



**US Army Corps  
of Engineers**

Hydrologic Engineering Center

---

# **Methods for Determination of Safe Yield and Compensation Water from Storage Reservoirs**

**October 1966**

# REPORT DOCUMENTATION PAGE

Form Approved OMB No. 0704-0188

The public reporting burden for this collection of information is estimated to average 1 hour per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. Send comments regarding this burden estimate or any other aspect of this collection of information, including suggestions for reducing this burden, to the Department of Defense, Executive Services and Communications Directorate (0704-0188). Respondents should be aware that notwithstanding any other provision of law, no person shall be subject to any penalty for failing to comply with a collection of information if it does not display a currently valid OMB control number.

**PLEASE DO NOT RETURN YOUR FORM TO THE ABOVE ORGANIZATION.**

<b>1. REPORT DATE</b> (DD-MM-YYYY) August 1965		<b>2. REPORT TYPE</b> Technical Paper		<b>3. DATES COVERED</b> (From - To)	
<b>4. TITLE AND SUBTITLE</b> Methods for Determination of Safe Yield and Compensation Water from Storage Reservoirs			<b>5a. CONTRACT NUMBER</b>		
			<b>5b. GRANT NUMBER</b>		
			<b>5c. PROGRAM ELEMENT NUMBER</b>		
			<b>5d. PROJECT NUMBER</b>		
<b>6. AUTHOR(S)</b> Leo R. Beard			<b>5e. TASK NUMBER</b>		
			<b>5f. WORK UNIT NUMBER</b>		
<b>7. PERFORMING ORGANIZATION NAME(S) AND ADDRESS(ES)</b> US Army Corps of Engineers Institute for Water Resources Hydrologic Engineering Center (HEC) 609 Second Street Davis, CA 95616-4687				<b>8. PERFORMING ORGANIZATION REPORT NUMBER</b> TP-3	
<b>9. SPONSORING/MONITORING AGENCY NAME(S) AND ADDRESS(ES)</b>				<b>10. SPONSOR/ MONITOR'S ACRONYM(S)</b>	
				<b>11. SPONSOR/ MONITOR'S REPORT NUMBER(S)</b>	
<b>12. DISTRIBUTION / AVAILABILITY STATEMENT</b> Approved for public release; distribution is unlimited.					
<b>13. SUPPLEMENTARY NOTES</b> Presented at Seventh International Water Supply Congress, 3-7 October 1966, Barcelona, Spain.					
<b>14. ABSTRACT</b> The simple mass-curve types of analyses are no longer practical for the design of reservoir projects because of the complex operation factors that must now be considered. The procedures used in reservoirs of all types and of regulated streamflows downstream of reservoirs consist generally of detailed routing studies using historical streamflow sequences. There is considerable effort currently being devoted to investigating the potential application of stochastic hydrology to reservoir planning and design studies.					
<b>15. SUBJECT TERMS</b> reservoir yield, reservoir design, reservoir storage, water shortage					
<b>16. SECURITY CLASSIFICATION OF:</b>			<b>17. LIMITATION OF ABSTRACT</b> UU	<b>18. NUMBER OF PAGES</b> 24	<b>19a. NAME OF RESPONSIBLE PERSON</b>
<b>a. REPORT</b> U	<b>b. ABSTRACT</b> U	<b>c. THIS PAGE</b> U			<b>19b. TELEPHONE NUMBER</b>

# **Methods for Determination of Safe Yield and Compensation Water from Storage Reservoirs**

**October 1966**

US Army Corps of Engineers  
Institute for Water Resources  
Hydrologic Engineering Center  
609 Second Street  
Davis, CA 95616

(530) 756-1104  
(530) 756-8250 FAX  
[www.hec.usace.army.mil](http://www.hec.usace.army.mil)

TP-3

Papers in this series have resulted from technical activities of the Hydrologic Engineering Center. Versions of some of these have been published in technical journals or in conference proceedings. The purpose of this series is to make the information available for use in the Center's training program and for distribution with the Corps of Engineers.

The findings in this report are not to be construed as an official Department of the Army position unless so designated by other authorized documents.

The contents of this report are not to be used for advertising, publication, or promotional purposes. Citation of trade names does not constitute an official endorsement or approval of the use of such commercial products.

16 August 1965

METHODS FOR DETERMINATION OF SAFE YIELD AND COMPENSATION  
WATER FROM STORAGE RESERVOIRS (a)

LEO R. BEARD (b) - U.S.A.

INTRODUCTION

Methods used in the United States for the determination of safe yield from reservoirs are governed by the complexity of the water resources development project. It is rare when simple mass-curve types of analysis are practical, as they may have been in years past. The trend toward multiple use of reservoir space and the consideration of prior rights and legal restrictions dictate that a detailed sequential analysis of inflows, outflows and other operation factors be made.

In general, the same principles are used in the determination of reservoir yield, regardless of the nature or type of water supply reservoir. The three types of reservoirs considered are designated for purposes of this report as: impounding reservoirs from which water supply is piped direct to destination, regulating reservoirs from which water is released to the river for downstream use, and pump storage reservoirs, into which water is diverted or pumped from the river for future use.

- (a) For use in preparation of General Report No. 2 for Seventh International Water Supply Congress, Barcelona, Spain, 3-7 October 1966. Not for publication as a separate paper without prior approval of the Chief of Engineers, U.S. Army.
- (b) Chief, Hydrologic Engineering Center, Corps of Engineers, U.S. Army, Sacramento, California.

The basic procedure used throughout the United States consists of performing a reservoir routing study or reservoir simulation study, with the assumption that historical inflows (exact historical sequences) will recur in the future, or that they represent an adequate test for design purposes. It has been recognized for many years that this assumption leaves much to be desired. It has been demonstrated, for example, <sup>(5)</sup> that storage determinations based on different halves of the same long records differ by a factor of 2 or more in half of the cases where high firm yields are required. Accordingly, considerable work has been done toward simulating streamflow sequences, with the expectation that a number of artificial sequences can be studied rather than the single sequences of record, thus providing more dependable estimates of firm yield. However, there is no record in literature of an actual design being based on such simulated streamflow series.

#### IMPOUNDING RESERVOIRS

Analysis of firm yield for a single impounding reservoir (i.e., from which water is piped direct to destination) represents a relatively simple type of yield analysis. However, even such projects usually present complex problems. Inasmuch as the seasonal variations of runoff and of use are different, the application of some simplified procedures are impractical. Evaporation and other losses are often an important element, and cannot be determined accurately without a detailed analysis of reservoir stages. Where power generation is included as a project function, reservoir stages also affect the amount of release from time to time.

Where storage is to be provided only for seasonal and short-term regulation of runoff (where water need not be retained from one year to the next), yield determinations can in simple cases be based on studies of the frequency of low flows. Frequency curves of annual minimum runoff volumes for durations ranging from a few days to a year can be used to establish the probability of drought. For a specified probability, a curve of minimum runoff versus duration can be constructed. A straight line with slope equal to the desired yield is then made tangent to this curve, and the negative intercept of that line represents the required storage. This type of analysis is simple and introduces the probability concept, but is practical only in cases where water use does not vary seasonally.

Duration curves of runoff are used in some cases of design for run-of-river power plants, but are of decreasing value, because of seasonal and short-term variations in power requirements. They are not used effectively in water yield studies.

For long-term storage (storage carried for more than 1 year) and for short-term storage where there is seasonal variation in use, the simplified analysis represented by the Rippl diagram might be used. However, as stated above, complex variations in evaporation, power generation, water rights, flood control, and other factors usually justify the more detailed analysis wherein a month-by-month routing (operation simulation study) showing reservoir stages, evaporation,

and other factors as well as inflow and outflow is made. Usually such routings are made on a monthly basis, with special consideration being given to daily variations in supply and to flood control.

Even in the case of single impounding reservoirs, there are often factors to be considered that require detailed studies of sequences. Stages of the reservoir during the recreation season and during the flood-control season are often governed by conditions of authorization. Some stage fluctuations for mosquito control may be required. There is usually a requirement to release water to the river to satisfy prior rights, quality control and fish requirements. The temperature of water released for fish, industrial purposes, or irrigation may impose some restrictions on the reservoir operation stage. This item has become increasingly important in recent years.

In general, then, the determination of safe yield from impounding reservoirs might be based on simple procedures such as the low-flow frequency analysis and Rippl diagram, but would usually be based on a detailed study of streamflow sequences, using reservoir routing procedures, because of the complexity of many of the factors affecting reservoir operation. Such a study is outlined in the section entitled "General Procedure".

#### REGULATING RESERVOIRS

Determination of safe yield and compensation water (low-flow regulation) for regulating reservoirs is considerably more complex than yield determinations for impounding reservoirs. This is generally due to the fact that



uses at the diversion point can be satisfied fully or in part by runoff contribution from areas tributary below the reservoir. Also, there are cases where percolation loss in the channel between the reservoir and the point of diversion greatly affect the supply. Accordingly, it is general practice in the United States to base yield determinations for regulating reservoirs on detailed monthly routing studies, giving due consideration to runoff intermediate between the reservoir and diversion point or points and to percolation losses in the channel, where appreciable.

In many cases of regulating reservoirs, there are interests who own water rights that are not being served by the project. Often such water rights have a variety of priority, sometimes intermixed with those served by the project. Consequently, it would be theoretically necessary to perform day-by-day analysis of inflows to determine the daily amounts of water owned by project entities and surplus water that can legally be stored in the reservoir. However, this is usually accomplished on a monthly basis, using approximate relationships derived from examination of daily streamflows. When periods of low flow occur in the operation of regulating reservoirs, streams contributing below the reservoir, as well as inflow to the reservoir, are low. The requirement at the point of diversion is satisfied only by releasing sufficient water to overcome the shortage of intermediate flow as well as to provide the supplemental supply. Thus the regulating storage requirement is a function of total supply above the point of diversion less that portion of the intermediate runoff that exceeds the requirement at the point of diversion.

Regulating reservoirs usually serve more than one function; consequently there is more frequent need for detailed analysis of this type of reservoir. The routing or simulation study can involve dozens of columns containing variables concerned in the study. A complete simulation study would include consideration of the monthly requirement at each point of diversion, varying runoff contribution and losses between these points and the reservoir, flood control and power requirements at the reservoir, flows or stages at critical river locations downstream of the reservoir where flood control, water quality, navigation, or fish requirements are specified, evaporation from the reservoir, and the deliveries to each point of diversion where prior water rights must be satisfied.

In general, then, it can be stated that determination of safe yield for regulating reservoirs usually requires a detailed monthly analysis of many factors. Such analysis is similar to, but usually more complex than, analyses for impounding reservoirs.

#### PUMPED-STORAGE RESERVOIRS

Pumped-storage or off-stream reservoirs usually serve a single purpose, and are consequently more easily studied than are the other two types of reservoirs. In general, all water above river flows required to satisfy prior rights can be diverted to the reservoir, up to diversion or pump capacity rates. Inasmuch as prior rights usually vary seasonally and often with rate of streamflow, and pump or diversion capacity may vary with head, simplified procedures cannot always be applied in such safe-yield

determinations. In general, a monthly routing of streamflows at the point of diversion and through the proposed reservoir is required. Such a routing or system simulation study would be similar to those made for the more complex studies of impounding and regulating reservoirs.

#### GENERAL PROCEDURE

Detailed routing studies require considerable time and effort, but this is of minor consideration in relation to other project design and construction requirements. Performance of a detailed project routing or simulation study is accomplished in the following steps:

a. Determine non-project streamflow at each reservoir site and at downstream points of diversion. Non-project conditions (often termed pre-project conditions) are defined as those that are expected to prevail during the lifetime of the proposed project if the project is not built. This determination requires consideration of all non-project reservoir and diversion effects in the river basin that may differ in the future from their historical effects. It also requires estimating streamflows where records were not obtained and estimating the effects of known future water regulatory structures that will be built whether or not the contemplated project is built.

b. Determine monthly firm supply required at each reservoir and at each point of diversion or point of low-flow regulation. Where these values depend on streamflow or runoff, as in the case of some water rights, they may be expressed as a function of those quantities. Power requirements at a reservoir may be expressed as energy so that reservoir release

requirement would depend on reservoir head at the time. Quality-control requirements may be expressed in parts per million maximum concentration of various pollutants, so that reservoir releases would depend on the amount and quality of water downstream as well as quality of released water.

c. Determine monthly reservoir stage limitations governed by flood-control, recreation and minimum-power-pool requirements. Reservoir level and release fluctuations for mosquito abatement should be considered, if appropriate.

d. Determine expected average monthly evaporation and seepage loss expressed as a depth over each reservoir area for each calendar month. This would ordinarily represent losses over and above those experienced in the reservoir area under non-project conditions, and can be negative under some circumstances.

e. Develop relation of river percolation losses to rate of flow, where such losses appreciably affect delivery quantities. Where necessary, antecedent channel conditions must be considered.

f. Develop multi-reservoir operation criteria. Where a water requirement can be served by more than one reservoir, develop rules for month-to-month selection of reservoir for such supply. These would be based on the existing reservoir stage in relation to multipurpose requirements at each reservoir and may vary seasonally.

g. Establish criteria for allowable shortages. These may differ for different types of supply such as irrigation, municipal,

industrial, power, navigation, quality control, etc. Rules for declaration of shortages should be in terms of information that would be available during actual operation.

h. Establish initial reservoir stage. Ordinarily, a project is designed on the basis of requirements during the most severe drought period recorded, and the initial reservoir stage for each reservoir is selected as full to top of conservation pool (bottom of flood-control space, if any) at the start of that period. As a more general rule, the reservoir should be assumed empty at start of operation, as it will be in actual use, when tested over its entire expected lifetime.

i. Perform routing operation, determining first the month-by-month release required from each reservoir to satisfy all needs and then subtracting the release and evaporation and seepage losses from the sum of storage and inflow. Where storage is inadequate to supply a requirement or where rules call for a shortage, tabulate the amount of shortage.

The routing study can be complicating by factors such as variable water-quality requirements related to flows of highly saline streams, along with incomplete mixing within each reservoir, varying and differing water temperature requirements for fish, irrigation and industry, and the necessity to forecast streamflows intermediate between the reservoirs and diversion points and into the reservoirs when shortages are pending or when surpluses might be declared. Irrigation requirement in actual practice will depend on rainfall in the agricultural area. One inch of rain more or

less over 50,000 acres of irrigated area can change the reservoir release requirement by 4,000 or 5,000 acre-feet. Because of the many complicating factors, it is essential that the routing study include contingency allowances for operation uncertainties in order to assure realistic simulation of actual operating conditions. These are sometimes introduced as a minimum storage pool to be used in extreme emergencies only, but are more realistically introduced as error allowances in each component operation.

#### ALLOWABLE SHORTAGES

The safe yield provided by a reservoir is usually defined as the annual flow delivered by the project without causing intolerable shortages under recurrence of historical droughts. In the case of municipal and industrial (M&I) supplies, it is usual practice to permit no shortage, and in some cases even to provide a reserve storage in the event of droughts that exceed the worst of record. In the case of irrigation supplies, shortages can usually be anticipated and can be sustained at infrequent intervals. Some irrigation projects have been designed to permit shortages of 25 to 35% in 4 of the years during the most severe drought of record. One-year shortages as large as 50% have been permitted. It has been found that a 10% irrigation shortage usually results in little or no damage, that a reduced crop can be obtained with a 25% shortage, and that perennials can be sustained without a usable crop during a season of 50% shortage. These findings would vary, of course, with types of crops, irrigation practice and geographic location.

There is increasing recognition of the fact that shortages more severe than the worst historical drought can occur. This is leading toward design criteria based on probability of shortage or expected shortage. Quality control determinations are usually based on the probability that supplies will meet required streamflows. Beyond this, there has been very little application of probability or expected-shortage criteria in the design of actual projects in the United States. As studies of streamflow simulation come into use in reservoir design, such criteria will be essential, because it is always possible that a more severe sequence of flows will occur than has heretofore been observed or anticipated. A convenient shortage index has been proposed<sup>(4)</sup> that should be helpful in design and planning studies. This is the sum of the squares of annual shortage ratios over a 100-year period. An index of 1.0, for example, would permit 100 10-percent shortages or four 50-percent shortages or one 100-percent shortage in 100 years. A more sophisticated treatment would balance cost of additional storage against benefits expected from decreased shortage.

#### NEW APPROACHES

The need for new approaches that eliminate the assumption of recurrence of historical streamflows in the exact sequences of record is obtaining increased recognition in the United States. During recent years, procedures of yield analysis such as the queueing theory have been developed. This theory or procedure utilizes the concept of probabilities of inflows and resulting probabilities of storages and outflows in such a manner that storage determinations are independent of exact historical sequences. The procedure is practical only for simple cases where use

is uniform and where factors such as reservoir evaporation, flood control, etc., are not significant. There are no published cases of actual design by such methods.

Increasing attention is being given by universities and water resource agencies to the potential application of streamflow simulation procedures (stochastic hydrology) to reservoir planning and design studies. Essentially, historical runoff is analyzed into its various frequency and correlation characteristics. Frequency and correlation characteristics derived from recorded data are then used to generate new series of correlated events that can as likely occur in the future as could recurrence of historical events. In this manner, numerous complete sequences of streamflows can be used for project analysis, instead of one historical set of data that usually are partially estimated and of duration shorter than desired for analysis. Such hypothetical (or synthetic) flows are of reliability limited by sample uncertainties of recorded data, but provide a greater variety of potential sequences and consequently permit more dependable estimates of safe yield.

Design procedures employing simulated streamflows would be identical to the detailed routing procedures that employ historical streamflows as a basis for study. Although no actual design based on simulated streamflows has been reported in literature, current thinking is that analyses would be based on historical streamflows as well as 10 or 20 simulation sequences of 50 to 100 years each. The determinations based on each of the many series would be compared and can reveal potential weaknesses of some of the contemplated designs. Ultimately, expected-benefits or expected-yields,



representing the average accomplishment indicated by many simulation sequences, might be adopted as a basis of design.

Use of simulated streamflows in design would greatly increase the amount of computation required. Consequently, it will be necessary that routing or system simulation studies would be accomplished by use of electronic computers. Once a routing study is programmed for electronic computers, it will be feasible to make many studies of different components in the design, inasmuch as the computation work becomes relatively inexpensive. A few computer programs have been developed for this purpose in the United States. These have been designed essentially along the lines of the hand computation procedures and are used for routing historical streamflows.

It has been found that "digesting" the results of computer studies requires far more time and expense than does computation of the results in the computer. Consequently, there is increasing attention being given to potential optimization of project accomplishments within the computer. This will consist essentially of formulating equations or computation procedures expressing project accomplishments as a single index (the objective function) in terms of various project components. By repeated analysis of the system and calculated changes in each component, the computer would maximize the accomplishments index. Although much "optimization" work is in progress, only a few reports on the subject have been published. (8,11,13)

## CONCLUSION

In summary, it can be stated that procedures used in the United States for determination of safe yields from reservoirs of all types and of regulated streamflows downstream of reservoirs consist generally of detailed routing (simulation) studies using historical streamflow sequences, modified for expected non-project changes, as a measure of what may occur in the future. There is considerable effort currently devoted to obtaining more dependable measures of potential streamflow by use of streamflow simulation procedures.

#### SELECTED BIBLIOGRAPHY

- (1) ASCE Committee on Water Resources Policy, Progress Report, "Basic Considerations in Water Resources Planning", Hydraulics Division Journal, Sept 1962.
- (2) American Water Works Association Panel Discussion, "Safe Yield From Surface Storage Reservoirs", Journal of the American Water Works Association, September 1950.
- (3) American Water Works Association Panel Discussion, "Controlled Draft From Reservoirs", Journal of the American Water Works Association, April 1956.
- (4) Beard, Leo R., "Estimating Long-Term Storage Requirements and Firm Yield of Rivers", General Assembly of Berkeley of International Union on Geodesy and Geophysics, 1964, Surface Waters, pp. 151-166.
- (5) Beard, Leo R., "Hydrologic Simulation Procedures in Water-Yield Analysis", International Committee on Irrigation and Drainage, El Paso, Texas, December 1964; New Delhi, January 1966.
- (6) Chow, Ven Te, "Handbook of Applied Hydrology", McGraw-Hill Book Co., 1964.
- (7) Fiering, Myron B., "Multivariate Technique for Synthetic Hydrology", ASCE Hydraulics Division Journal, September 1964.
- (8) Hall, Warren A., "Optimum Design of a Multiple Purpose Reservoir", ASCE Hydraulics Division Journal, July 1964.
- (9) Hurst, H.E., "Long-Term Storage Capacity of Reservoirs", ASCE Separate No. 11, April 1960.
- (10) Langbein, W.B., "Queueing Theory and Water Storage", ASCE Hydraulics Division Journal, October 1958.
- (11) Maass, Arthur, et.al., "Design of Water Resources Systems", Howard University Press, 1962.
- (12) Maughan, W. Don and Kawano, R.Y., "Project Yields by a Probability Method", ASCE Hydraulics Division Journal, May 1963.
- (13) Rosenbrock, H. H., "An Automatic Method of Finding the Greatest or Least Value of a Function", The Computer Journal, 1960, p. 175.
- (14) Select Committee on National Water Resources, United States Senate, "Water Resources Activities in the United States", August 1960.
- (15) Stall, John B. and Neill, James C., "Calculated Risks of Impounding Reservoir Yield", ASCE Hydraulics Division Journal, January 1963.
- (16) U.S. Bureau of Reclamation, "Reclamation Manual", Part 5, "Project Operation Studies".
- (17) U.S. Dept. of Interior, Bureau of Reclamation, "Design of Small Dams", 1960.



## Technical Paper Series

TP-1	Use of Interrelated Records to Simulate Streamflow	TP-39	A Method for Analyzing Effects of Dam Failures in Design Studies
TP-2	Optimization Techniques for Hydrologic Engineering	TP-40	Storm Drainage and Urban Region Flood Control Planning
TP-3	Methods of Determination of Safe Yield and Compensation Water from Storage Reservoirs	TP-41	HEC-5C, A Simulation Model for System Formulation and Evaluation
TP-4	Functional Evaluation of a Water Resources System	TP-42	Optimal Sizing of Urban Flood Control Systems
TP-5	Streamflow Synthesis for Ungaged Rivers	TP-43	Hydrologic and Economic Simulation of Flood Control Aspects of Water Resources Systems
TP-6	Simulation of Daily Streamflow	TP-44	Sizing Flood Control Reservoir Systems by System Analysis
TP-7	Pilot Study for Storage Requirements for Low Flow Augmentation	TP-45	Techniques for Real-Time Operation of Flood Control Reservoirs in the Merrimack River Basin
TP-8	Worth of Streamflow Data for Project Design - A Pilot Study	TP-46	Spatial Data Analysis of Nonstructural Measures
TP-9	Economic Evaluation of Reservoir System Accomplishments	TP-47	Comprehensive Flood Plain Studies Using Spatial Data Management Techniques
TP-10	Hydrologic Simulation in Water-Yield Analysis	TP-48	Direct Runoff Hydrograph Parameters Versus Urbanization
TP-11	Survey of Programs for Water Surface Profiles	TP-49	Experience of HEC in Disseminating Information on Hydrological Models
TP-12	Hypothetical Flood Computation for a Stream System	TP-50	Effects of Dam Removal: An Approach to Sedimentation
TP-13	Maximum Utilization of Scarce Data in Hydrologic Design	TP-51	Design of Flood Control Improvements by Systems Analysis: A Case Study
TP-14	Techniques for Evaluating Long-Term Reservoir Yields	TP-52	Potential Use of Digital Computer Ground Water Models
TP-15	Hydrostatistics - Principles of Application	TP-53	Development of Generalized Free Surface Flow Models Using Finite Element Techniques
TP-16	A Hydrologic Water Resource System Modeling Techniques	TP-54	Adjustment of Peak Discharge Rates for Urbanization
TP-17	Hydrologic Engineering Techniques for Regional Water Resources Planning	TP-55	The Development and Servicing of Spatial Data Management Techniques in the Corps of Engineers
TP-18	Estimating Monthly Streamflows Within a Region	TP-56	Experiences of the Hydrologic Engineering Center in Maintaining Widely Used Hydrologic and Water Resource Computer Models
TP-19	Suspended Sediment Discharge in Streams	TP-57	Flood Damage Assessments Using Spatial Data Management Techniques
TP-20	Computer Determination of Flow Through Bridges	TP-58	A Model for Evaluating Runoff-Quality in Metropolitan Master Planning
TP-21	An Approach to Reservoir Temperature Analysis	TP-59	Testing of Several Runoff Models on an Urban Watershed
TP-22	A Finite Difference Methods of Analyzing Liquid Flow in Variably Saturated Porous Media	TP-60	Operational Simulation of a Reservoir System with Pumped Storage
TP-23	Uses of Simulation in River Basin Planning	TP-61	Technical Factors in Small Hydropower Planning
TP-24	Hydroelectric Power Analysis in Reservoir Systems	TP-62	Flood Hydrograph and Peak Flow Frequency Analysis
TP-25	Status of Water Resource System Analysis	TP-63	HEC Contribution to Reservoir System Operation
TP-26	System Relationships for Panama Canal Water Supply	TP-64	Determining Peak-Discharge Frequencies in an Urbanizing Watershed: A Case Study
TP-27	System Analysis of the Panama Canal Water Supply	TP-65	Feasibility Analysis in Small Hydropower Planning
TP-28	Digital Simulation of an Existing Water Resources System	TP-66	Reservoir Storage Determination by Computer Simulation of Flood Control and Conservation Systems
TP-29	Computer Application in Continuing Education	TP-67	Hydrologic Land Use Classification Using LANDSAT
TP-30	Drought Severity and Water Supply Dependability	TP-68	Interactive Nonstructural Flood-Control Planning
TP-31	Development of System Operation Rules for an Existing System by Simulation	TP-69	Critical Water Surface by Minimum Specific Energy Using the Parabolic Method
TP-32	Alternative Approaches to Water Resources System Simulation		
TP-33	System Simulation of Integrated Use of Hydroelectric and Thermal Power Generation		
TP-34	Optimizing flood Control Allocation for a Multipurpose Reservoir		
TP-35	Computer Models for Rainfall-Runoff and River Hydraulic Analysis		
TP-36	Evaluation of Drought Effects at Lake Atitlan		
TP-37	Downstream Effects of the Levee Overtopping at Wilkes-Barre, PA, During Tropical Storm Agnes		
TP-38	Water Quality Evaluation of Aquatic Systems		

- TP-70 Corps of Engineers Experience with Automatic Calibration of a Precipitation-Runoff Model
- TP-71 Determination of Land Use from Satellite Imagery for Input to Hydrologic Models
- TP-72 Application of the Finite Element Method to Vertically Stratified Hydrodynamic Flow and Water Quality
- TP-73 Flood Mitigation Planning Using HEC-SAM
- TP-74 Hydrographs by Single Linear Reservoir Model
- TP-75 HEC Activities in Reservoir Analysis
- TP-76 Institutional Support of Water Resource Models
- TP-77 Investigation of Soil Conservation Service Urban Hydrology Techniques
- TP-78 Potential for Increasing the Output of Existing Hydroelectric Plants
- TP-79 Potential Energy and Capacity Gains from Flood Control Storage Reallocation at Existing U.S. Hydropower Reservoirs
- TP-80 Use of Non-Sequential Techniques in the Analysis of Power Potential at Storage Projects
- TP-81 Data Management Systems of Water Resources Planning
- TP-82 The New HEC-1 Flood Hydrograph Package
- TP-83 River and Reservoir Systems Water Quality Modeling Capability
- TP-84 Generalized Real-Time Flood Control System Model
- TP-85 Operation Policy Analysis: Sam Rayburn Reservoir
- TP-86 Training the Practitioner: The Hydrologic Engineering Center Program
- TP-87 Documentation Needs for Water Resources Models
- TP-88 Reservoir System Regulation for Water Quality Control
- TP-89 A Software System to Aid in Making Real-Time Water Control Decisions
- TP-90 Calibration, Verification and Application of a Two-Dimensional Flow Model
- TP-91 HEC Software Development and Support
- TP-92 Hydrologic Engineering Center Planning Models
- TP-93 Flood Routing Through a Flat, Complex Flood Plain Using a One-Dimensional Unsteady Flow Computer Program
- TP-94 Dredged-Material Disposal Management Model
- TP-95 Infiltration and Soil Moisture Redistribution in HEC-1
- TP-96 The Hydrologic Engineering Center Experience in Nonstructural Planning
- TP-97 Prediction of the Effects of a Flood Control Project on a Meandering Stream
- TP-98 Evolution in Computer Programs Causes Evolution in Training Needs: The Hydrologic Engineering Center Experience
- TP-99 Reservoir System Analysis for Water Quality
- TP-100 Probable Maximum Flood Estimation - Eastern United States
- TP-101 Use of Computer Program HEC-5 for Water Supply Analysis
- TP-102 Role of Calibration in the Application of HEC-6
- TP-103 Engineering and Economic Considerations in Formulating
- TP-104 Modeling Water Resources Systems for Water Quality
- TP-105 Use of a Two-Dimensional Flow Model to Quantify Aquatic Habitat
- TP-106 Flood-Runoff Forecasting with HEC-1F
- TP-107 Dredged-Material Disposal System Capacity Expansion
- TP-108 Role of Small Computers in Two-Dimensional Flow Modeling
- TP-109 One-Dimensional Model for Mud Flows
- TP-110 Subdivision Froude Number
- TP-111 HEC-5Q: System Water Quality Modeling
- TP-112 New Developments in HEC Programs for Flood Control
- TP-113 Modeling and Managing Water Resource Systems for Water Quality
- TP-114 Accuracy of Computer Water Surface Profiles - Executive Summary
- TP-115 Application of Spatial-Data Management Techniques in Corps Planning
- TP-116 The HEC's Activities in Watershed Modeling
- TP-117 HEC-1 and HEC-2 Applications on the Microcomputer
- TP-118 Real-Time Snow Simulation Model for the Monongahela River Basin
- TP-119 Multi-Purpose, Multi-Reservoir Simulation on a PC
- TP-120 Technology Transfer of Corps' Hydrologic Models
- TP-121 Development, Calibration and Application of Runoff Forecasting Models for the Allegheny River Basin
- TP-122 The Estimation of Rainfall for Flood Forecasting Using Radar and Rain Gage Data
- TP-123 Developing and Managing a Comprehensive Reservoir Analysis Model
- TP-124 Review of U.S. Army corps of Engineering Involvement With Alluvial Fan Flooding Problems
- TP-125 An Integrated Software Package for Flood Damage Analysis
- TP-126 The Value and Depreciation of Existing Facilities: The Case of Reservoirs
- TP-127 Floodplain-Management Plan Enumeration
- TP-128 Two-Dimensional Floodplain Modeling
- TP-129 Status and New Capabilities of Computer Program HEC-6: "Scour and Deposition in Rivers and Reservoirs"
- TP-130 Estimating Sediment Delivery and Yield on Alluvial Fans
- TP-131 Hydrologic Aspects of Flood Warning - Preparedness Programs
- TP-132 Twenty-five Years of Developing, Distributing, and Supporting Hydrologic Engineering Computer Programs
- TP-133 Predicting Deposition Patterns in Small Basins
- TP-134 Annual Extreme Lake Elevations by Total Probability Theorem
- TP-135 A Muskingum-Cunge Channel Flow Routing Method for Drainage Networks
- TP-136 Prescriptive Reservoir System Analysis Model - Missouri River System Application
- TP-137 A Generalized Simulation Model for Reservoir System Analysis
- TP-138 The HEC NexGen Software Development Project
- TP-139 Issues for Applications Developers
- TP-140 HEC-2 Water Surface Profiles Program
- TP-141 HEC Models for Urban Hydrologic Analysis

- TP-142 Systems Analysis Applications at the Hydrologic Engineering Center
- TP-143 Runoff Prediction Uncertainty for Ungauged Agricultural Watersheds
- TP-144 Review of GIS Applications in Hydrologic Modeling
- TP-145 Application of Rainfall-Runoff Simulation for Flood Forecasting
- TP-146 Application of the HEC Prescriptive Reservoir Model in the Columbia River Systems
- TP-147 HEC River Analysis System (HEC-RAS)
- TP-148 HEC-6: Reservoir Sediment Control Applications
- TP-149 The Hydrologic Modeling System (HEC-HMS): Design and Development Issues
- TP-150 The HEC Hydrologic Modeling System
- TP-151 Bridge Hydraulic Analysis with HEC-RAS
- TP-152 Use of Land Surface Erosion Techniques with Stream Channel Sediment Models
- TP-153 Risk-Based Analysis for Corps Flood Project Studies - A Status Report
- TP-154 Modeling Water-Resource Systems for Water Quality Management
- TP-155 Runoff simulation Using Radar Rainfall Data
- TP-156 Status of HEC Next Generation Software Development
- TP-157 Unsteady Flow Model for Forecasting Missouri and Mississippi Rivers
- TP-158 Corps Water Management System (CWMS)
- TP-159 Some History and Hydrology of the Panama Canal
- TP-160 Application of Risk-Based Analysis to Planning Reservoir and Levee Flood Damage Reduction Systems
- TP-161 Corps Water Management System - Capabilities and Implementation Status

