Long-Term Reservoir Yields**

February 1969

US Army Corps of Engineers Institute for Water Resources Hydrologic Engineering Center 609 Second Street Davis, CA 95616

(530) 756-1104 (530) 756-8250 FAX www.hec.usace.army.mil Papers in this series have resulted from technical activities of the Hydrologic Engineering Center. Versions of some of these have been published in technical journals or in conference proceedings. The purpose of this series is to make the information available for use in the Center's training program and for distribution with the Corps of Engineers.

The findings in this report are not to be construed as an official Department of the Army position unless so designated by other authorized documents.

The contents of this report are not to be used for advertising, publication, or promotional purposes. Citation of trade names does not constitute an official endorsement or approval of the use of such commercial products.

TECHNIQUES FOR EVALUATING LONG-TERM RESERVOIR YIELDS <sup>(1)</sup> by AUGUSTINE J. FREDRICH <sup>(2)</sup>

#### INTRODUCTION

The ever-increasing demand for water from reservoirs for municipal and industrial requirements, for water quality control and for agricultural uses has caused increased emphasis upon the development of maximum feasible yield at each new reservoir site. Furthermore, because human nature and economic necessity have caused utilization of water in this country to consistently press upon the upper bounds of the available supply, the dependability of existing and future water supplies becomes an increasingly important factor in reservoir yield studies.

The need for maximum feasible development and the need to determine dependability are two reasons that the techniques used to evaluate water yield should allow consideration of all factors which significantly influence the yield from the project. The purpose of this paper is to compare the results of reservoir storage-yield analyses under several different assumptions in order to demonstrate the effects of the assumptions on the analyses.

<sup>(1)</sup> For presentation at the ASCE National Meeting on Water Resources Engineering in New Orleans, 8 February 1969.

<sup>(2)</sup> Engineer-in-Charge, International Hydrological Decade Project, The Hydrologic Engineering Center, US Army Corps of Engineers, Sacramento, California.

### PURPOSE OF THE STUDY

Previous work by individuals and agencies concerned with planning, design, and operation of reservoirs and reservoir systems indicates that there are numerous instances where so-called "short-cut" methods do not provide reliable estimates of the water yield available from a reservoir. Likewise, advocates of the "short-cut" methods have demonstrated that, under certain conditions, their techniques do provide reliable estimates at a significant saving of both time and money. What has not been shown, however, are the results of some of the assumptions which are implicit in many of the simplified techniques. The subject study has not been conducted for the purpose of degrading the simplified methods but rather to clearly demonstrate the effects of some basic assumptions so that one may more readily identify instances where these assumptions are significant and thus choose techniques which will produce the desired results with the expenditure of minimum time, funds, and manpower.

The techniques for evaluating long-term reservoir yield may be divided into two general categories: (1) non-sequential methods which generally consider only the quantity of water available; and (2) sequential methods which consider both the quantity of water available and its time distribution. Within each of these two categories there are two subdivisions: (1) deterministic methods which produce yield estimates based upon the streamflow as it actually occurred;

and (2) stochastic methods which provide yield estimates based upon the statistical characteristics of the streamflow data. The intent of this study is to compare techniques from each of the two general categories under a representative variety of hydrologic conditions.

The three factors about which assumptions are frequently made in the simplified techniques are evaporation, seasonal variation in demand, and drought duration. Consequently, this study places emphasis upon the relationship among these three factors, various hydrologic conditions, and the techniques used to determine the yield.

## BASIC ASSUMPTIONS

Seven streamflow stations in the United States were selected for this study. Each of the seven locations had a relatively long continuous record of streamflow data, and at each of the seven stations the streamflow data used were essentially unregulated. Furthermore, the seven locations were selected so as to be representative of the wide variety of hydrologic conditions which exist in the United States. Table 1 shows some pertinent physical and statistical data for the selected stations. An area-capacity relationship was developed for each site from examination of topographic characteristics of the site and from analysis of area-capacity data for existing reservoirs nearby. Monthly net evaporation rates were then computed from long-term evaporation and precipitation data at climatological stations near to the streamflow stations. These data were used in the sequential routing

studies to simulate evaporation losses reasonably characteristic of each particular region.

A single seasonally varying demand schedule was adopted for use in all aspects of the study relating to the effect of seasonal variations in water demands. It was recognized that the seasonal variations in demand would differ from region to region but, since the purpose of the study is the demonstration of the effect rather than the establishment of quantitative values for use in design studies, the adoption of a single schedule which is reasonably characteristic of several water uses throughout the country is consistent with the study objectives. In the adopted schedule the maximum demand occurred in the three summer months and the maximum monthly demand was 1.92 times the average annual demand. The minimum demands occurred in the months of November, December, January, and February and in each of these months the demand was 0.48 times the average annual demand.

Several other factors which may be important in yield analyses but which are obviously beyond the scope of simplified techniques were not considered in the study. Examples of these factors include multiple demands with different priorities, unusual starting or ending conditions, and complex operating rules.

# STUDY PROCEDURE

The procedure used for the study was to determine by monthly sequential routing for the selected period of record the yield (with no shortage) corresponding to each of at least fifteen different storages

at each site. The storages were selected so as to produce yields ranging from 10 to 90 percent of the average annual flow. Since the determination of yield by sequential routing techniques is not a direct solution, successive approximations - each approximation consisting of a complete monthly sequential routing for the entire period of historical record - were used to determine the yields. A generalized computer program developed in The Hydrologic Engineering Center, "Reservoir Yield" [1], was modified to perform the successive routings necessary to determine the yield from each storage.

Four sets of routings were made for each of the seven sites. Each set was intended to produce the yields which would be obtained from a reservoir at the site under typically assumed conditions. The four conditions are: 1) no evaporation and no seasonal variation in demand - the traditional mass curve analysis; 2) with evaporation, but without seasonal variation in demand; 3) without evaporation, but with seasonal variation in demand; and 4) with both evaporation and seasonal variations in demand. All other variables such as starting conditions, operating rules, and outlet capacities were the same for all four sets of routings.

The first set of routings produced for each of the seven sites, the yields which one might obtain using a simple mass curve analysis for each of the assumed storages. The time of occurrence and the duration of the critical period were tabulated for each of the assumed storages.

The second set of routings provided the yields and critical periods for the same storages used in the first set but with evaporation included in the analysis. Consequently the results of this set of routings, when compared with the results of the first set, would give an indication of the effect of evaporation on the yield and on the duration of the critical period.

The third set of routings produced the same type of data as the two previous sets and, thus, by comparing the results of this set with the first set, one could assess the effect of seasonal variation in demand; and by comparing the results of this set with the second set a comparison of the relative magnitude of these two effects could be obtained.

The final set produced, for purposes of this study, the yields which would actually be obtained if the assumed projects were constructed. When compared with the results of the first set, these results would indicate the consequences of ignoring evaporation and seasonal variation in demand. Furthermore, comparison of these results with the combined results of the second and third sets would indicate whether the effects of evaporation and seasonal variation are independent or whether there is an interrelationship between the two which would accentuate or diminish the combined effects.

#### OPTIMIZATION ROUTINE

The key to development of analytical techniques for comparison of many sites at many levels of utilization and under several alternative assumptions is a completely computerized optimization routine for

use in the storage-yield analysis. In the past, storage-yield studies by computer have been hampered by the necessity for either human intervention in the successive approximation procedure or unwieldly numbers of trials based on a systematic search scheme. Since the subject study required almost 600 separate yield determinations each based on more than 40 years of hydrologic record for each trial it was apparent that the work could not be accomplished at a reasonable cost or in a reasonable amount of time unless a very efficient scheme could be developed to utilize the results of the first sequential routing to estimate the yield available from the specified storage.

Since the objective of the studies was to find the yield, with no shortage, from a given storage, it was not possible to use efficient techniques based upon the shortage index [2] or other similar mathematical indices of shortage frequency or magnitude, because available methods [3] require a target index that is non-zero - a condition which is violated when the objective is to have no shortages. Furthermore, since about 600 initial estimates of yield were required - a number which prohibited preliminary studies to obtain good first estimates in each case, it was desirable that the optimization scheme be rapidly converging irrespective of the accuracy of the initial estimate of the yield available from a given storage.

Because there is no way of pre-determining whether an estimated yield is too large or too small, the routine would have to be efficient in either case and would also have to be developed in a way which allows

collection of data necessary for subsequent estimates without prior knowledge of which conditions would prevail. Furthermore, since each trial consists of a sequential monthly routing at least 40 years long, it was apparent that the routine should not be dependent upon having the complete results of prior trials available for subsequent estimates. In other words, the routine should provide for condensation of all important aspects of a trial into as few significant parameters as possible.

The method used was proposed by Mr. Leo R. Beard and was based upon his belief that the most efficient optimization scheme would, of necessity, involve examination of the internal sequential analysis rather than a mathematical technique which used only the final results of the initial analysis to predict the yield. The method consists of computing two ratios at appropriate times during a sequential routing. The first ratio, used to estimate yield in a subsequent trial when the current estimates proves to be too small, is the ratio of the remaining storage at the end of a given month to the accumulated demand since the last time the reservoir was full. This ratio is computed each month during the sequential routing if the end-of-month storage is greater than the minimum storage. As soon as the ratio is calculated it is checked to see if it is smaller than any value previously obtained. If so, it becomes the stored value, and the corresponding month and year are also stored. Thus, at the end of a sequential routing this ratio is an indicator of the point at which an increase in yield will have the greatest effect on the storage and, also, it is an indicator of the magnitude of the change necessary to completely utilize the storage in terms of the trial demand.

Obviously, the ratio described above would be zero for one or more months if the trial yield is too large for the available storage. Consequently, a second ratio is computed for all months when the demand cannot be met. This ratio, used to estimate the yield in subsequent trials when the current estimated yield is too large, is the ratio of the accumulated shortage to the accumulated demand - both accumulations beginning at the time the reservoir was last full. In this case it is necessary to store the largest value obtained. Again, at the end of the sequential routing this ratio indicates, in terms of the estimated yield, the magnitude of reduction in yield necessary to keep from exceeding the capability of the available storage and also indicates the time when the maximum drawdown will occur.

Since both ratios contain accumulated demands, the routine is equally adaptable to both uniform and seasonally varying yields. Furthermore, since the routine does account for the state of the reservoir with respect to the demand at any point in time it was found to be sensitive to the effect of evaporation also. A few initial runs quickly indicated that the routine was capable of identifying, after the first trial routing, the ultimate critical period even though the first estimate of the yield was off by more than 100 percent and despite the fact that the critical period under the first estimate of the yield was not in the same decade as the ultimate critical period

and was not of the same duration. Additional testing of the routine indicated that in most cases the correct yield could be computed without human intervention in three trials, irrespective of the error in the first estimate. Where there was no evaporation, the technique gave the correct result in two trials and, in cases where the evaporation had a major influence on the results, the routine could require as many as five trials. An example of the convergence of the routine is shown by the computer output included as Figure 1.

#### RESULTS

The results of this study are tabulated in Tables 2 to 8 and are illustrated in Figures 2 to 8. Although detailed analysis of the data in the tables would point out many interesting facets of the study to the engineer who is familiar with storage-yield analyses, it is more useful to point out only some of the more apparent results that are discernible to those who are not as familiar with this type of hydrologic engineering study.

First, it is apparent that seasonal variation in demand is an extremely important factor in yield analysis. At these seven sites, the analysis indicated that the seasonal variation in demand could reduce yields from 10 to 33 percent. In effect, this indicates that the assumption of uniform yield for simplified studies can result in serious over-estimates of yield when the demand is actually seasonally varying. Furthermore, it appears that the duration of the critical

period is influenced by the seasonal variation in some cases. Finally, examination of the graphical data indicates that the influence of seasonal variation is not always predictable for the full range of development at a site. For this reason it would seem advisable to caution against applying a "factor" to account for this influence unless the exact nature of the influence is well-understood in the particular case.

Second, it appears from these studies that the influence of evaporation upon the yield may not be as significant as the influence of seasonal variation. Consequently, one might reason that assuming zero evaporation would be less serious than assuming no seasonal variation. However, it should be noted that the influence of such an assumption would tend to favor larger projects over small projects, because the influence of evaporation appears to be proportional to the size of the reservoir whereas the influence of seasonal variation appears to be dependent primarily upon factors such as the length of the critical period and the variations in time distribution of streamflow.

Finally, as shown on Figure 3, the combined effects of evaporation and seasonal variation in demand are apparently not always what one would expect from observing the effects of each alone. Therefore, it would appear to be advisable to <u>carefully</u> investigate the interrelationship between these two factors before attempting to deduce their combined effect.

#### CONCLUSIONS

The factors investigated in this study (as well as those described initially as being beyond the scope of this study) are of major importance in the analysis of the storage-yield relationship. This study indicates that storage requirements to meet a given yield may be underestimated by more than 40 percent if evaporation and seasonal variation are ignored when they are actually significant. Furthermore, the effects of multiple demands and complex operating rules are known to significantly alter the storage-yield relationship. However, it is difficult to devise a study to illustrate these effects because of the number of variables involved.

None of the simplified techniques known to the writer are capable of accounting for all of the factors which affect the yield in a complex multiple purpose project. Since present conditions in some areas and foreseeable future conditions in many other areas are such that these factors cannot be ignored and since multiple demands with varying priorities and attendant complex operating rules are becoming an inseparable part of even the smallest water resources developments, it appears that there is a real and ever-increasing need to emphasize the sequential routing study as the primary tool in hydrologic analyses of the storage-yield relationship.

The study described herein has demonstrated that the capability now exists to perform these sequential routings by electronic computer, even in the initial planning phases of a study. Furthermore, these routings

may be used with generated streamflow data as shown by Young and Pisano  $\begin{bmatrix} 4 \end{bmatrix}$  to establish estimates of the statistical aspects of the storage-yield relationship. Hopefully, this type of approach will gain increased accept-ance as electronic computers become more widely available and as engineers become more conversant with the capabilities of the various techniques.

# ACK NOWLEDGMENTS

The writer is indebted to Mr. Leo R. Beard for his advice and comments during the studies. The work described herein was performed using the facilities of the U. S. Army Corps of Engineers, The Hydrologic Engineering Center, Davis, California. However, the views and opinions contained herein are those of the writer and do not necessarily reflect the policy of the Corps of Engineers.

### REFERENCES

- U. S. Army Corps of Engineers, "Reservoir Yield", Computer Program Description, The Hydrologic Engineering Center, Program No. 23-45, 1967.
- 2. Beard, Leo R., "Estimating Long-Term Storage Requirements and Firm Yield of Rivers", General Assembly of Berkeley of HVGG, 1964, Surface Waters, pp. 151-166.
- 3. Fredrich, Augustine J., "A Pilot Study of Storage Requirements for Low Flow Augmentation" 49th Annual Meeting - American Geophysical Union, Washington, D. C., 1968.
- 4. Young, G. K. and Pisano, W. C., "Reservoir Analysis for Low Flow Control," Technical Note, Journal of Sanitary Engineering, ASCE, pp 1305-07, December, 1968.

TABLE 1 PERTINENT PHYSICAL AND STATISTICAL DATA SEVEN SELECTED STATIONS

					ANNUAL SI	ANNUAL STATTSTICS	RANC	RANGE OF MOWTHLY STATISTICS	ILY STATIS'	TCS
	496-446 XIV	- Carlos and	Years	Average			Minimum	Maximum	Minimum	Maximum
		Drainage	of	Annual		ingo,y ster	Month	Month	Month	Month
į	Gaging	Area	Record	Flow	Geometric	Standard	Geometric	Standard Geometric Geometric	St t	Standard
Stream	Station	(Sq.Mi.)	Used	(cfs)	Mean	Deviation	Mean	Mean		
Westfield River	Knightsville, Mass.	162	24	315	2.489	971.	1.791	2.931	.187	.401
Arroyo Seco	Soledad, Cal	241	58	163	2.100	.352	-0.086	2 1180	363	or l'
Deerfield River	Charlemont, Mass	362	23	874	2.938	680	2.251	3.422		396
Tygart River	Belington, W.V.	1+08	2S S	792	2.898	• 095	2.001	3.299	.166	.621
Grand River	Gallatin, Mo.	2250	40	1125	2.956	.320	2.117	3.137	526	750
Willamette River	Albany, Ore.	4840	61	14,274	4.021	.108	3.534	4.410	000	502
Little River	Cameron, Tex.	7034	43	1805	3.110	.391	2.249	3.434	.438	502.

TABLE 2 RESULTS OF STORAGE-YIELD ANALYSES WESTFIELD RIVER

USABLE STORAGE	WITHOUN	WITHOUT EVAPORATION OR SEASONAL VARIATION	OR	WITH EV SEAS	WITH EVAPORATION - WITHOUT SEASONAL VARIATION	WITHOUT SEASC	WITHOUT EVAPORATION - WITH SEASONAL VARIATION	WITH E SEASON	WITH EVAPORATION & SEASONAL VARIATION
(acre-feet)	Yield (cfs)	Critical Period (month & year)	iod ar)	Yield (cfs)	Critical Period (month & year)	Yield (cfs)	Critical Period (month & year)	Yield (cfs)	Critical Period (month & year)
290,000 248,000 248,000 1199,000 1149,000 1120,000 80,000 20,000 20,000 20,000	271.6 263.8 263.8 263.6 260.6 240.4 240.4 2213.0 2213.0 2213.0 2213.0 2213.0 2213.0 2213.0 2213.0 2213.0 2213.0 2213.0 221.0 201.0 200.0 2	++++++ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	272.0 264.0 264.0 260.8 240.4 240.4 219.7 211.9 211.9 211.9 211.9 211.9 211.9 211.9 211.9 211.9 211.9 211.9 211.9 211.9 211.9 211.9 21.0	7 7 7 7 7 7 7 7 7 7 7 7 7 7	261.9 254.2 251.3 251.3 239.1 239.1 239.1 239.1 239.1 261.6 202.4 117.2 202.4 117.2 210.1 117.2 210.1 117.2 210.1 202.4 200.10	7-61 to 9-66 7-61 to 9-66 7-61 to 9-66 7-61 to 9-66 7-61 to 9-66 7-64 to 11-65 7-64 to 11-	261.8 254.0 254.0 251.0 254.0 215.4 208.5 209.8 209.8 209.8 209.6 200.8 209.6 200.8	7-61 to 9-66 7-61 to 9-66 7-61 to 9-66 7-61 to 9-66 7-61 to 9-66 7-64 to 11-65 7-64 to 11-65 7-65 to 11-65 to 11-6

TABLE 3 RESULTS OF STORAGE YIELD ANALYSES ARROYO SECO

USABLE STORAGE	WITHOUT SEASO	WITHOUT EVAPORATION OR SEASONAL VARIATION	WITH EVA SEASO	H EVAPORATION - WITHOUT SEASONAL VARIATION	WITHOUT SEASO	HOUT EVAPORATION - WITH SEASONAL VARIATION	WITH E SEASON	WITH EVAPORATION & SEASONAL VARIATION
(acre-ieet)	Yield	Critical Period	Yield	Critical Period	Yield		Yield	Critical Period
	(CIS)	(monta & year)	(cts)	(month & year)	(cfs)	(month & year)	(cfs)	(month & year)
*000,007	162.7	3-17 to $-1-37$	153.7	3-17 to 12-35	161.7	4-17 to 12-35	152 6	h-17 to 19-25
650,000*	159.2	-	150.5		158.1	5 S	140.4	
620,000	157.0	ţ	148.5	3-17 to 12-35	155.9	4-17 to 12-35	747.4	_
600,000	155.5	ę	147.0	ţ t	154.4	ţ	145.8	\$
200,000	148.2	ţ0	140.2	ş	147.1	ţ	139.1	ę
100,000	140.8	ş	133.8	\$	139.7	с С	132.7	ş
350,000	137.1	ţ	130.6	ş	135.8	ę	129.5	
300,000	131.0	с С	125.7	ĝ	128.7	ş	123.4	<del>с</del>
250,000	122.4	3	117.5	\$	118.8	ţ	113.6	ţ
200,000	110.8	3	106.3	t t	105.7	ę	101.2	to to
145,000	6. 6 6	ц С	90 <b>.</b> 2	\$	89.6	ţ	85 <b>.</b> 9	ţ
120,000	86 <b>.</b> 3	ş	8 <b>3•</b> 0	с С	82.3	\$	79.0	4 - 46 to $10 - 50$
100,000	80.1	с С	76.9	ţ	76.4	4	73.4	ş
90°.06	76.4	4-46 to 12-49	73.3	-	73.3	4-46 to 12-49	70.3	
80°000	72.6	\$	9 <b>.</b> 69	ţ t	1.69	ę	66.0	ę
70 <b>,</b> 000	61.6	\$	6.49	-	64.1	ş	61.2	ę
60°000	62.6	Ş	59.9	\$	58.2	4-30 to 11-31	55.5	to to
50,000	56.7	4-30 to 11-31	54.0	\$	50.5	\$	148°0	<del>1</del> 0
30,000	39.3	4-30 to 11-31	37.1	4-30 to 11-31	34.7	5-30 to 11-31	<b>32.</b> 8	to 11-
	-						-	

\* With this amount of storage the yield is directly dependent upon the storage and the storage, once drawn down, cannot be completely replenished from natural streamflow.

TABLE 4 RESULTS OF STORAGE YIELD ANALYSES DEERFTELD RIVER

USABLE STORAGE	WITHOUT	WITHOUT EVAPORATION OR SEASONAL VARIATION	WITH EVAJ SEASO	IAPORATION - WITHOUT SONAL VARIATION	WITHOUT I	WITHOUT EVAPORATION - WITH SEASONAL VARIATION	WITH EVAP SEASONAL	WITH EVAPORATION & SEASONAL VARIATION
(acre-feet)	Yield (cfs)	Critical Period (month & year)	Yield (cfs)	Critical Period (month & year)	Yield (cfs)	Critical Period (month & year)	Yield (cfs)	Critical Period (month & year)
580,000 1414,000 1400,000 1400,000 332,000 332,000 280,000 221,000 112,000 112,000 100,000 100,000	765.8 748.6 731.0 726.5 675.5 675.5 675.5 675.5 632.2 632.2 551.7 551.7 551.7 551.7 551.7 551.7 551.7 381.0 381.0	7 7 7 7 7 7 7 7 7 7 7 7 7 7	7465.9 7465.9 731.2 731.2 725.9 673.3 673.3 673.3 673.3 673.3 725.9 1457.5 1457	77777777777777777777777777777777777777	737.8 719.8 701.9 689.0 689.0 653.8 653.8 653.8 184.5 184.5 184.5 184.5 128.3 251.8 275.2 251.8 275.2 251.8 275.2 251.8 275.2 251.8 275.2 251.3 2557.3 255	7 7 7 7 7 7 7 7 7 7 7 7 7 7	736.6 736.6 698.0 650.1 659.2 659.2 659.2 711.3 795.6 791.3 771.3 771.3 771.3 771.3 771.3 771.3 771.3 771.3 771.3 771.3 773.6 773.7 773.6 773.7 774.7 774.7 774.7 774.7 774.7 774.7 774.7 774.7 774.7 77777.7 7777	7-61 to 10-65 7-61 to 10-65 7-61 to 10-65 7-64 to 10-65 7-64 to 10-65 7-64 to 10-65 7-64 to 10-65 7-64 to 10-65 7-64 to 11-64 to 11-64 to 11-64 to 11-64 to 11-64

TABLE 5 RESULTS OF STORAGE-YIELD ANALYSES TYGART RIVER

Critical Period (month & year) 11-30 11-30 11-30 11-30 12-30 12-30 12-30 12-30 12-30 12-30 11-30 12-30 11-30 11-31 11-31 11-31 11-31 11-31 11-31 WITH EVAPORATION & SEASONAL VARIATION с С t t t 0 t t с С ß t 0 ç t t t t ß t t с С t t t C t C t C t C to 4-30 5-30 6-30 6-30 6-30 5-29 4-30 4-30 4-30 4-30 5-30 6-30 4-30 4-30 4-30 4-30 4-30 4-30 4-30 269.6 214.7 655.9 640.2 624.6 470.0 420.2 369.8 319.8 175.9 92.9 48.6 26.5 616.7 0.000 587.5 554.1 137.1 684.7 Yield (cfs) WITHOUT EVAPORATION - WITH Critical Period (month & year) **11-30** 11-30 11-30 11-30 11 - 3011-30 12-30 12-30 12-30 12-30 12-30 12-30 12-30 11-31 11-31 11-31 11-31 11-31 11-31 SEASONAL VARIATION ç to to to t t to to ţ t t to to to to to t t to to to t t t 0 6-30 4-30 5-30 5-30 6-30 4-30 4-30 4-30 4-30 4-30 4-30 4-30 4-30 4-30 4-30 6-30 6-30 5-29 610.1 589.2 555.7 471.4 421.4 370.9 270.4 215.4 176.4 93.0 Yield 684.4 656.8 641.2 137.4 48.6 26.5 625.6 617.8 320.7 (efs) WITH EVAPORATION - WITHOUT Critical Period (month & year) 12-30 12-30 12-30 12-30 12-30 **12-30** 11-30 11-30 12-30 12-30 11-31 11-31 11-31 11-31 1-31 11-31 11-31 11-31 7-42 SEASONAL VARIATION t t ц С to to ç to t C to t t t t ç to t t t to t t t0 t 0 5 4-30 6-30 5-29 5-29 4-30 4-30 4-30 4-30 4-30 4-30 4-30 5-30 6-30 6-30 4-39 5-29 5-29 4-30 687.4 681.9 667.6 453.9 392.2 330.7 184.4 135.5 650.0 571.2 515.2 228.0 72.3 39.4 703.8 692.8 269.1 Yield 40.1 715.1 (cfs) WITHOUT EVAPORATION OR Critical Period (month & year) 12-30 12-30 12-30 12-30 12-30 12-30 12-30 12-30 11-30 7-42 11-31 11-31 11-31 11-31 11-31 11-31 11-31 1-31 11-30 SEASONAL VARIATION t 0 to t t to to to to t 0 t0 to t t to to to to to t0 t t to 5 5-29 4-30 4-30 4-30 4-30 4-30 4-30 4-30 5-30 6-30 6-30 5-29 4-30 4-30 6-30 4-39 5-29 5-29 5-29 269.8 650.9 516.7 393.2 331.5 228.7 184.7 135.4 72.4 39.4 668.4 572.2 455.1 39.5 714.4 703.2 692.2 686.7 681.2 (cfs) **Yield** USABLE STORAGE (acre-feet) 160,000 100,000 80,000 60,000 40,000 20,000 10,000 400,000 300,000 250,000 220,000 190,000 380,000 360,000 340,000 320,000 150,000

TABLE 6 RESULTS OF STORAGE-YIELD ANALYSES GRAND RIVER

USABLE STORAGE WITHOUT EVAPORATION OR SEASONAL VARIATION	E WITHOUT SEASO	THOUT EVAPORATION SEASONAL VARIATION	ON OR ION	WITH EVAF SEASONA	WITH EVAPORATION -WITHOUT SEASONAL VARIATION	ITHOUT	WITHOUT EV SEASON	WITHOUT EVAPORATION - SEASONAL VARIATION	HTIW -	WITH EV SEASON/	WITH EVAPORATION & SEASONAL VARIATION	& ON
acre-feet	Yield (cfs)	Critical Period (month & year)	Period ۵ year)	Yield (cfs)	Critical Period (month & year)	. Period & year)	Yield (cfs)	Critical Period (month & year)	l Períod & year)	Yield (cfs)	Critical Period (month & year)	l Period & year)
2,500,000 2,300,000 2,400,000 2,200,000 2,000,000 1,500,000 1,000,000 600,000 400,000 400,000 80,000 80,000 80,000	919.9 888.2 904.0 872.1 872.1 720.0 547.8 547.8 547.8 547.8 405.2 405.2 176.3 152.8 152.8 119.8	8-32 to 8-32 to 8-32 to 8-32 to 8-32 to 6-52 to 6-52 to 6-52 to 7-37 to 7-37 to 7-37 to 7-37 to 7-37 to 7-55 to	<b>5</b> <b>5</b> <b>5</b> <b>5</b> <b>5</b> <b>5</b> <b>5</b> <b>5</b> <b>5</b> <b>5</b>	871.9 842.7 857.9 857.9 857.9 685.3 685.3 685.3 685.3 493.4 493.4 493.4 493.4 493.4 493.2 453.3 388.3 388.3 388.3 388.3 145.4 117.3	6-32 to 6-32 to 6-32 to 6-32 to 6-52 to 6-52 to 6-52 to 6-52 to 7-37 to 7-37 to 7-37 to 7-55 to 7-55 to 7-55 to 7-55 to	7 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	903.7 874.6 874.6 889.1 889.1 826.5 705.4 705.4 705.4 583.9 583.9 575.1 345.8 345.8 345.8 345.8 345.3 173.6 156.3	6-32 to 6-32 to 6-32 to 6-32 to 6-32 to 6-52 to 6-52 to 6-53 to 6-53 to 7-37 to 7-37 to 7-37 to 7-37 to 7-37 to 7-37 to	9-41 9-41 9-41 9-41 1-58 1-58 1-58 1-58 1-58 1-58 1-58 1-5	855.2 827.8 841.6 841.6 814.5 785.7 785.7 785.7 555.0 555.0 558.7 480.9 452.8 452.8 452.8 331.3 331.3 246.6 1161.6		9-41 9-41 9-41 9-41 1-58 1-58 1-58 1-58 1-58 1-58 2-39 2-39 2-39 2-39
30,000 15,000	103.1 70.0 45.1	<u>6666</u>	5-56 5-56 5-56	95.8 62.8 38.1	55 to 55 to 55 to	5-56 -56 5-56	110.5 75.0 48.4	7-55 to 7-55 to 7-55 to 7-55 to	5-56 5-56 5-56	120.3 102.7 65.6 32.1	6-53 to 7-55 to 6-53 to 6-53 to	3-54 5-56 1-54 10-53

TABLE 7 RESULTS OF STORAGE YIELD ANALYSES WILLAMETTE RIVER

USABLE STORAGE	WITHOUT SEASON	WITHOUT EVAPORATION OR SEASONAL VARIATION	WITTH EV. SEAS	WITH EVAPORATION - WITHOUT SEASONAL VARIATION	WITHOUT	WITHOUT EVAPORATION - WITH SEASONAL VARIATION	WITH EY SEASON	WITH EVAPORATION & SEASONAL VARIATION
(acre-feet)	Yield (cfs)	Critical Period (month & year)	Yield (cfs)	Critical Period (month & year)	Yield (cfs)	Critical Period (month & year	Yield (cfs)	Critical Period (month & year)
5,800,000 5,400,000 4,500,000 3,400,000 3,400,000 1,440,000 700,000 700,000 700,000 700,000	10,652 10,652 10,652 10,004 10,223 10,004 10,223 10,004 10,223 10,004 10,223 10,004 10,223 10,004 10,0000 10,0000 10,0000 10,0000 10,0000 10,0000 10,00000000	5-28 to 10-31 5-28 to 10-31 5-28 to 10-31 5-28 to 10-31 5-28 to 10-31 4-40 to 10-41 4-40 to 10-41 4-40 to 10-41 4-40 to 10-41 5-40 to 10-41 5-40 to 10-41 5-40 to 10-41 5-40 to 10-40 5-40 to 10-40 5-40 to 10-40	10,80 10,482 10,482 10,125 9,420 9,420 9,420 9,420 9,420 9,237 9,247 9,2	5-28 to 10-31 5-28 to 10-31 5-28 to 10-31 5-28 to 10-31 5-28 to 10-31 5-28 to 10-31 5-28 to 10-33 5-44 to 10-41 5-44 to 10-410 t	10,120 9,968 9,968 9,969 9,966 7,747 7,474 650 857 857 857 857 857 857 857 857 857 857	5-28 to 10-31 5-28 to 10-31 4-40 to 10-31 4-40 to 10-41 5-28 to 10-31 5-40 to 10-41 5-40 to 10-41 5-40 to 10-41 5-40 to 10-40 5-40 to 10-400 5-40 to 10-400 to 10-400 5-40 to 10-400 5-40 to 10-400 5-40 to 10-400	10,111 10,112 10,112 10,112 10,112 10,112 10,112 10,112 10,112 10,112 10,112 10,112 10,112 10,112 10,112 10,112 10,112 10,111 10,112 10,111	5-28 to 10-31 5-28 to 10-31 5-28 to 10-31 5-28 to 10-31 4-40 to 10-41 5-40 to 10-41 5-40 to 10-41 5-40 to 10-40 5-40 to 10-40
300,000 100,000 50,000 20,000	3,658 2,837 2,580 2,356	6-40 to 10-40 6-40 to 10-40 6-40 to 9-40 7-40 to 9-40	3,648 2,830 2,572 2,349	ပိုရီစစ	2,185 1,628 1,450 1,233	\$ \$ \$ \$ \$	2,179 1,622 1,446 1,229	2222

TABLE 8 RESULTS OF STORAGE-YIELD ANALYSES LITTLE RIVER

Critical Period (month & year) 10-56 10-56 10-56 10-56 10-56 3-57 3-57 3-57 2-57 2-57 2-57 10-56 10-56 **10-54** 10-54 3-57 3-57 WITH EVAPORATION & SEASONAL VARIATION to to to to to to t 0 to to t C t t t t t t to t t to B 5 2-53 6-49 6-49 6-49 **1-54** 5-47 1-54 5-54 5-47 5-47 5-47 5-47 5-47 5-47 5-47 5-47 6-47 5-54 1237.0 1172.6 1108.4 1044.6 917.8 520.8 480.4 727.0 599.6 553.8 274.3 854.2 411.7 320.2 70.7 981.1 7.067 175.1 Yield (cfs) Critical Period WITHOUT EVAPORATION - WITH (month & year) 10-56 10-56 10-56 10-56 10-56 10-56 10-56 1-35 3-57 10-54 3-57 3-57 3-57 3-57 2-57 2-57 2-57 3-57 SEASONAL VARIATION t t t C to ţ to to to с С r C ţ ç Ç t C 2 t t t t t t t t 50 5-47 5-47 5-47 6-49 6-49 6-49 6-49 12-53 1-54 **1-5**4 5-54 5-54 5-47 5-47 5-47 5-47 5-47 5-47 1377.4 1237.4 1167.5 1097.5 1027.8 957.4 887.7 816.9 582.5 537.0 673.9 462.5 366.4 318.2 216.2 113.2 619.1 1307.2 Yield (cfs) WITH EVAPORATION - WITHOUT Critical Period (month & year) 10-56 10-56 1-55 3-57 3-57 3-52 3-57 3-57 3-57 3-57 3-57 3-57 2-57 3-57 3-57 3-57 3-57 3-57 SEASONAL VARIATION to to <del>С</del> t C t t to to t t to to t t t t to 2 t0 to 2 t t 6-49 6-49 6-49 2-53 12-53 5-47 12-53 5-47 5-47 5-47 5-47 6-47 **1-5**4 5-47 5-47 6-51 5-47 5-47 1111.4 1047.4 856.8 L240.4 1175.8 983.7 920.3 729.9 605.0 793.4 530.5 491.2 98.3 Yield 562.1 421.1 330.3 283.7 187.2 (cfs) Critical Period (month & year) WITHOUT EVAPORATION OR 3-57 3-57 3-57 2-57 3-52 1-55 3-52 3-57 3-57 3-57 3-57 3-57 3-57 3-57 3-57 3-57 3-57 10-56 SEASONAL VARIATION t 0 t t t t t t t t t t <u>с</u> t t t t t t t t t t t t to to t t to to 2-53 12-53 9-50 12-53 6-49 6-49 6-49 5-47 6-51 5-47 5-47 6-47 6-47 5-47 5-47 5-47 5-47 5-47 L382.8 1312.1 1241.8 1171.5 679.0 626.4 466.9 224.8 820.7 590.9 546.4 375.9 321.8 133.8 Yield (cfs) 961.1 891.1 101.8 031.6 USABLE STORAGE (acre-feet) 6,000,000\* 4,000,000 3,500,000 3,000,000 6,500,000\* 200,000 100,000 50,000 5,500,000 5,000,000 4,500,000 2,500,000 1,500,000 1,200,000 1,000,000 750,000 500,000 300,000

\* With this amount of storage the yield is directly dependent upon the storage and the storage, once drawn down, cannot be completely replenished from natural streamflow.

# GRAND AVERAGE

CES END OF MONTH STORAGE IN AC-FT AC-FT CFS TO PIPELINE PER INFIOW MAX MIN BUFFER ACTUAL EVAP REQ ACTUAL SHRTG YR 14274 100000 3500000 **--**6 3 0 0 0 SHOR TAGE INDEX, PIPELINE O. OUTLET .197 DOWNSTREAM O. POWER O. TRIAL NO. 1 TRIAL VIELD IS 10999.9 BEGINNING OF CRITICAL PERIOD IS 5-1928 TIME OF MAXIMUM DRANDOWN IS 10-1931 MINIMUM STORAGE IS 200000 NEXT TRIAL YIELD IS 9836.87 GRAND AVERAGE END OF MONTH STORAGE IN AC-FT AC-FT CFS CPS TO PIPELINE MIN BUFFER ACTUAL PER INFLOW ΜΑΧ ΕVΑΡ REQ ACTUAL SHRTG YR 14274 100000 3500000 -614 0 0  $\cap$ SHORTAGE INDEX, PIPELINE O, OUTLET ,000 DOWNSTREAM O, POWER 0. TRIAL NO. 2 TRIAL YIELD IS 9836.9 BEGINNING OF CRITICAL PERIOD IS 4-1940 TIME OF MAXIMUM DRAWDOWN IS 10-1941 MINIMUM STORAGE IS 100000 NEXT TRIAL YIELD IS 9783.50

GRAND AVERAGE

CFS END OF MONTH STORAGE IN AC-FT AC-FT CFS TO PIPELINE MIN BJFFER ACTUAL PER INFLOW MAX εναρ RED ACTUAL SHRTS YR 14274 100000 3500000 **-** 👌 🖸 <u>()</u> . 0 0 SHORTAGE INDEX, PIPELINE 0. OUTLET .000 DOWNSTREAM 0. POWER 0. TRIAL NO. 3 TRIAL YIELD IS 9783.5 BEGINNING OF CRITICAL PERIOD IS 4-1940 TIME OF MAXIMUM DRAHDOMN IS 10-1941

MINIMUM STORAGE IS 100000

FIGURE 1



STORAGE IN 1000 AC.-FT.



FIGURE 3

YIELD IN CFS



STORAGE IN 100,000 AC.-FT.

FIGURE 4

 $\mathbb{C}^{n} \mathbb{C}^{n}$ 



YIELD IN CFS

STORAGE IN 1000 AC. +FT.

FIGURE 5



STORAGE IN 100,000 AC.-FT.

FIGURE 6



STORAGE IN 1,000,000 AC.-FT.

ę.,



FIGURE 8

STORAGE IN 1,000,000 AC.-FT.

# **Technical Paper Series**

- TP-1 Use of Interrelated Records to Simulate Streamflow TP-2 Optimization Techniques for Hydrologic Engineering TP-3 Methods of Determination of Safe Yield and Compensation Water from Storage Reservoirs TP-4 Functional Evaluation of a Water Resources System TP-5 Streamflow Synthesis for Ungaged Rivers TP-6 Simulation of Daily Streamflow TP-7 Pilot Study for Storage Requirements for Low Flow Augmentation TP-8 Worth of Streamflow Data for Project Design - A Pilot Study TP-9 Economic Evaluation of Reservoir System Accomplishments Hydrologic Simulation in Water-Yield Analysis **TP-10 TP-11** Survey of Programs for Water Surface Profiles **TP-12** Hypothetical Flood Computation for a Stream System **TP-13** Maximum Utilization of Scarce Data in Hydrologic Design **TP-14** Techniques for Evaluating Long-Tem Reservoir Yields **TP-15** Hydrostatistics - Principles of Application **TP-16** A Hydrologic Water Resource System Modeling Techniques Hydrologic Engineering Techniques for Regional **TP-17** Water Resources Planning **TP-18** Estimating Monthly Streamflows Within a Region **TP-19** Suspended Sediment Discharge in Streams **TP-20** Computer Determination of Flow Through Bridges TP-21 An Approach to Reservoir Temperature Analysis **TP-22** A Finite Difference Methods of Analyzing Liquid Flow in Variably Saturated Porous Media **TP-23** Uses of Simulation in River Basin Planning **TP-24** Hydroelectric Power Analysis in Reservoir Systems **TP-25** Status of Water Resource System Analysis **TP-26** System Relationships for Panama Canal Water Supply **TP-27** System Analysis of the Panama Canal Water Supply **TP-28** Digital Simulation of an Existing Water Resources System **TP-29** Computer Application in Continuing Education **TP-30** Drought Severity and Water Supply Dependability TP-31 Development of System Operation Rules for an Existing System by Simulation **TP-32** Alternative Approaches to Water Resources System Simulation **TP-33** System Simulation of Integrated Use of Hydroelectric and Thermal Power Generation **TP-34** Optimizing flood Control Allocation for a Multipurpose Reservoir **TP-35** Computer Models for Rainfall-Runoff and River Hydraulic Analysis **TP-36** Evaluation of Drought Effects at Lake Atitlan **TP-37** Downstream Effects of the Levee Overtopping at Wilkes-Barre, PA, During Tropical Storm Agnes **TP-38** Water Quality Evaluation of Aquatic Systems
- TP-39 A Method for Analyzing Effects of Dam Failures in Design Studies
- TP-40 Storm Drainage and Urban Region Flood Control Planning
- TP-41 HEC-5C, A Simulation Model for System Formulation and Evaluation
- TP-42 Optimal Sizing of Urban Flood Control Systems
- TP-43 Hydrologic and Economic Simulation of Flood Control Aspects of Water Resources Systems
- TP-44 Sizing Flood Control Reservoir Systems by System Analysis
- TP-45 Techniques for Real-Time Operation of Flood Control Reservoirs in the Merrimack River Basin
- TP-46 Spatial Data Analysis of Nonstructural Measures
- TP-47 Comprehensive Flood Plain Studies Using Spatial Data Management Techniques
- TP-48 Direct Runoff Hydrograph Parameters Versus Urbanization
- TP-49 Experience of HEC in Disseminating Information on Hydrological Models
- TP-50 Effects of Dam Removal: An Approach to Sedimentation
- TP-51 Design of Flood Control Improvements by Systems Analysis: A Case Study
- TP-52 Potential Use of Digital Computer Ground Water Models
- TP-53 Development of Generalized Free Surface Flow Models Using Finite Element Techniques
- TP-54 Adjustment of Peak Discharge Rates for Urbanization
- TP-55 The Development and Servicing of Spatial Data Management Techniques in the Corps of Engineers
- TP-56 Experiences of the Hydrologic Engineering Center in Maintaining Widely Used Hydrologic and Water Resource Computer Models
- TP-57 Flood Damage Assessments Using Spatial Data Management Techniques
- TP-58 A Model for Evaluating Runoff-Quality in Metropolitan Master Planning
- TP-59 Testing of Several Runoff Models on an Urban Watershed
- TP-60 Operational Simulation of a Reservoir System with Pumped Storage
- TP-61 Technical Factors in Small Hydropower Planning
- TP-62 Flood Hydrograph and Peak Flow Frequency Analysis
- TP-63 HEC Contribution to Reservoir System Operation
- TP-64 Determining Peak-Discharge Frequencies in an Urbanizing Watershed: A Case Study
- TP-65 Feasibility Analysis in Small Hydropower Planning
- TP-66 Reservoir Storage Determination by Computer Simulation of Flood Control and Conservation Systems
- TP-67 Hydrologic Land Use Classification Using LANDSAT
- TP-68 Interactive Nonstructural Flood-Control Planning
- TP-69 Critical Water Surface by Minimum Specific Energy Using the Parabolic Method

TP-70	Corps of Engineers Experience with Automatic
	Calibration of a Precipitation-Runoff Model
TP-71	Determination of Land Use from Satellite Imagery
	for Input to Hydrologic Models
TP-72	Application of the Finite Element Method to
	Vertically Stratified Hydrodynamic Flow and Water
	Quality
TP-73	Flood Mitigation Planning Using HEC-SAM
TP-74	Hydrographs by Single Linear Reservoir Model
TP-75	HEC Activities in Reservoir Analysis
TP-76	Institutional Support of Water Resource Models
TP-77	Investigation of Soil Conservation Service Urban Hydrology Techniques
TP-78	Potential for Increasing the Output of Existing
11-78	Hydroelectric Plants
TP-79	Potential Energy and Capacity Gains from Flood
11-77	Control Storage Reallocation at Existing U.S.
	Hydropower Reservoirs
TP-80	Use of Non-Sequential Techniques in the Analysis
11 00	of Power Potential at Storage Projects
TP-81	Data Management Systems of Water Resources
	Planning
TP-82	The New HEC-1 Flood Hydrograph Package
TP-83	River and Reservoir Systems Water Quality
	Modeling Capability
TP-84	Generalized Real-Time Flood Control System
	Model
TP-85	Operation Policy Analysis: Sam Rayburn
	Reservoir
TP-86	Training the Practitioner: The Hydrologic
	Engineering Center Program
TP-87	Documentation Needs for Water Resources Models
TP-88	Reservoir System Regulation for Water Quality
TD 90	Control
TP-89	A Software System to Aid in Making Real-Time Water Control Decisions
TP-90	Calibration, Verification and Application of a Two-
11-90	Dimensional Flow Model
TP-91	HEC Software Development and Support
TP-92	Hydrologic Engineering Center Planning Models
TP-93	Flood Routing Through a Flat, Complex Flood
	Plain Using a One-Dimensional Unsteady Flow
	Computer Program
TP-94	Dredged-Material Disposal Management Model
TP-95	Infiltration and Soil Moisture Redistribution in
	HEC-1
TP-96	The Hydrologic Engineering Center Experience in
	Nonstructural Planning
TP-97	Prediction of the Effects of a Flood Control Project
	on a Meandering Stream
TP-98	Evolution in Computer Programs Causes Evolution
	in Training Needs: The Hydrologic Engineering
<b>TD</b> 00	Center Experience
TP-99	Reservoir System Analysis for Water Quality
TP-100	Probable Maximum Flood Estimation - Eastern United States
TP-101	
1P-101	Use of Computer Program HEC-5 for Water Supply
TP-102	Analysis Role of Calibration in the Application of HEC-6
TP-102 TP-103	Engineering and Economic Considerations in
11-105	Formulating
TP-104	Modeling Water Resources Systems for Water
-0.	Quality

- TP-105 Use of a Two-Dimensional Flow Model to Quantify Aquatic Habitat
- TP-106 Flood-Runoff Forecasting with HEC-1F
- TP-107 Dredged-Material Disposal System Capacity Expansion
- TP-108 Role of Small Computers in Two-Dimensional Flow Modeling
- TP-109 One-Dimensional Model for Mud Flows
- TP-110 Subdivision Froude Number
- TP-111 HEC-5Q: System Water Quality Modeling
- TP-112 New Developments in HEC Programs for Flood Control
- TP-113 Modeling and Managing Water Resource Systems for Water Quality
- TP-114 Accuracy of Computer Water Surface Profiles -Executive Summary
- TP-115 Application of Spatial-Data Management Techniques in Corps Planning
- TP-116 The HEC's Activities in Watershed Modeling
- TP-117 HEC-1 and HEC-2 Applications on the Microcomputer
- TP-118 Real-Time Snow Simulation Model for the Monongahela River Basin
- TP-119 Multi-Purpose, Multi-Reservoir Simulation on a PC
- TP-120 Technology Transfer of Corps' Hydrologic Models
- TP-121 Development, Calibration and Application of Runoff Forecasting Models for the Allegheny River Basin
- TP-122 The Estimation of Rainfall for Flood Forecasting Using Radar and Rain Gage Data
- TP-123 Developing and Managing a Comprehensive Reservoir Analysis Model
- TP-124 Review of U.S. Army corps of Engineering Involvement With Alluvial Fan Flooding Problems
- TP-125 An Integrated Software Package for Flood Damage Analysis
- TP-126 The Value and Depreciation of Existing Facilities: The Case of Reservoirs
- TP-127 Floodplain-Management Plan Enumeration
- TP-128 Two-Dimensional Floodplain Modeling
- TP-129 Status and New Capabilities of Computer Program HEC-6: "Scour and Deposition in Rivers and Reservoirs"
- TP-130 Estimating Sediment Delivery and Yield on Alluvial Fans
- TP-131 Hydrologic Aspects of Flood Warning -Preparedness Programs
- TP-132 Twenty-five Years of Developing, Distributing, and Supporting Hydrologic Engineering Computer Programs
- TP-133 Predicting Deposition Patterns in Small Basins
- TP-134 Annual Extreme Lake Elevations by Total Probability Theorem
- TP-135 A Muskingum-Cunge Channel Flow Routing Method for Drainage Networks
- TP-136 Prescriptive Reservoir System Analysis Model -Missouri River System Application
- TP-137 A Generalized Simulation Model for Reservoir System Analysis
- TP-138 The HEC NexGen Software Development Project
- TP-139 Issues for Applications Developers
- TP-140 HEC-2 Water Surface Profiles Program
- TP-141 HEC Models for Urban Hydrologic Analysis

- TP-142 Systems Analysis Applications at the Hydrologic Engineering Center
- TP-143 Runoff Prediction Uncertainty for Ungauged Agricultural Watersheds
- TP-144 Review of GIS Applications in Hydrologic Modeling
- TP-145 Application of Rainfall-Runoff Simulation for Flood Forecasting
- TP-146 Application of the HEC Prescriptive Reservoir Model in the Columbia River Systems
- TP-147 HEC River Analysis System (HEC-RAS)
- TP-148 HEC-6: Reservoir Sediment Control Applications
- TP-149 The Hydrologic Modeling System (HEC-HMS): Design and Development Issues
- TP-150 The HEC Hydrologic Modeling System
- TP-151 Bridge Hydraulic Analysis with HEC-RAS
- TP-152 Use of Land Surface Erosion Techniques with Stream Channel Sediment Models

- TP-153 Risk-Based Analysis for Corps Flood Project Studies - A Status Report
- TP-154 Modeling Water-Resource Systems for Water Quality Management
- TP-155 Runoff simulation Using Radar Rainfall Data
- TP-156 Status of HEC Next Generation Software Development
- TP-157 Unsteady Flow Model for Forecasting Missouri and Mississippi Rivers
- TP-158 Corps Water Management System (CWMS)
- TP-159 Some History and Hydrology of the Panama Canal
- TP-160 Application of Risk-Based Analysis to Planning Reservoir and Levee Flood Damage Reduction Systems
- TP-161 Corps Water Management System Capabilities and Implementation Status