



**US Army Corps
of Engineers**

Hydrologic Engineering Center

Dredged-Material Disposal Management Model

January 1984

REPORT DOCUMENTATION PAGE

Form Approved OMB No. 0704-0188

The public reporting burden for this collection of information is estimated to average 1 hour per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. Send comments regarding this burden estimate or any other aspect of this collection of information, including suggestions for reducing this burden, to the Department of Defense, Executive Services and Communications Directorate (0704-0188). Respondents should be aware that notwithstanding any other provision of law, no person shall be subject to any penalty for failing to comply with a collection of information if it does not display a currently valid OMB control number.

PLEASE DO NOT RETURN YOUR FORM TO THE ABOVE ORGANIZATION.

1. REPORT DATE (DD-MM-YYYY) January 1984		2. REPORT TYPE Technical Paper		3. DATES COVERED (From - To)	
4. TITLE AND SUBTITLE Dredged-Material Disposal Management Model				5a. CONTRACT NUMBER	
				5b. GRANT NUMBER	
				5c. PROGRAM ELEMENT NUMBER	
6. AUTHOR(S) David T. Ford				5d. PROJECT NUMBER	
				5e. TASK NUMBER	
				5f. WORK UNIT NUMBER	
7. PERFORMING ORGANIZATION NAME(S) AND ADDRESS(ES) US Army Corps of Engineers Institute for Water Resources Hydrologic Engineering Center (HEC) 609 Second Street Davis, CA 95616-4687				8. PERFORMING ORGANIZATION REPORT NUMBER TP-94	
9. SPONSORING/MONITORING AGENCY NAME(S) AND ADDRESS(ES)				10. SPONSOR/ MONITOR'S ACRONYM(S)	
				11. SPONSOR/ MONITOR'S REPORT NUMBER(S)	
12. DISTRIBUTION / AVAILABILITY STATEMENT Approved for public release; distribution is unlimited.					
13. SUPPLEMENTARY NOTES Published in ASCE Journal of Water Resources Planning and Management Division, Vol. 16, No.1, pp. 57-74, January 1984.					
14. ABSTRACT To identify efficient dredged-material disposal management strategies for the Delaware River navigation system near Philadelphia, the system operation problem is formulated and solved as a generalized minimum cost network flow programming problem. This formulation represents material sources and available disposal sites as nodes of the network and transportation links and carry-over storages as arcs. The dewatering, consolidation, and densification of dredged material is modeled with an arc gain factor, thereby allowing reduction of the total volume of material within the network but requiring use of a network-with-gains algorithm for solution of the operation problem. Application of the model defines cost-efficient dynamic schemes for allocation of material to available disposal sites. A generalized computer program was developed to define automatically the nodes, arcs, and parameters of the arcs of the network, given a description of the dredged-material disposal system. Structured analysis and structured programming techniques were used, thus providing a clear definition of the computations required, the order in which they must be accomplished, and the flow of data. This software development technique reduces the effort required for subsequent modification of the program to analyze the system capacity-expansion problem.					
15. SUBJECT TERMS dredged-material management, network-flow programming, software engineering, dredging, waste disposal, systems engineering, benefit-cost analysis, navigation					
16. SECURITY CLASSIFICATION OF:			17. LIMITATION OF ABSTRACT UU	18. NUMBER OF PAGES 20	19a. NAME OF RESPONSIBLE PERSON
a. REPORT U	b. ABSTRACT U	c. THIS PAGE U			19b. TELEPHONE NUMBER

Dredged-Material Disposal Management Model

January 1984

US Army Corps of Engineers
Institute for Water Resources
Hydrologic Engineering Center
609 Second Street
Davis, CA 95616

(530) 756-1104
(530) 756-8250 FAX
www.hec.usace.army.mil

TP-94

Papers in this series have resulted from technical activities of the Hydrologic Engineering Center. Versions of some of these have been published in technical journals or in conference proceedings. The purpose of this series is to make the information available for use in the Center's training program and for distribution with the Corps of Engineers.

The findings in this report are not to be construed as an official Department of the Army position unless so designated by other authorized documents.

The contents of this report are not to be used for advertising, publication, or promotional purposes. Citation of trade names does not constitute an official endorsement or approval of the use of such commercial products.

Dredged-Material Disposal Management Model

David T. Ford¹, M. ASCE

Abstract

To identify efficient dredged-material disposal management strategies for the Delaware River navigation system near Philadelphia, the system operation problem is formulated and solved as a generalized minimum cost network flow programming problem. This formulation represents material sources and available disposal sites as nodes of the network and transportation links and carry-over storages as arcs. The dewatering, consolidation, and densification of dredged material is modeled with an arc gain factor, thereby allowing reduction of the total volume of material within the network but requiring use of a network-with-gains algorithm for solution of the operation problem. Application of the model defines cost-efficient dynamic schemes for allocation of material to available disposal sites. A generalized compute program was developed to define automatically the nodes, arcs, and parameters of the arcs of the network, given a description of the dredged-material disposal system. Structured analysis and structured programming techniques were used, thus providing a clear definition of the computations required, the order in which they must be accomplished, and the flow of data. This software development technique reduces the effort required for subsequent modification of the program to analyze the system capacity-expansion problem.

Delaware River Disposal Management Problem

Background. - The Corps of Engineers has been responsible for maintenance of the navigable waterways of the United States since 1824. The maintenance includes excavation and disposal of the sediment deposited in the waterways. Current common practice is to excavate the material with a mechanical or hydraulic dredge (10) and to transport it to a disposal site either by pumping through a pipeline or by carrying the material to the site in barges or in hoppers on the dredge. The disposal site may be an offshore site selected to minimize interference with navigation or the disposal site may be a contained upland site. Contained disposal sites are natural or manmade ponding areas into which the dredged material is pumped or lifted. In the disposal site, water gradually drains and evaporates from the dredged material, and the solids densify and consolidate. The rate of dewatering, densifying, and consolidating can be increased by surface trenching, wicking, surcharging, and pumping with well pants. Detailed descriptions of these techniques and other technical aspects of dredged-material management are presented in Reference 3 and in associated reports of the Corps' Dredged Material Research Program.

Management of the long-term operation of a dredged-material disposal system requires selection of the equipment to be used for excavating and transporting the material from the channel to the disposal sites, allocation of the capacity of the available disposal sites to satisfy the demand for storage imposed by the dredging operation, selection of appropriate disposal-site management practices, and identification of capacity expansion schemes if the system capacity is exhausted at some time. Due to the complexity

¹ Hydraulic Engineer, Hydrologic Engineering Center, U.S., Army Corps of Engineers, Davis, California 95616.

Note. - Discussion open until June 1, 1984. To extend the closing date one month, a written request must be filed with the ASCE Manager of Technical and Professional Publications. The manuscript for this paper was submitted for review and possible publication on October 14, 1982. This paper is part of the Journal of Water Resources Planning and Management, Vol. 110, No. 1, January, 1984. ASCE, ISSN 0733-9496/84/0001-0051/\$01.00. Paper No. 18526.

of the long-term problem, equipment selection and capacity allocation generally is addressed only for the short-term, with equipment and sites selected for minimum cost at the time the dredging is performed. Physical and environmental limitations may constrain this selection and allocation. Likewise, problems of disposal-site operation and of capacity expansion generally are addressed with heuristic rules as problems arise.

Delaware River System. - The Delaware River, Delaware Bay, and associated tributaries are maintained in a navigable condition by the Philadelphia District, Corps of Engineers. Within this area, shown in Figure 1, twenty-three Federal navigation projects yield approximately 8,100,000 cu yd (6,200,000 m³) of dredged material annually. Non-Federal maintenance dredging contributes an additional 3,400,000 cu yd (2,600,000 m³). The material is disposed in twenty-one containment sites. According to estimates published in a 1979 study, by 1999 all these sites will be filled or unavailable due to lease expiration with continued maintenance dredging at current rates and with no change in management practices (4). This in turn would mean reduction or cessation of dredging and consequent reduction or cessation of navigation. The 1979 study identifies a number of management alternatives that may be employed, including:

1. Capacity expansion alternatives: (a) Acquisition of new upland sites; (b) open-water disposal of dredged material; and (c) extension of leases on sites.
2. Operation alternatives: (a) Dewatering of disposal sites; (b) increase in containment dike height; (c) reuse of dredged material; (d) reduction of maintenance dredging; (e) use of deposition basins to reduce shoaling; (f) reduction of sediment erosion; and (g) improvements in site management.



Figure 1 Delaware River System

Management Model Description

Model Objective. - The dredged-material disposal management model was developed for systematic evaluation of and comparison of alternative management schemes. With the model, capacity expansion alternatives can be analyzed, and the minimum-cost combination and schedule can be determined for new site acquisition and lease extension. Also, the minimum-net-cost operation policy for any specified system can be determined. This policy is required both for long-term system operation planning and for solution of the expansion problem; the total cost of any alternative capacity expansion scheme is a function of site acquisition, lease extension, and operation costs. The minimum operation cost and the associated operation policy are determined by formulating a mathematical programming model that represents the problem of allocating efficiently the available capacity. Disposal-site dewatering rates, containment dike heights, and other characteristics of the disposal system are specified by the model user, so management schemes that involve changes in these parameters are evaluated by systematic variation and re-execution of the model.

Initial development of the disposal management model is limited to formulation of the mathematical programming model presented herein for analysis of operation of a defined system. Ultimately the model will be expanded to address the capacity expansion problem, using a branch-and-bound algorithm which iteratively enumerates a limited number of alternative site acquisitions and lease renegotiation schemes, evaluates the efficiency and feasibility with the operation model, and identifies efficient schemes for expanding the system. The branch-and-bound procedure provides rules for eliminating from consideration many costly or infeasible schemes without actual evaluation with the operation model.

Mathematical Programming Formulation. - The mathematical programming formulation of the dredged-material disposal system operation problem includes continuity constraints for material sources and for disposal sites, transportation link and disposal-site capacity constraints, and carry-over storage constraints. The continuity and capacity constraints define the operation problem for each period. The carry-over storage constraints relate conditions within each period, yielding a multi-period operation problem. Unit costs are associated with transportation and disposal of dredged material. The objective is to minimize the total discounted cost of system operation. This formulation is similar to the solid-waste disposal model formulated by Marks and Liebman (13) and to the wastewater disposal model formulated by Brill and Nakamura (2).

A continuity constraint is included for each material source and for each disposal site for each period of analysis. The form of the equation for each material source, I , for each period T is:

$$\sum_{J=1}^{NDISP} F(I, J, T) = V(I, T) \quad (1)$$

in which:

- J = index of disposal sites
- $NDISP$ = total number of disposal sites
- $F(I, J, T)$ = volume of material transported from source I to site J in period T
- $V(I, T)$ = total volume of material dredged at source I during period T

The forms of the equation for each disposal site J for each period T is:

$$S(J, T-1) + VF(J) \left[\sum_{J=1}^{NDRG} F(I, J, T) \right] + \left[\sum_{\substack{J'=1 \\ J' \neq J}}^{NDISP} RT(J', J, T) - RT(J, J', T) \right] - RU(J, T) = S(J, T) \quad (2)$$

in which:

- $NDRG$ = total number of dredged-material sources
- $S(J, T-1)$ = volume of material stored at site J at beginning of period T and at the end of period $T-1$
- $S(J, T)$ = volume of material stored at site J at end of period T
- $RT(J, J', T)$ = volume of material transferred to site J from site J'
- $RT(J', J, T)$ = volume of material transferred from site J to site J' in period T
- $RU(J, T)$ = volume of material from site J removed and sold for reuse
- $VF(J)$ = an average volume-reduction factor.

The volume reduction factor reflects: (1) the wet-to-dry volume ratio of the dredged material; and (2) the efficiency of the disposal site management practices. The wet-to-dry volume ratio defines the average

volume of dry material per time period that must be stored at the disposal site as a fraction of the total volume of material in situ. E.g., a wet-to-dry ratio of 2.0 indicates that the dredged material, when wet, will occupy twice the volume occupied by the dried material. In this formulation the volume reduction is assumed to occur within one period. The efficiency of the disposal site in terms of achievement of the reduction depends on the site management techniques. If the techniques employed are one-hundred percent efficient, $VF(J)$ will equal the reciprocal of the wet-to-dry ratio; otherwise, $VF(J)$ will equal the product of this reciprocal and the estimated efficiency of the dewatering techniques used at the site. Typical values of $VF(J)$ range from 0.50 to 1.00.

The total volume of material to be transported to or from a disposal site is constrained by the characteristics of the pipeline, hopper, or other device used for transportation. Likewise, the volume of material deposited at a site each period T is constrained by the size of the site. These limitations are expressed mathematically as:

$$F(I, J, T) \leq FMAX(I, J) \quad (3)$$

$$RT(J, J', T) \leq RTMAX(J, J') \quad (4)$$

$$RU(J, T) \leq RUMAX(J) \quad (5)$$

$$S(J, T) \leq SMAX(J) \quad (6)$$

in which:

- FMAX(I,J) = capacity of the transportation link between dredged-material source I and disposal site J
- RTMAX(J,J') = capacity of the facilities for removing material from disposal site J and transferring it site J'
- RUMAX(J) = capacity of the facilities for removing material from disposal site J for reuse
- SMAX(J) = storage capacity of disposal site J

In addition to the restrictions on transportation, disposal-site management practices may pose a limitation on the rate of addition of "wet" material to the site. This limitation is imposed each period by the following constraint:

$$\sum_{I=1}^{NDRG} F(I, J, T) \leq ADDMAX(J) \quad (7)$$

in which:

ADDMAX(J) = maximum allowable volume addition per period

The operation problem is to determine the "best" scheme for allocating the material dredged each period to the available sites over the planning horizon. The efficiency of operation is defined as the algebraic sum of present value of costs of disposal and transportation and the benefits of reuse. Mathematically, this is expressed as

$$Z = \sum_{T=1}^{NPERS} (1+R)^{-T} \left\{ \sum_{I=1}^{NDRG} \sum_{J=1}^{NDISP} CF(I, J) * F(I, J, T) \right\} + \left\{ \sum_{J=1}^{NDISP} CS(J) * \sum_{I=1}^{NDRG} F(I, J, T) \right\} +$$

$$\left[\left\{ \sum_{J=1}^{NDISP} \sum_{\substack{J'=1 \\ J' \neq J}}^{NDISP} CRT(J, J') * RT(J, J', T) \right\} - \left\{ \sum_{J=1}^{NDISP} CRU(J) * RU(J, T) \right\} \right] \quad (8)$$

In which:

- Z = the present value of system net benefits for the period of analysis
- R = discount rate
- NPERS = number of time intervals
- CF(I,J) = unit cost of transporting material from dredge site *I* to disposal site *J*
- CS(J) = unit cost of adding material to disposal site *J*
- CRT(J,J') = unit cost of removing material from site *J*, transporting to and disposing in site *J'*
- CRU(J) = unit benefit of reuse of material from site *J*.

These costs and benefits are assumed to be constant over time. The objective is minimization of the net cost, *Z*.

Mathematical Programming Problem Solution. - The dredged-material system operation model as presented includes linear constraints and a linear objective function, so a linear programming (LP) algorithm can be used to determine the optimal allocation of dredged material to the available sites. However, the constraints define only conservation requirements and transportation limitations, and the costs and benefits are functions of the volume of material transported or stored, so the operation problem can be formulated as a network-flow programming problem. In a network-flow problem, the decisions required are visualized as flows in the arcs connecting the nodes of a network, and the objective is to choose the flow in each arc to optimize some efficiency measure, such as total cost. The arcs of the network are characterized by the allowable direction of flow, the maximum and minimum amounts of flow that can pass through each arc, the unit cost of use of the arc, and a gain that represents the fraction of flow that is lost (or gained) in each arc. The constraints are limited to conservation of flow at the nodes of the network and to upper and lower bounds on flows in the arcs. Algorithms for solution of the network-flow problems are more efficient than those for solution of the general LP problems. Ford and Fulkerson (8) and Jensen and Barnes (12) provide detailed descriptions of the characteristics of network-flow problems.

The network-flow model of the disposal operation problem represents material sources and available disposal sites as nodes and transportation links and carry-over storage as arcs. The network representation of a small disposal system is shown as Figure 2. Nodes 1 and 2 represent the Pedricktown North disposal site, nodes 3 and 4 represent the Pedricktown South site, and nodes 5 and 6 represent Overflow Site 1. Nodes 7, 8, and 9 represent the Marcus Hook, Bellevue, and Cherry Island dredge sites, respectively. The arcs connecting nodes 7, 8, and 9 with nodes 1, 3, and 5 represent the transportation links between the material sources and disposal

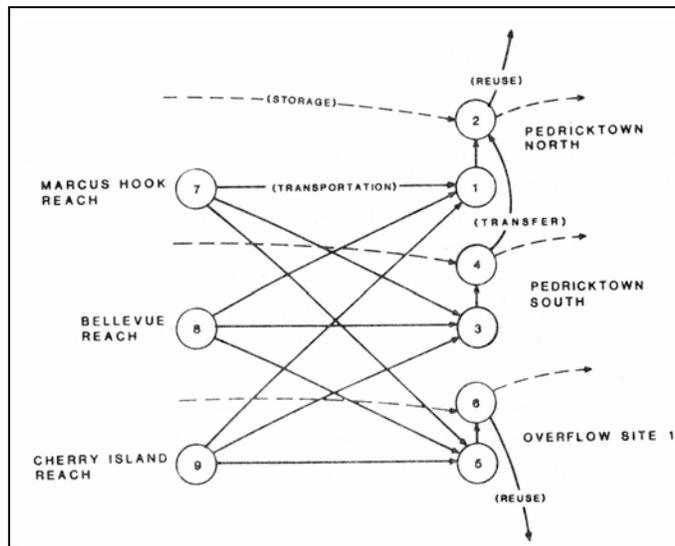


Figure 2 Single-Period Example of Network Representation

sites. The "flows" in these arcs represent the volumes of material allocated to the disposal sites. For a more complex system, arcs are included to connect each source with each site available for disposal of the material from that source. An upper bound is imposed on flow in these arcs, as dictated by the transportation method represented.

Removal of material from the disposal sites for reuse is represented by an arc originating at the disposal site node and terminating at a node that represents the point of sale of the reused material. Material removed from the Pedricktown South site and transferred to the Pedricktown North site is represented by flow in the arc originating at node 4 and terminating in node 2.

The arc originating at node 1 and terminating at node 2, the arc originating at node 3 and terminating at node 4, and the arc originating at node 5 and terminating at node 6 are included as a computational mechanism to represent the drying of material added to a disposal site and to limit the rate of addition of material to the site. The gains for these arcs are the volume reduction factors $[VF(J)]$ of Equation 2, and the upper bounds are the maximum allowable rates of disposal $[ADDMAX(J)]$ of Equation 7.

Material is introduced to the network at the nodes that represent the dredged-material sites. In the terminology of Jensen and Barnes (12), these volumes are node external flows; the quantity of flow entering the network is fixed.

The dashed arcs of Figure 2 represent the storage of material in the system disposal sites. The flow in the storage arc that terminates at node 2 represents the net volume of dried material deposited in the Pedricktown North site in all periods prior to the current period $[S(J,T-1)]$ of Equation 2. The storage arc originating at node 2 represents the cumulative volume of dried dredged-material deposited in the site after the addition and removal of material in the period shown $[S(J,T-1)]$ of Equation 2. When the network is expanded for analysis of multiple-period operation, these storage arcs link the networks that represent single-period problems. This is shown in Figure 3.

A unit cost is associated with the flow in each arc; the objective of the solution algorithm is to determine the allocation of flow to the various network arcs to minimize the sum of the product of flow in each arc and the corresponding cost. The unit costs assigned to the network arcs are the discounted units costs of storing, transporting, or re-handling material $[CS(J), CF(I,J), \text{ and } CRT(J,J')]$ of Equation 8] and the negative of the unit benefit of reuse $[CRU(J)]$ of Equation 8].

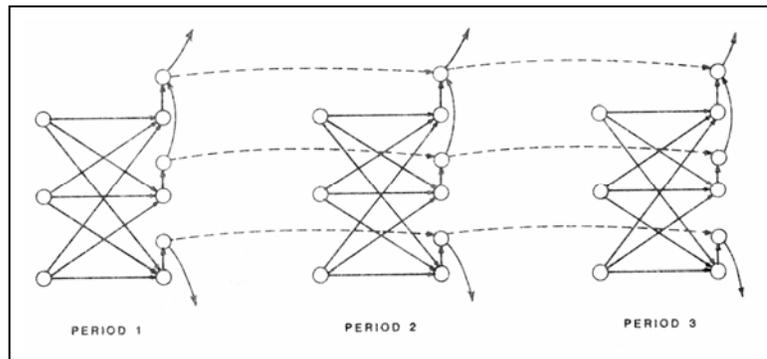


Figure 3 Multiple-Period Network Representation

The traditional network flow programming solution algorithms, such as the out-of-kilter algorithm (5,7,9) were developed for problems in which all gain factors are unity and, thus, are not applicable. Consequently, for solution of the management problem as formulated, a specialized network-with-gains algorithm is employed. This algorithm solves the generalized minimum-cost network flow problem with any nonnegative gain factors using a flow-augmentation algorithm. In this application, the algorithm begins with flow in all arcs set equal to zero. The minimum cost per unit of additional flow to each node of the network and the path over which that flow may be obtained is determined. The total flow through the network is increased along the minimum-cost path until the flow in one or more arcs in the path exceeds the bounds. This process continues iteratively until the required system input flows are satisfied or a maximum possible flow through the network is obtained. This algorithm guarantees achievement of

a feasible, global optimal solution if such a solution exists. Additional details of the algorithm are presented by Jensen and Bhaumik (11) and by Jensen and Barnes (12); a previous application of the algorithm is described in Reference 14.

Software Development

A generalized computer program was developed to implement the proposed disposal-system management model to evaluate alternative system capacity expansion plans. The program development employed state-of-the-art software engineering techniques, including structured analysis and structured programming (1).

Structure Analysis. - Structured analysis is a logical process for transforming information about program requirements into specifications for the program that is to be developed. This approach is contrary to usual engineering program development activities in which everyone eagerly gets on the "real" work - writing code. As described by Demarco (6), the structured analysis approach has the following characteristics: (1) it yields a paper model of the program-to-be; (2) the program is designed in a top-down, hierarchical fashion with a smooth progression from abstract definition of program components to a detailed definition; (3) it yields a set of connected "mini-specifications" of the identified program components; and (4) it uses diagrams for communication of ideas, especially between the program user, program designer, and computer system analyst.

Top-down program design begins with the establishment of firm requirements for the tasks to be accomplished by the program and with the definition of data required to accomplish the tasks. The overall program structure (top-level) is then defined, with progressive refinement of lower-level components of the program. Figure 4 shows the organization of the top level of the dredged-material disposal management program. The program consists of an "executive" routine controlling an "input", a "process", and an "output" routine. Figures 5 and 6 show further refinement of the process component; specification of the other components is refined in a similar manner. Development of the system management model was planned (and funded) for completion in two separate stages: Stage 1 includes only the operation model development, while Stage 2 addresses the capacity expansion problem in more detail. In Stage 1, several of the components shown were defined only in conceptual terms. For example, detailed specification of computational techniques was delayed initially in the case of component 3.1. Nevertheless, the data transfers and the required results of execution of each module were defined.

The network-flow programming model formulated to determine the optimal allocation of dredged material is identified as component 3.3.4 in Figures 6 and 7. As one of the goals of program development is to produce a management model usable by engineers and planners who are not familiar with mathematical programming techniques, component 3.3.4.1 is included here to translate disposal-system descriptive data into the node-arc representation. The resulting generalized minimum-cost network-flow problem is solved with code included in component 3.3.4.2. Definition of this component is further refined to include components of the network-with-gains algorithm.

Structured Programming. - The actual computer code to implement the disposal management model was developed from the structured analysis using structured programming techniques. Each of the components was translated into one or more subprograms that perform independently single tasks required for solution of the operation problem. The benefits of this approach are: (1) the actual development time is reduced because a number of programmers may simultaneously develop the modules, or existing code may be used easily; (2) complex programs may be tested in parts, with each module verified independently; (3) the code is easier to understand and to maintain; (4) the resulting code is flexible and may be modified by changing single modules independently; and (5) documentation of the code is easier. Items 3, 4, and 5 are significant given the environment within which the computer code

described here will be used. Although a single application motivated development, application to other disposal operation problems is likely and despite careful program design, past experience indicates a frequent need for special-case modification. Often these modifications must be performed by someone other than the original program writer, thus the need for understandable code.

Application

Operation Evaluation. - The operation model has been used to evaluate the operation of various existing and proposed configurations of the Delaware River dredged-material disposal system, including the subsystem between Philadelphia and the sea. As modeled, this subsystem includes nineteen dredge sites and eight disposal sites (of which two are imaginary sites for overflow if the system capacity is insufficient). Pertinent data describing the disposal sites are presented in Table 1. Material is dredged at average annual rates shown in Table 2 and is transported by barge, hopper, or pipeline to the disposal sites. The dredging and transporting costs depend on the machinery used and the distance which the material is transported; costs vary from \$1.62 to \$25.00/cu yd and are shown in Table 3. The unit costs of placing the material in the system disposal sites vary from \$0.00 to \$0.50/cu yd, as indicated in Table 1.

Disposal Site (1)	Capacity Remaining, in cubic yards (2)	Wet-to-dry Ratio (3)	Disposal Cost, in dollars per cubic yard (4)
Artificial Island	16,500,000	1.50	0.32
Overflow Site 1	99,000,000	1.00	0.00
Overflow Site 2	99,000,000	1.00	0.00
National Park	7,100,000	1.50	0.50
Killcohook	36,900,000	1.50	0.11
Penns Neck	16,000,000	1.50	0.25
Pedricktown North	21,700,000	1.50	0.16
Pedricktown South	21,700,000	1.50	0.17

Note: 1 cubic yard = 0.765 m³; 1 acre = 0.405 ha

The operation of the Philadelphia-to-sea subsystem was analyzed for fifty years using twenty-five consecutive two-year intervals. The resulting network consisted of 877 nodes and 3,709 arcs. Time required for definition of the network parameters and for solution of the minimum-cost optimization program on a commercial CYBER 175 computer system used by the Corps was approximately 59 CP seconds. The minimum present-value net cost for system operation with an annual discount rate of 7 5/8% is \$273 x 10⁶.

Figures 8-11 are reproductions of portions of the computer program output. Figure 8 is a summary of the dimensions of the system to be analyzed. Also, for each type of dredge, a function relating the unit excavating and transporting cost to distance transported is presented. Figure 9 is one of eight disposal site reports. In this report, the physical and economic characteristics of the disposal site are summarized. Site acquisition and lease renegotiation data are included for future use when the program is expanded to address the capacity expansion problem. Figure 10 is one of nineteen dredge site reports for this system. The alternative sites for disposal of material are shown, and the capacity and the unit cost of excavating

Dredge Site (1)	Volume (2)
Eddystone	12,300
Chester	890
Marcus Hook	1,850,300
Bellevue	49,900
Cherry Island	180,900
Deepwater Point	1,402,900
Bulkhead Bar	28,700
Newcastle	1,269,400
Reddy Island	28,000
Baker	10,800
Liston	220,800
Miah Maull	41,000
Brandywine	1,500
W. Horseshoe	25,800
Mifflin	67,400
Billingsport	5,600
Tinicum	43,200
Upper Philadelphia Harbor	6,500
Lower Philadelphia Harbor	181,500

Note: 1 cubic yard = 0.765 m³

Dredge Site (1)	Disposal Site							
	Artificial Island (2)	Overflow Site 1 (3)	Overflow Site 2 (4)	National Park (5)	Killcohook (6)	Penns Neck (7)	Pedricktown North (8)	Pedricktown South (9)
Eddystone	6.99	25.00	25.00	17.89	****	****	2.83	3.06
Chester	6.57	25.00	25.00	****	****	****	2.42	2.65
Marcus Hook	6.12	25.00	25.00	17.01	****	****	1.99	2.22
Bellevue	4.91	25.00	25.00	****	****	2.47	2.09	1.86
Cherry Island	****	25.00	25.00	15.40	****	2.08	2.48	2.25
Deepwater Point	3.57	25.00	25.00	****	2.01	1.95	***	3.23
Bulkhead Bar	3.16	25.00	25.00	****	1.61	2.35	***	***
Newcastle	2.48	25.00	25.00	****	1.98	3.02	***	***
Reddy Island	1.62	25.00	25.00	****	2.84	3.90	***	***
Baker	1.63	25.00	25.00	****	3.25	4.29	***	***
Liston	2.22	25.00	25.00	****	3.85	4.86	***	***
Miah Maull	7.25	19.25	19.25	****	8.88	9.94	***	***
Brandywine	8.70	15.24	15.24	****	10.33	11.36	***	***
W. Horseshoe	8.56	25.00	25.00	19.30	****	****	4.37	4.60
Mifflin	8.09	25.00	25.00	19.02	****	****	3.93	4.15
Billingsport	7.85	25.00	25.00	18.78	****	****	3.69	3.93
Tinicum	7.23	25.00	25.00	18.14	****	****	3.07	3.31
Upper Philadelphia Harbor	9.69	25.00	25.00	19.30	****	****	5.50	5.75
Lower Philadelphia Harbor	9.24	25.00	25.00	19.30	****	****	5.29	5.04

Note: * indicates sites not linked (1 cubic yard = 0.765 m³)

and transporting material to each site are tabulated. (The unit cost is determined from the appropriate unit cost versus distance function.) The estimated volumes of material to be removed each period are shown. (Note that the value shown for each period in Figure 10 is twice the corresponding value from Table 2 because each period in the analysis corresponds to two years.)

The results of optimal operation of the dredged-material disposal system are presented as a tabulation of material added to each disposal site each period and of end-of-period storage and the corresponding elevation and surface area. Figure 11 is an example of the tabulation for one site. The results of the minimum-cost operation are summarized in Table 4. All disposal sites except the overflow sites will be filled by the end of the fifty-year model. Overflow Site 1 is used initially in the ninth two-year interval, indicating that system capacity falls short of demand within eighteen years of the first year. This conclusion is significant but is difficult to draw with traditional mass analysis techniques because of the complex interconnections.

Systematic Evaluation of Operation Alternatives. - Evaluation of disposal site management alternatives is accomplished by systematic application of the operation model with variation of the appropriate input parameters. For example, to evaluate the cost effectiveness of use of trenching devices that speed the drying of deposited material in the disposal sites, the volume-reduction factor, $VF(J)$, and the maximum allowable volume addition per period, $ADDMAX(J)$, are changed to reflect the improvement possible, and the disposal cost, $CS(J)$, is altered as appropriate. The operation problem is resolved to determine the least-costly operation scheme. The cost of the trenching machinery is added to the operation cost to determine the total system cost. This total cost is compared with the total cost without the trenching devices; if the cost is less, the trenching device is cost effective.

Any management alternative can be evaluated by systematic analysis with the operation model, if the improvements attributable to that alternative can be expressed in terms of the volume-reduction factor, maximum allowable addition per period, maximum storage, capacity of transfer facilities, capacity of reuse facilities, or the time at which facilities are available. Techniques that alter the volume or time distribution of dredged material that must be disposed can be analyzed in the same systematic manner because the volumes are specified by the user for each period.

Conclusions

To identify efficient dredged-material disposal management strategies, the system operation problem can be formulated and solved as a generalized minimum-cost network flow programming problem. In this formulation, the material sources and disposal sites are represented as nodes, and the transportation links and carry-over storages are represented as arcs. The network-with-gains algorithm is used for solution, thereby allowing modeling of the drying material in the disposal sites.

The proposed disposal system operation model can be used for the evaluation of alternative management schemes by systematically varying the appropriate model parameters, re-executing the model, and comparing the net operation cost to determine the effectiveness of the scheme. In the future, this model will be linked with a branch-and-bound algorithm to identify efficient disposal system expansion schemes.

A generalized computer program to implement the proposed operations model was developed using software engineering methods. Structured analysis techniques were used to define the program requirements, and structured programming was used to transform the requirements into executable computer code.

The dredge-material disposal management model has been used successfully to evaluate the operation of the Delaware River disposal system.

Acknowledgements

The Philadelphia District, Corps of Engineers, provided funds for development of the dredged-material disposal management model and assisted in model formulation and verification. The special assistance of Brian Heverin of that office is appreciated. The software engineering concepts were introduced at the Hydrologic Engineering center by Rochelle Barkin, who patiently guided development of the FORTRAN program that implements the solution technique.

References

1. Boehm, B.W., "Software Engineering," *Classics in Software Engineering*, E.N. Yourdon, ed., Yourdon Press, New York, NY, 1979.
2. Brill, E.D., and Nakamura, M., "A Branch and Bound Method for Use in Planning Regional Wastewater Treatment Systems," *Water Resources Research*, Vol. 14, No. 1, February 1978, pages 109-118.
3. Corps of Engineers, U.S. Army, "Dredged Material Research Program Executive Overview and Detailed Summary," U.S. Army Engineer Waterways Experiment Station, Vicksburg, MS, December 1978.
4. Corps of Engineers, U.S. Army, "Delaware River Dredging Disposal Study, Stage 1 Reconnaissance Report," U.S. Army Engineer District, Philadelphia, PA, June 1979.
5. Classen, R.J., "The Numerical Solution of Network Problems Using the Out-of-Kilter Algorithm," *Rand Memorandum RM-5456-PR*, Santa Monica, CA, 1968.
6. Demarco, T., "Structured Analysis and System Specification," *Classics in Software Engineering*, E.N. Yourdon, ed., Yourdon Press, New York, NY, 1979.
7. Durbin, E.P., and Kroenke, D.M., "The Out-of-Kilter Algorithm: A Primer," *Rand Memorandum RM-5472-PR*, Santa Monica, CA, 1967.
8. Ford, L.R., and Fulkerson, D.R., *Flows in Networks*, Princeton University Press, Princeton, NJ, 1962.
9. Fulkerson, D.R., "An Out-of-Kilter Method for Solving Minimal Cost Flow Problems," *Society of Industrial and Applied Mathematics Journal of Applied Mathematics*, Vol. 9, 1961, pages 18-27.
10. Huston, J., *Hydraulic Dredging: Theoretical and Applied*, Cornell Maritime Press, Inc., Cambridge, MS, 1970.
11. Jensen, P.A., and Bhaumik, G., "A Flow Augmentation Approach to the Network with Gains Minimum Cost Flow Problem," *Management Science*, Vol. 23, 1977, pages 631-643.
12. Jensen, P.A., and Barnes, J.W., *Network Flow Programming*, John Wiley and Sons, Inc., New York, NY, 1980.
13. Marks, D.H., and Liebman, J.C., "Mathematical Analysis of Solid Waste Collection," *Public Health Service Publication No. 2104*, U.S. Department of Health, Education, and Welfare, Washington, DC, 1970.
14. Texas Water Development Board, "AL-V Surface Water Resources Allocation Model," UM-35, Austin, TX, 1981.

Notation

The following symbols are used in this paper:

- ADDMAX(J) = maximum allowable volume addition per period, site J ;
CF(I,J) = unit cost of transporting material from source I to disposal site J ;
CRT(J,J) = unit cost of re-handling material from disposal site J to site J ;
CRU(J) = unit benefit of reuse of material from site J ;
CS(J) = unit cost of adding material to site J ;
F(I,J,T) = volume of material transported from source I to site J in period T ;
FMAX(I,J) = capacity of transportation link between dredged-material source I and disposal site J ;
NDISP = number of disposal sites;
NDRG = number of dredged-material sources;
NPERS = number of the periods;
RT(J,J',T) = volume of material transferred from site J to site J' , period T ;
RTMAX(J,J') = capacity of transfer facilities between site J and site J' ;
RU(J,T) = volume of material removed from site J and sold for reuse, period T ;
RUMAX(J) = capacity of reuse facilities, disposal site J ;
S(J,T) = volume stored in disposal site J , period T ;
SMAX(J) = capacity of disposal site J ;
V(I,T) = total volume of material dredged at source I during period T ;
VF(J) = volume reduction factor, disposal site J ; and
Z = net system operating cost.

Subscripts

- I = dredged-material source;
 J = disposal site, and
 T = time period.

Technical Paper Series

TP-1	Use of Interrelated Records to Simulate Streamflow	TP-39	A Method for Analyzing Effects of Dam Failures in Design Studies
TP-2	Optimization Techniques for Hydrologic Engineering	TP-40	Storm Drainage and Urban Region Flood Control Planning
TP-3	Methods of Determination of Safe Yield and Compensation Water from Storage Reservoirs	TP-41	HEC-5C, A Simulation Model for System Formulation and Evaluation
TP-4	Functional Evaluation of a Water Resources System	TP-42	Optimal Sizing of Urban Flood Control Systems
TP-5	Streamflow Synthesis for Ungaged Rivers	TP-43	Hydrologic and Economic Simulation of Flood Control Aspects of Water Resources Systems
TP-6	Simulation of Daily Streamflow	TP-44	Sizing Flood Control Reservoir Systems by System Analysis
TP-7	Pilot Study for Storage Requirements for Low Flow Augmentation	TP-45	Techniques for Real-Time Operation of Flood Control Reservoirs in the Merrimack River Basin
TP-8	Worth of Streamflow Data for Project Design - A Pilot Study	TP-46	Spatial Data Analysis of Nonstructural Measures
TP-9	Economic Evaluation of Reservoir System Accomplishments	TP-47	Comprehensive Flood Plain Studies Using Spatial Data Management Techniques
TP-10	Hydrologic Simulation in Water-Yield Analysis	TP-48	Direct Runoff Hydrograph Parameters Versus Urbanization
TP-11	Survey of Programs for Water Surface Profiles	TP-49	Experience of HEC in Disseminating Information on Hydrological Models
TP-12	Hypothetical Flood Computation for a Stream System	TP-50	Effects of Dam Removal: An Approach to Sedimentation
TP-13	Maximum Utilization of Scarce Data in Hydrologic Design	TP-51	Design of Flood Control Improvements by Systems Analysis: A Case Study
TP-14	Techniques for Evaluating Long-Term Reservoir Yields	TP-52	Potential Use of Digital Computer Ground Water Models
TP-15	Hydrostatistics - Principles of Application	TP-53	Development of Generalized Free Surface Flow Models Using Finite Element Techniques
TP-16	A Hydrologic Water Resource System Modeling Techniques	TP-54	Adjustment of Peak Discharge Rates for Urbanization
TP-17	Hydrologic Engineering Techniques for Regional Water Resources Planning	TP-55	The Development and Servicing of Spatial Data Management Techniques in the Corps of Engineers
TP-18	Estimating Monthly Streamflows Within a Region	TP-56	Experiences of the Hydrologic Engineering Center in Maintaining Widely Used Hydrologic and Water Resource Computer Models
TP-19	Suspended Sediment Discharge in Streams	TP-57	Flood Damage Assessments Using Spatial Data Management Techniques
TP-20	Computer Determination of Flow Through Bridges	TP-58	A Model for Evaluating Runoff-Quality in Metropolitan Master Planning
TP-21	An Approach to Reservoir Temperature Analysis	TP-59	Testing of Several Runoff Models on an Urban Watershed
TP-22	A Finite Difference Methods of Analyzing Liquid Flow in Variably Saturated Porous Media	TP-60	Operational Simulation of a Reservoir System with Pumped Storage
TP-23	Uses of Simulation in River Basin Planning	TP-61	Technical Factors in Small Hydropower Planning
TP-24	Hydroelectric Power Analysis in Reservoir Systems	TP-62	Flood Hydrograph and Peak Flow Frequency Analysis
TP-25	Status of Water Resource System Analysis	TP-63	HEC Contribution to Reservoir System Operation
TP-26	System Relationships for Panama Canal Water Supply	TP-64	Determining Peak-Discharge Frequencies in an Urbanizing Watershed: A Case Study
TP-27	System Analysis of the Panama Canal Water Supply	TP-65	Feasibility Analysis in Small Hydropower Planning
TP-28	Digital Simulation of an Existing Water Resources System	TP-66	Reservoir Storage Determination by Computer Simulation of Flood Control and Conservation Systems
TP-29	Computer Application in Continuing Education	TP-67	Hydrologic Land Use Classification Using LANDSAT
TP-30	Drought Severity and Water Supply Dependability	TP-68	Interactive Nonstructural Flood-Control Planning
TP-31	Development of System Operation Rules for an Existing System by Simulation	TP-69	Critical Water Surface by Minimum Specific Energy Using the Parabolic Method
TP-32	Alternative Approaches to Water Resources System Simulation		
TP-33	System Simulation of Integrated Use of Hydroelectric and Thermal Power Generation		
TP-34	Optimizing flood Control Allocation for a Multipurpose Reservoir		
TP-35	Computer Models for Rainfall-Runoff and River Hydraulic Analysis		
TP-36	Evaluation of Drought Effects at Lake Atitlan		
TP-37	Downstream Effects of the Levee Overtopping at Wilkes-Barre, PA, During Tropical Storm Agnes		
TP-38	Water Quality Evaluation of Aquatic Systems		

TP-70	Corps of Engineers Experience with Automatic Calibration of a Precipitation-Runoff Model	TP-105	Use of a Two-Dimensional Flow Model to Quantify Aquatic Habitat
TP-71	Determination of Land Use from Satellite Imagery for Input to Hydrologic Models	TP-106	Flood-Runoff Forecasting with HEC-1F
TP-72	Application of the Finite Element Method to Vertically Stratified Hydrodynamic Flow and Water Quality	TP-107	Dredged-Material Disposal System Capacity Expansion
TP-73	Flood Mitigation Planning Using HEC-SAM	TP-108	Role of Small Computers in Two-Dimensional Flow Modeling
TP-74	Hydrographs by Single Linear Reservoir Model	TP-109	One-Dimensional Model for Mud Flows
TP-75	HEC Activities in Reservoir Analysis	TP-110	Subdivision Froude Number
TP-76	Institutional Support of Water Resource Models	TP-111	HEC-5Q: System Water Quality Modeling
TP-77	Investigation of Soil Conservation Service Urban Hydrology Techniques	TP-112	New Developments in HEC Programs for Flood Control
TP-78	Potential for Increasing the Output of Existing Hydroelectric Plants	TP-113	Modeling and Managing Water Resource Systems for Water Quality
TP-79	Potential Energy and Capacity Gains from Flood Control Storage Reallocation at Existing U.S. Hydropower Reservoirs	TP-114	Accuracy of Computer Water Surface Profiles - Executive Summary
TP-80	Use of Non-Sequential Techniques in the Analysis of Power Potential at Storage Projects	TP-115	Application of Spatial-Data Management Techniques in Corps Planning
TP-81	Data Management Systems of Water Resources Planning	TP-116	The HEC's Activities in Watershed Modeling
TP-82	The New HEC-1 Flood Hydrograph Package	TP-117	HEC-1 and HEC-2 Applications on the Microcomputer
TP-83	River and Reservoir Systems Water Quality Modeling Capability	TP-118	Real-Time Snow Simulation Model for the Monongahela River Basin
TP-84	Generalized Real-Time Flood Control System Model	TP-119	Multi-Purpose, Multi-Reservoir Simulation on a PC
TP-85	Operation Policy Analysis: Sam Rayburn Reservoir	TP-120	Technology Transfer of Corps' Hydrologic Models
TP-86	Training the Practitioner: The Hydrologic Engineering Center Program	TP-121	Development, Calibration and Application of Runoff Forecasting Models for the Allegheny River Basin
TP-87	Documentation Needs for Water Resources Models	TP-122	The Estimation of Rainfall for Flood Forecasting Using Radar and Rain Gage Data
TP-88	Reservoir System Regulation for Water Quality Control	TP-123	Developing and Managing a Comprehensive Reservoir Analysis Model
TP-89	A Software System to Aid in Making Real-Time Water Control Decisions	TP-124	Review of U.S. Army corps of Engineering Involvement With Alluvial Fan Flooding Problems
TP-90	Calibration, Verification and Application of a Two-Dimensional Flow Model	TP-125	An Integrated Software Package for Flood Damage Analysis
TP-91	HEC Software Development and Support	TP-126	The Value and Depreciation of Existing Facilities: The Case of Reservoirs
TP-92	Hydrologic Engineering Center Planning Models	TP-127	Floodplain-Management Plan Enumeration
TP-93	Flood Routing Through a Flat, Complex Flood Plain Using a One-Dimensional Unsteady Flow Computer Program	TP-128	Two-Dimensional Floodplain Modeling
TP-94	Dredged-Material Disposal Management Model	TP-129	Status and New Capabilities of Computer Program HEC-6: "Scour and Deposition in Rivers and Reservoirs"
TP-95	Infiltration and Soil Moisture Redistribution in HEC-1	TP-130	Estimating Sediment Delivery and Yield on Alluvial Fans
TP-96	The Hydrologic Engineering Center Experience in Nonstructural Planning	TP-131	Hydrologic Aspects of Flood Warning - Preparedness Programs
TP-97	Prediction of the Effects of a Flood Control Project on a Meandering Stream	TP-132	Twenty-five Years of Developing, Distributing, and Supporting Hydrologic Engineering Computer Programs
TP-98	Evolution in Computer Programs Causes Evolution in Training Needs: The Hydrologic Engineering Center Experience	TP-133	Predicting Deposition Patterns in Small Basins
TP-99	Reservoir System Analysis for Water Quality	TP-134	Annual Extreme Lake Elevations by Total Probability Theorem
TP-100	Probable Maximum Flood Estimation - Eastern United States	TP-135	A Muskingum-Cunge Channel Flow Routing Method for Drainage Networks
TP-101	Use of Computer Program HEC-5 for Water Supply Analysis	TP-136	Prescriptive Reservoir System Analysis Model - Missouri River System Application
TP-102	Role of Calibration in the Application of HEC-6	TP-137	A Generalized Simulation Model for Reservoir System Analysis
TP-103	Engineering and Economic Considerations in Formulating	TP-138	The HEC NexGen Software Development Project
TP-104	Modeling Water Resources Systems for Water Quality	TP-139	Issues for Applications Developers
		TP-140	HEC-2 Water Surface Profiles Program
		TP-141	HEC Models for Urban Hydrologic Analysis

- TP-142 Systems Analysis Applications at the Hydrologic Engineering Center
- TP-143 Runoff Prediction Uncertainty for Ungauged Agricultural Watersheds
- TP-144 Review of GIS Applications in Hydrologic Modeling
- TP-145 Application of Rainfall-Runoff Simulation for Flood Forecasting
- TP-146 Application of the HEC Prescriptive Reservoir Model in the Columbia River Systems
- TP-147 HEC River Analysis System (HEC-RAS)
- TP-148 HEC-6: Reservoir Sediment Control Applications
- TP-149 The Hydrologic Modeling System (HEC-HMS): Design and Development Issues
- TP-150 The HEC Hydrologic Modeling System
- TP-151 Bridge Hydraulic Analysis with HEC-RAS
- TP-152 Use of Land Surface Erosion Techniques with Stream Channel Sediment Models
- TP-153 Risk-Based Analysis for Corps Flood Project Studies - A Status Report
- TP-154 Modeling Water-Resource Systems for Water Quality Management
- TP-155 Runoff simulation Using Radar Rainfall Data
- TP-156 Status of HEC Next Generation Software Development
- TP-157 Unsteady Flow Model for Forecasting Missouri and Mississippi Rivers
- TP-158 Corps Water Management System (CWMS)
- TP-159 Some History and Hydrology of the Panama Canal
- TP-160 Application of Risk-Based Analysis to Planning Reservoir and Levee Flood Damage Reduction Systems
- TP-161 Corps Water Management System - Capabilities and Implementation Status

